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FINAL
HYDROLOGY REPORT
OUTER LOOP HIGHWAY
CAMELBACK WALK CHANNEL TO
THE ARIZONA CANAL

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HYDROLOGY REPORT
OUTER LOOP HIGHWAY
CAMELBACK WALK CHANNEL TO
THE ARIZONA CANAL

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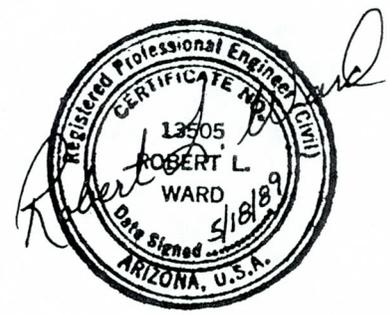


TABLE OF CONTENTS

	<u>Page</u>
LIST OF FIGURES, TABLES, PLATES, AND APPENDICES	ii
I. INTRODUCTION	1
II. DRAINAGE BASIN CHARACTERISTICS	2
2.1 <u>Drainage Area</u>	2
2.1.1 <u>CAP Aqueduct</u>	2
2.1.2 <u>Scottsdale Ranch</u>	4
2.1.3 <u>Urban Retention/Detention Basins</u>	5
2.1.4 <u>Arizona Canal</u>	5
2.2 <u>Land Use</u>	6
2.3 <u>Soil Data</u>	6
2.4 <u>Existing Drainage Facilities</u>	8
III. RAINFALL CHARACTERISTICS	9
IV. HYDROLOGIC MODEL (HEC-1)	12
4.1 <u>Delineation of Drainage Sub-Basins</u>	12
4.2 <u>Interception/Infiltration</u>	13
4.3 <u>Configuration of Hydrologic Model</u>	16
4.3.1 <u>Kinematic Wave Parameters</u>	18
4.3.2 <u>Channel Routing Along Arizona Canal</u>	20
4.3.3 <u>Model Assumptions for Scottsdale Ranch Lake System</u>	22
4.3.4 <u>Model Assumptions for Onsite Detention/Retention</u>	23
4.4 <u>Hydrologic Modeling Results</u>	26
4.5 <u>Verification of Model Results</u>	27
4.5.1 <u>Agricultural Fields</u>	29
4.5.2 <u>Undeveloped Desert</u>	29
4.5.3 <u>Urbanized Land</u>	31
V. CONCLUSIONS AND RECOMMENDATIONS	33

APPENDICES



TABLE OF CONTENTS
(continued)

	<u>Page</u>
LIST OF FIGURES	
Figure 2.1 Vicinity Map and Drainage Area	3
Figure 3.1 Rainfall Distribution	11
Figure 4.1 Percent of Impervious Cover vs. Dwelling Units/Acre	17
Figure 4.2 Typical Cross Section Used for Modified Puls Routing Along Arizona Canal	21
Figure 4.3 Outflow Hydrograph From Scottsdale Ranch Lake	24

LIST OF TABLES

Table 3.1 Rainfall Depths as a Function of Recurrence Interval and Duration	10
Table 4.1 Baseline Curve Numbers for Agricultural Fields	15
Table 4.2 Baseline Curve Numbers for Undeveloped Desert	15
Table 4.3 Peak Discharge Summary Along Outer Loop Alignment ..	28
Table 4.4 Summary of Verification Calculations for Undeveloped Desert Parcels	31
Table 4.5 Summary of Verification Calculations for Urbanized Parcels	32

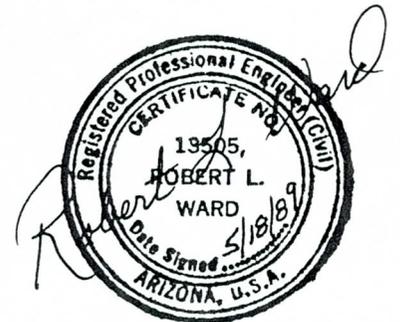
LIST OF PLATES

Plate 1	Aerial Photograph of Watershed Boundaries
Plate 2	Hydrologic Soil Group Map
Plate 3	Drainage Patterns, Sub-Basin Boundaries and Concentration Points

LIST OF APPENDICES

APPENDIX A	Summary of HEC-1 Input and Output Data
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PAZ-DC-02 Task 2/PH/2R01



I. INTRODUCTION

The purpose of this report is to document the assumptions and methodologies used to develop the offsite hydrology for the Concept Drainage Plan for an approximate 2.9 mile reach of the Outer Loop Highway located between the Camelback Walk Channel and the Arizona Canal.

The following sections of this report present a discussion of drainage area characteristics, meteorological conditions, and the results of the hydrologic modeling process that was undertaken to develop the rainfall/runoff data required for the design of a cross-drainage system for this reach of the highway.

This report supercedes a "Preliminary Hydrology Report" published in December 1986 for this segment of the Outer Loop. The 1986 study was based on a highway alignment along Pima Road. This revised 1989 study is based on a new highway alignment that follows a curvilinear path ranging from approximately 400 feet to 3700 feet east of Pima Road. The 1989 study also includes some additional drainage area, intercepted by the Arizona Canal, that should be considered in the design of the Outer Loop bridge over the Arizona Canal.

II. DRAINAGE BASIN CHARACTERISTICS

2.1 Drainage Area

Figure 2.1 presents a vicinity map showing the preliminary highway alignment and the perimeter of the drainage area that contributes runoff to this segment of the highway.

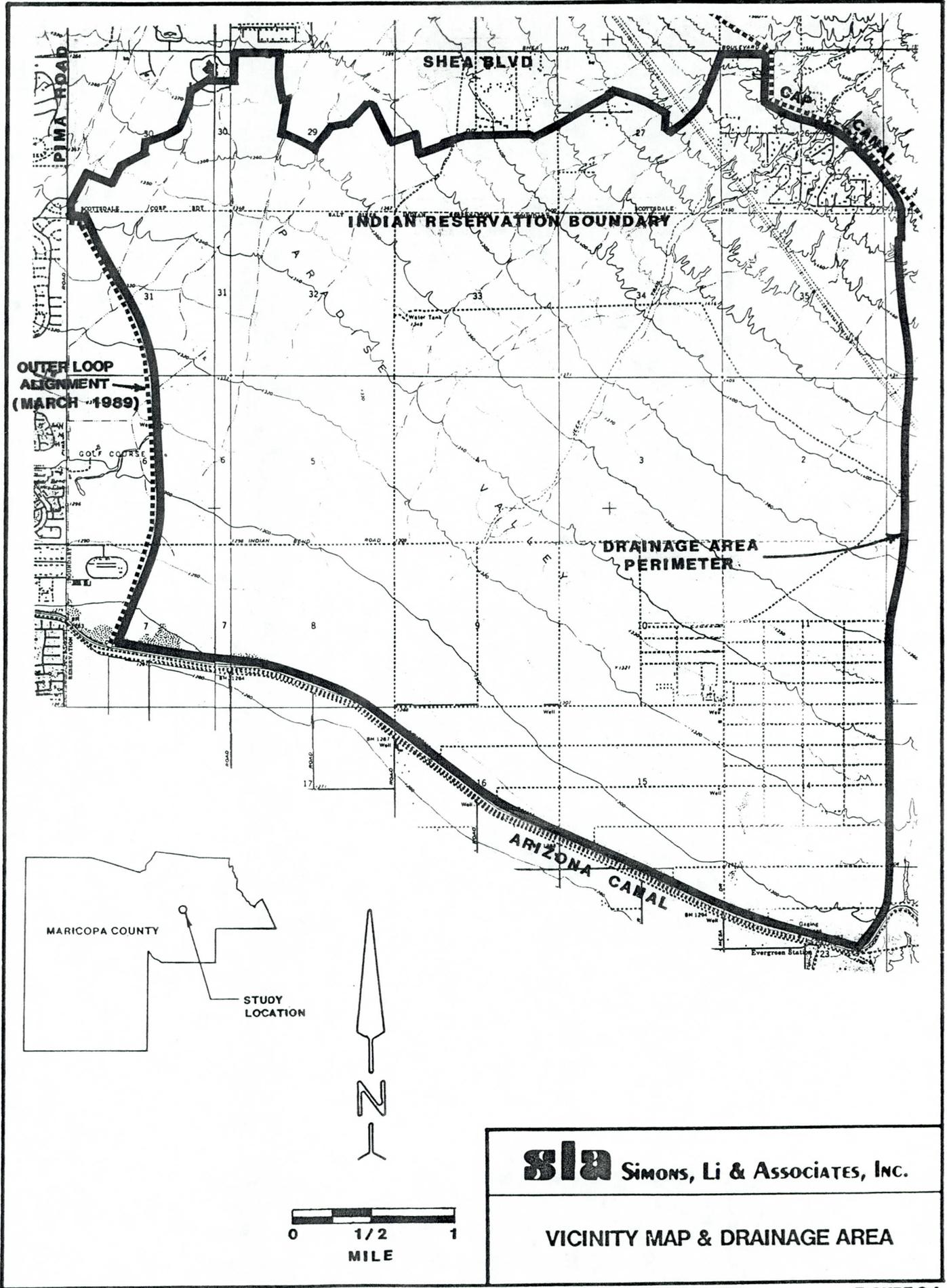
Several manmade alterations to the drainage area have created a unique set of conditions that have a significant impact on the runoff characteristics of the watershed, as related to the determination of a peak discharge for use in designing an offsite drainage system for the Outer Loop. These alterations include: 1) construction of the Central Arizona Project (CAP) Aqueduct; 2) development of a Master Drainage and Flood Control Plan for Scottsdale Ranch; 3) inclusion of onsite detention/retention basins for private development surrounding the ranch; and, 4) construction of the Arizona Canal. The importance of these alterations are discussed in the following paragraphs.

2.1.1 CAP Aqueduct

The large flood control dikes constructed along the upstream side of the CAP intercept runoff from the McDowell Mountains that would, under natural conditions, ultimately reach the proposed highway alignment along Pima Road. This intercepted drainage area is approximately 8.8 square miles. However, for the drainage area under investigation, there are four cross-drainage structures that allow some runoff to cross the CAP aqueduct. The first of these structures is a lined, 48-inch CMP culvert located at the intersection of the CAP with 112th Street. The runoff that passes through this structure is further controlled by a downstream lake system on Scottsdale Ranch.

The second cross-drainage structure consists of three 72-inch RCP culverts located approximately 750 feet south of Shea Boulevard (CAP Station 234+10). The discharge from this structure ($Q_p = 834$ cfs) will be intercepted by the Arizona Canal and diverted west, where it will influence the design of the Outer Loop bridge over the Arizona Canal.

The third CAP structure is a 36-inch RCP located about 1700 feet south of Shea Boulevard (CAP Station 243+00). The peak discharge from this structure is 37 cfs. The outflow hydrograph from this site will also be intercepted by the



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VICINITY MAP & DRAINAGE AREA

FIGURE 2.1

Arizona Canal and diverted west where it will influence the design of the Outer Loop bridge over the canal.

The fourth cross-drainage structure is located approximately 6300 feet south of Shea Boulevard (CAP Station 332+75). This site consists of two 66-inch lined CMP culverts with a maximum outflow of 437 cfs. These two culverts are at the eastern edge of the drainage area boundary for this reach of the Outer Loop. The braided channel pattern that exists downstream of the culvert outlets creates difficulty in accurately identifying the specific amount of culvert outflow that will actually enter the Outer Loop drainage system; i.e., substantial portions of these culvert flows will probably be discharged to the Salt River through the Evergreen Wasteway gates on the Arizona Canal.

Based on a review of aerial photographs, it is evident that the outflow from these two culverts will be captured by a system of washes that will carry the outflow to a location along the Arizona Canal that is approximately two miles east of the Evergreen Wasteway. As a result, the intercepted flows would be discharged through the wasteway gates and not have any impact on the Outer Loop.

However, as the culvert outflow travels from the CAP to the Arizona Canal, the braided channel pattern makes a slight contact with the east edge of the watershed boundary for the Outer Loop. It is conceivable that some of this shallow, braided flow might break into the Outer Loop drainage area.

To acknowledge this possibility, an outflow hydrograph for this CAP cross-drainage structure was included in the Outer Loop HEC-1 model. A divert routine was also included in the model to control the amount of CAP cross-drainage flow that is allowed to enter the Outer Loop model. Based on the existence of the very prominent channel pattern that should prevent this water from entering the Outer Loop watershed, only 15 percent of the cross-drainage flow from CAP Station 332+75 was diverted into the Outer Loop.

2.1.2 Scottsdale Ranch

The Master Drainage and Flood Control Plan for Scottsdale Ranch includes a large manmade lake, which is designed to provide approximately 344.5 acre-feet of flood control storage, as well as provisions for several onsite retention basins on various parcels of land within the ranch. Discussions with representatives from Collar, Williams, and White Engineering, Inc. (consulting engineers

for the Scottsdale Ranch development) and with representatives from the City of Scottsdale, indicate this lake system was designed on the basis of a 100-year, 24-hour storm. Accordingly, this lake, along with the onsite retention basins, have the capacity to substantially attenuate runoff hydrographs that emanate from Scottsdale Ranch or upstream drainage areas. This attenuation is an important factor to consider when conducting the hydrology analysis for the off-site drainage design for this reach of the Outer Loop.

2.1.3 Urban Retention/Detention Basins

The third manmade alteration that impacts this hydrology study is the construction of onsite detention/retention basins for those portions of the contributing drainage area west of 96th Street. Field inspections of this area indicated substantial onsite detention storage is present. Depending on site-specific design criteria, these basins may be able to capture all or part of the runoff from their contributing drainage area.

2.1.4 Arizona Canal

The Arizona Canal is the fourth manmade alteration that effectively forms a southern boundary for the watershed. This irrigation canal has a raised embankment (of variable height) along its northern side, as well as several inlet locations to allow ponded runoff to enter the canal. Southerly flowing storm runoff will pond along this embankment and, depending on location along the canal, will either flow towards the east or west and/or be allowed to spill into the canal.

Specific modeling techniques used to account for the influence of these four manmade alterations will be discussed in detail in subsequent sections of this report.

Considering the hydrologic control exerted on the watershed by the CAP, the Scottsdale Ranch lake system, and the Arizona Canal, the drainage area can be sub-divided into two very distinct categories: 1) the commercial/industrial/-urbanized area within the City of Scottsdale (approximately 2.9 square miles);

and 2) the undeveloped and agricultural property on the Salt River Indian Reservation (approximately 16.6 square miles).

The following subsections discuss the specific characteristics of each of these two major drainage area components.

2.2 Land Use

Plate 1 is an aerial photograph of the project area showing the degree of development existing as of October 1986. As can be seen from this photo, the majority of the drainage area north of the Salt River Indian Reservation is already developed.

The lower portion of the watershed lies within the Salt River Indian Reservation. This area is composed of approximately 30% irrigated agricultural fields and about 70% undeveloped desert land.

2.3 Soil Data

Soils information is needed in order to model the infiltration characteristics of the watershed. Such information is generally available from Soil Survey Reports published by the Soil Conservation Service (SCS). The watershed for this project was included in the Soil Survey of Aguila-Carefree Area, Parts of Maricopa and Pinal Counties, Arizona, U.S. Department of Agriculture, April 1986.

Using the standard SCS hydrologic soil group classification system, an estimate can be made of the runoff potential of the soils within any given sub-basin of the project watershed. The SCS system is based on four hydrologic soil groups, A through D. Soils in group A have very low runoff potential (i.e., high infiltration rate), those in group B have moderately low runoff potential, those in group C have moderately high runoff potential, and those in group D have high runoff potential (i.e., very slow infiltration rate).

Plate 2 illustrates the composition of the project watershed in terms of hydrologic soil groups. The information in this figure is based on the previously referenced soil survey. As can be seen from Plate 2, the watershed is composed of two hydrologic soil groups (B and C).

From the SCS soil survey, the following descriptions are provided for some of the major soil classifications comprising the drainage area.

1. Soil Group B - Mohall Clay Loam

This deep and well drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Slope is 0 to 3 percent.

Typically, the surface layer is light brown clay loam about two inches thick. The upper 19 inches of the subsoil is light brown clay loam, and the lower 21 inches is light reddish brown and light brown, calcareous clay loam. The substratum to a depth of 60 inches or more is light reddish brown, calcareous extremely cobbly loamy sand.

Permeability of this Mohall soil is moderately slow. Available water capacity is high. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard of water erosion is slight.

2. Soil Group C - Contine Clay Loam

This deep and well drained soil is on fan terraces. It formed in alluvium derived dominantly from acid and basic igneous rock. Slope is 0 to 3 percent.

Typically, the surface layer is brown, calcareous clay loam about two inches thick. The subsoil is reddish brown, calcareous clay loam and clay 28 inches thick. The substratum to a depth of 60 inches or more is light reddish brown, calcareous sandy loam.

Permeability of this Contine soil is slow. Available water capacity is high. Effective rooting depth is 60 inches or more. Runoff is slow, and the hazard of water erosion is slight.

The soils data discussed in this section was used to select Soil Conservation Service (SCS) curve numbers. These curve numbers, which model the hydrologic abstractions and infiltration characteristics of the watershed, are discussed in more detail in Section 4.2 of this report.

2.4 Existing Drainage Facilities

As previously discussed in Section 2.1, the portion of the drainage area lying within the City of Scottsdale incorporates major drainage improvements which may have a significant influence on the runoff response of the watershed. These improvements are in the form of onsite detention/retention basins and a master drainage/flood control plan for Scottsdale Ranch.

Other than some small earth berms alongside certain dirt roads/trails, there are no significant drainage facilities on the Reservation.

III. RAINFALL CHARACTERISTICS

The hydrologic response of a watershed is dependent upon rainfall characteristics such as depth, duration, and the spatial and temporal distribution of the rainfall event. The rainfall depth is a function of the probability of occurrence and the duration of the event. This probability is expressed as a recurrence interval (50-year, 100-year, etc.), which is defined as the average interval of time within which the magnitude of an event will be equaled or exceeded once. Mathematically, recurrence interval is defined as the reciprocal of the probability of occurrence.

Evaluating storms with different recurrence intervals is required when considering the risk and economic factors associated with the design of a drainage system for a specific meteorological event. In order to incorporate a risk analysis into the freeway design process, ADOT has requested that both the 50- and 100-year storms be evaluated as part of the hydrologic analysis of the Outer Loop.

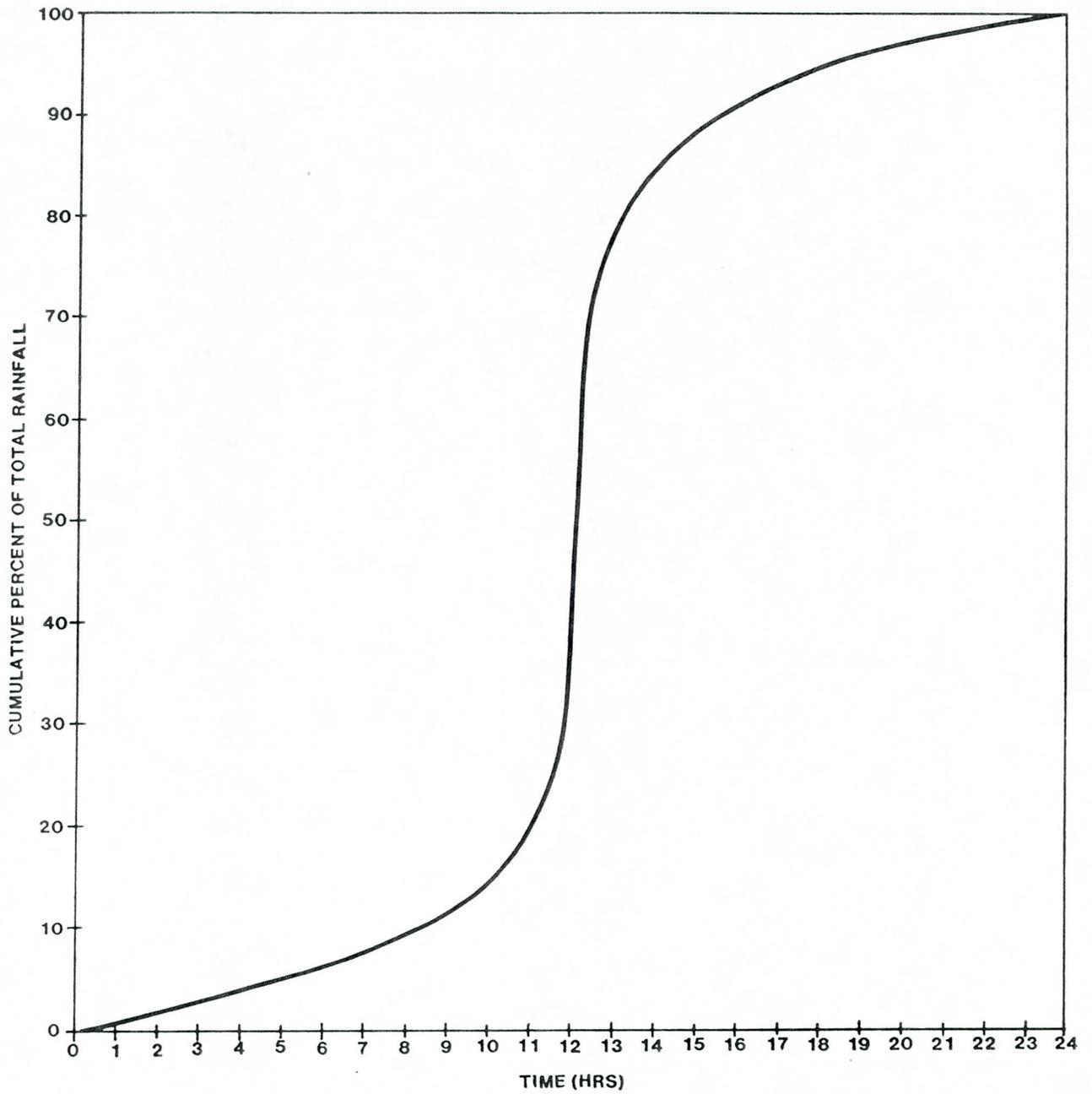
Rainfall depths for the project drainage area were developed using isopluvial maps and regression equations presented in the Precipitation-Frequency Atlas of the Western United States, Volume VIII - Arizona. Table 3.1 Summarizes point precipitation values for the 50- and 100-year storms for durations of 5 minutes, 15 minutes, and 1-, 2-, 3-, 6-, 12-, and 24-hours.

When using the hypothetical storm distribution in HEC-1, point rainfall values are automatically reduced (by the computer program) in accordance with procedures outlined in Weather Bureau Technical Paper No. 40, May 1961, to account for areal reduction of rainfall depths over large drainage areas. Due to the small size of the watershed under investigation, and the use of a 24-hour storm duration, no appreciable areal reduction in point precipitation values was used. This was simulated by inserting a drainage area size of 0.1 square mile on the PH card in the HEC-1 model.

Since De Leuw, Cather & Company and ADOT have previously selected the 24-hour, HEC-1 hypothetical storm distribution for use in designing the offsite drainage improvements for the reach of the Outer Loop immediately north of this segment, no attempt was made during this study to evaluate different storm distributions, i.e., the analysis was confined to the 100- and 50-year, 24-hour events. Figure 3.1 presents a graphical illustration of the selected rainfall

distribution. The reader is referred to the Hydrology Report, Outer Loop Highway, CAP Aqueduct to the Salt River Indian Reservation, August 19, 1986 by Simons, Li & Associates, Inc. for an overview of HEC-1 model sensitivity to several different rainfall distributions.

Recurrence Interval (Years)	Point Precipitation Values (inches)							
	Duration							
	5-min	15-min	1-hr	2-hr	3-hr	6-hr	12-hr	24-hr
50	0.63	1.25	2.19	2.43	2.59	2.90	3.26	3.62
100	0.72	1.42	2.50	2.77	2.95	3.29	3.68	4.08



24-HR HEC-1 HYPOTHETICAL RAINFALL DISTRIBUTION

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RAINFALL DISTRIBUTION

FIGURE 3.1

IV. HYDROLOGIC MODEL (HEC-1)

A computerized rainfall/runoff model was developed for the watershed using the U.S. Army Corps of Engineers Flood Hydrograph Package (HEC-1). HEC-1 uses numerical parameters to describe the amount and temporal distribution of rainfall, the runoff characteristics of the watershed, and the hydraulic properties of channels that collect and convey the direct runoff to concentration points. The computer output provides a runoff hydrograph at user selected locations. These hydrographs can be used to design drainage channels, detention/retention basins, or to evaluate the capacity of existing drainage facilities.

This section of the report presents a detailed discussion of specific components of the computer model that were created to simulate the rainfall/runoff response of the watershed. The results of the modeling process are also presented (Section 4.4) along with a brief discussion of model verification (Section 4.5).

4.1 Delineation of Drainage Sub-Basins

As discussed previously, there are several manmade features which effectively control the delineation of drainage area boundaries for this watershed. The proposed alignment of the Outer Loop (east of Pima Road) sets the western boundary of the watershed, while the Arizona Canal and the Camelback Walk Channel/Scottsdale Ranch development establish south and north boundaries, respectively.

The northeast and eastern boundaries of the watershed are a function of the CAP Aqueduct and the land slope along the northern side of the Arizona Canal. The northeastern boundary formed by the CAP is easily delineated. However, because of the very flat east-west slope along the north side of the Arizona Canal, the eastern boundary of the drainage area is less discernable. As part of the hydrology analysis for the Indian Bend Wash Interceptor Channel, the Corps of Engineers selected the Evergreen Wasteway gates, on the Arizona Canal, as a physical location for establishing an eastern watershed boundary for the Interceptor Channel design. The logic for this selection is based on Salt River Project policy to open the wasteway gates and empty the Arizona Canal at the earliest time that it can be determined that significant runoff will reach the canal. Under this scenario, it can be reasonably assumed that any floodwaters

entering the canal east of the Evergreen Wasteway would be discharged through the wasteway gates to the Salt River, therefore, being prevented from flowing further west where they might impact the design of the Outer Loop bridge over the Arizona Canal.

SLA concurs in the Corps' logic used to establish the Evergreen Wasteway as the eastern watershed boundary. Accordingly, this wasteway was also used to define the eastern boundary of the HEC-1 model used to analyze this reach of the Outer Loop Highway. A detailed discussion of the Corps' hydrology assumptions is presented in "Design Memorandum No. 4 - Feature Design for Interceptor Channel, Project Design for Indian Bend Wash", U.S. Army Corps of Engineers, January 1980.

Application of HEC-1 to an area with such diverse land uses, requires that the drainage area be further subdivided into smaller, homogeneous sub-basins with similar hydrologic properties. Accordingly, the watershed was divided into 179 sub-basins as shown on Plates 1, 2, and 3.

It should be noted that there were no current topographic maps available for this watershed other than 7.5 minute U.S.G.S. quadrangle maps. As a result, several sources of data were used to delineate drainage basin boundaries. These sources included the U.S.G.S. quadrangle maps, 1"=400' aerial photographs (June 6, 1986) obtained from Cooper Aerial Survey Co., 1"=1000' aerial photography (October 1986) provided by Landis Aerial surveys, subdivision plats obtained from the City of Scottsdale, and extensive field inspections. Representatives of the Salt River Indian Reservation provided the drainage pattern data for the agricultural fields located along the north side of the Arizona Canal, east of Alma School Road.

As a result of these numerous data sources, there may not be perfect "scale" correlation when the same sub-basin area or channel routing length is measured on two different maps. However, these type of scaling errors should have an insignificant impact on the hydrologic modeling results.

4.2 Interception/Infiltration

Precipitation losses due to interception and infiltration were modeled using the SCS curve number option in HEC-1. Selection of curve numbers was based on information gathered relative to type of soil cover, vegetation density, land

use, and soil moisture conditions. An Antecedent Moisture Condition 2 (AMC 2) was assumed for all curve number selections.

Three primary land-use categories were used for the development of curve numbers for this watershed: 1) agricultural; 2) undeveloped desert; and, 3) commercial/residential areas.

Agricultural areas were modeled with curve number recommendations taken from Table 2-2b, Urban Hydrology for Small Watersheds, Technical Release 55 (TR55), SCS, June 1986. The fields were considered to be representative of "straight row crops" and curve numbers were selected at one point below those listed for "Poor" hydrologic condition. The possibility of a fully saturated field (as a result of recent irrigation) was used to weight the curve numbers toward a "Poor" condition rather than a "Good" condition. Table 4.1 lists the agricultural field curve numbers as a function of hydrologic soil group.

The percentage of each soil group in each field was estimated on the basis of a soil map overlay onto the HEC-1 sub-basin delineation map. An area-weighting procedure was then used to determine a composite curve number that would be representative of each field.

Curve numbers for undeveloped desert areas were based on recommendations from Table 2-2d of TR55. A "Poor" hydrologic condition was assumed to represent the sparse ground cover that is typical of this area. Table 4.2 lists the baseline desert curve numbers as a function of hydrologic soil group. As with the agricultural fields, an area-weighted curve number was developed for each desert parcel to reflect the existence of multiple soil groups within a specific sub-basin boundary.

Areas of the watershed that contain commercial and residential development were modeled using a pre-development baseline curve number and a percent of impervious cover. Since these areas were part of the undeveloped desert category prior to becoming urbanized, a baseline curve number was selected for each parcel using the values in Table 4.2. Based on a review of the 1986 aerial photo (Plate 1) and the Scottsdale Ranch Master Drainage Plan, a land-use classification was selected for each sub-basin. This land-use classification was then converted to a percent of impervious cover to reflect the additional runoff that is generated as a result of urbanization. This conversion to percent of impervious

TABLE 4.1 Baseline Curve Numbers for Agricultural Fields (Straight row crops)				
Curve Number	Hydrologic Soil Group			
	A	B	C	D
	71	80	87	90

TABLE 4.2 Baseline Curve Numbers for Undeveloped Desert ("Poor" hydrologic condition)				
Curve Number	Hydrologic Soil Group			
	A	B	C	D
	63	77	85	88

cover was based on a graphical relationship developed for the General Drainage Plan for North Scottsdale, Arizona, Water Resources Associates, Inc. 1989. This relationship, which is reproduced herein as Figure 4.1, relates residential dwelling units per acre to percent of impervious surface cover. Commercial areas and apartment complexes were, with two exceptions, assigned values of 85 percent impervious cover. Figure 4.1 was developed, in part, from percents of impervious cover taken from Table 2-2a, TR55.

In conclusion, it should be noted that the curve numbers selected as a result of the previously described procedures are considered representative of a 24-hour duration storm. Any attempt to model shorter storm durations should include revisions to the curve numbers, as recommended by Woodward in Runoff Curve Numbers for Semiarid Range and Forest Conditions, 1973.

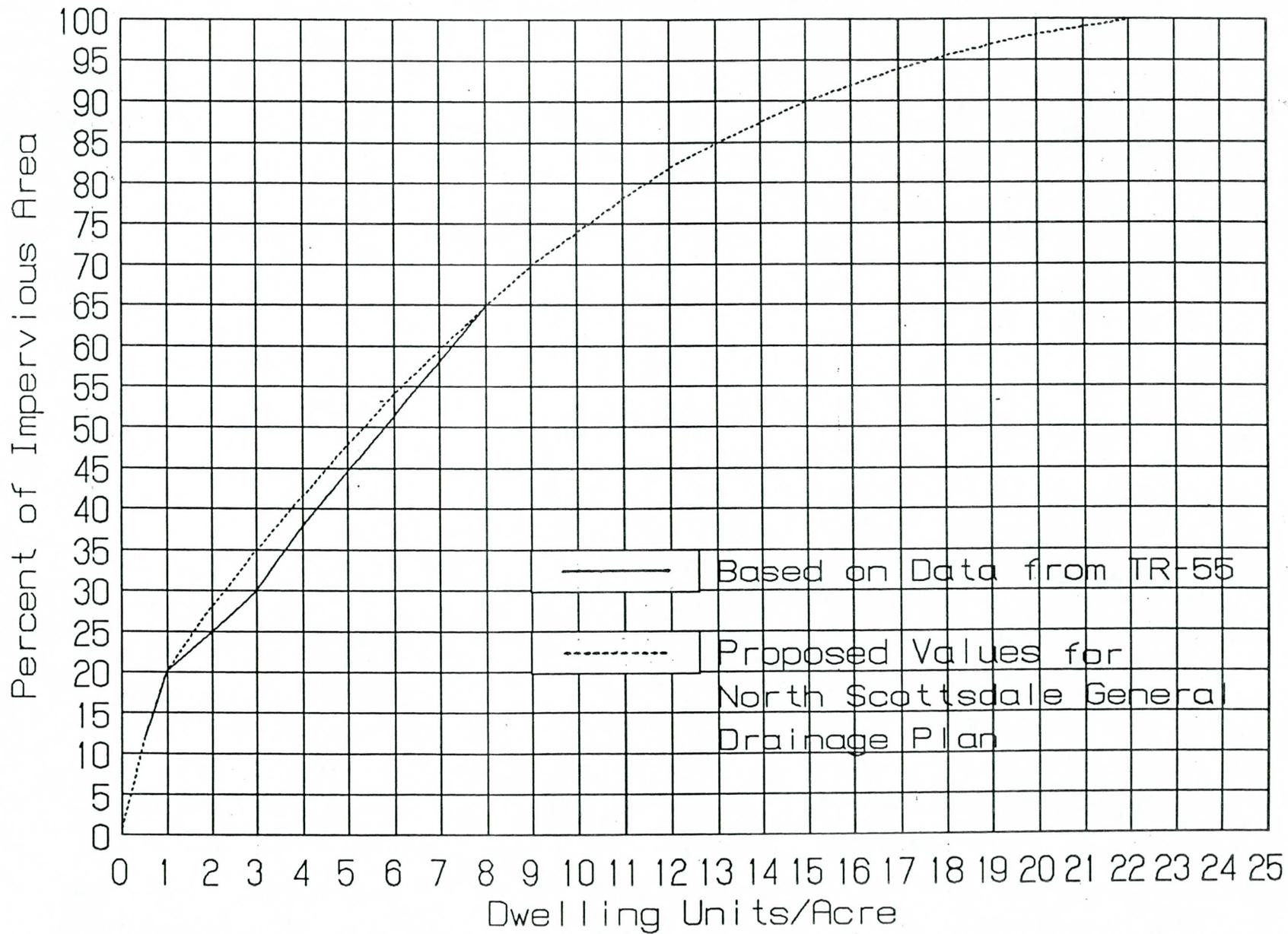
4.3 Configuration of the Hydrologic Model

The HEC-1 model developed by the Corps of Engineers Hydrologic Engineering Center, was used to simulate runoff conditions in the study area. The kinematic wave option was used to determine the hydrologic response of the sub-basin areas and for routing the resulting hydrographs through the tributary channels of the basin. This option was selected because runoff processes can be simulated using measurable geographic features such as overland flow elements and the shape, boundary roughness, length, and slope of channel elements. Unlike unit hydrograph techniques, the kinematic wave approach also provides for a non-linear response of runoff characteristics, i.e., peak discharge does not necessarily increase linearly with direct runoff when using the kinematic wave methodology.

A network of sub-basins and connecting channels was configured that simulates the natural drainage pattern in the basin. Plate 3 presents an illustration of the drainage patterns, sub-basin boundaries, and concentration points used in the model.

The following subsections describe the parameters that were used to model the physical characteristics of the watershed. Appendix A contains a complete listing of all HEC-1 input and output data for the 100-year event. Appendix B contains the output data for the 50-year event.

Figure 4.1
Percent of Impervious Area vs.
Dwelling Units/Acre



4.3.1 Kinematic Wave Parameters

The kinematic wave calculations used in HEC-1 are based on the hydraulic properties of simple geometric elements such as overland flow planes and prismatic channel cross-sections. These elements are selected and dimensioned so as to simulate the physical drainage characteristics of a specific watershed sub-basin.

For this project, overland flow planes were used to model the wide, flat shallow flow areas across the desert and urbanized areas of the watershed. These planes are described in terms of an average overland flow length, slope, and roughness coefficient. Lengths and slopes were measured from aerial photographs, 7.5-minute quadrangle maps, and the Master Drainage and Flood Control Plan for Scottsdale Ranch, while roughness values were based on recommendations presented in the HEC-1 Users Manual. Typical roughness values for desert areas were 0.1, those used in urban sub-basins were 0.09, while those used for agricultural fields were 0.15.

As one travels northeasterly through the watershed, a much more defined drainage pattern is encountered. This is in contrast to the very flat, smooth, relatively unincised land surface that is encountered west of Alma School Road. This change in drainage pattern justifies the selection of much shorter overland flow lengths for the more defined tributary pattern that exists east of Alma School Road. The selection of overland flow lengths for undeveloped desert areas was made with the use of the October 1986 aerial photograph.

For the long overland flow lengths west of Alma School Road, overland flow slopes were based on U.S.G.S. 7.5 minute quadrangle maps. Slopes for the short overland flow lengths east of Alma School Road were based on surveyed desert cross-sections taken north of the CAP. These cross-sections were previously used by SLA for the Outer Loop Hydrology Study north of the CAP (April 1987).

To establish consistency and uniformity to the overland flow slopes, the following assumptions were used:

1. Overland flow slopes for well-incised sub-basins were based on surveyed cross-sections taken on wide strips of desert north of the CAP. The average slope was found to be 0.0213 ft/ft. This slope was applied to all desert sub-basins with overland flow lengths of 145 feet.

2. Overland flow slopes for all other desert sub-basins were based on the average land slope taken from 7.5 minute quadrangle maps. The following two values were used:
 - a. East of Alma School Road - $20/2500 = 0.0080$ ft/ft.
 - b. West of Alma School Road - $20/3500 = 0.0057$ ft/ft.

The above criteria was applied to all Indian Reservation lands.

Agricultural fields were treated the same as those encountered below the Arizona Canal (see Final Hydrology Report, Outer Loop Highway, Arizona Canal to the Salt River, SLA, April 1989).

The streets and washes that collect runoff from the overland flow planes were modeled as either triangular or trapezoidal cross-sections. Data requirements for kinematic wave channel routing include reach length, channel slope, channel shape, and roughness coefficient. The dimensions and roughness values for those collector channels that were simulating subdivision streets, were determined from a comparison of the hydraulic conveyance of measured cross-sections to the conveyance of an equivalent prismatic section. This was accomplished by developing a rating curve of Depth versus Conveyance for a wide range of depths in the existing street cross-section.

Once the baseline conveyance curve was established, a trial and error procedure was employed whereby different prismatic sections were evaluated over the same range of flow depths used for the street section. The geometry and "n" value of the prismatic sections were adjusted until a cross-section was found that exhibited a similar conveyance curve to that generated for the natural baseline section. The prismatic section that most nearly duplicated the conveyance characteristics of the street section was used for the kinematic routing of flow through the subdivision.

As noted previously, the undeveloped desert areas exhibit a wide variation in channel pattern. Kinematic channel routing parameters for these areas was based largely on engineering judgement. However, this judgement was guided by hydraulic calculations to compute approximate flow velocities across the desert. An initial set of channel geometry was assigned to each desert sub-basin. The HEC-1 model was then executed and the resulting peak discharge values were combined with the assigned channel geometry and Manning's Equation to compute the flow velocity associated with these assigned conditions. These computed

velocities were then reviewed in context with the channel pattern serving each sub-basin. The flat, relatively unincised land surface west of Alma School Road would be expected to have low velocities as a result of shallow sheetflow conditions. In contrast, the more incised channel pattern east of Alma School Road would present more concentrated flow conditions with higher flow velocities. Accordingly, channel geometry west of Alma School Road was selected to produce flow velocities in the 2 to 4 fps range, while the channels east of Alma School Road were configured to produce velocities in the range of 4 to 7 fps.

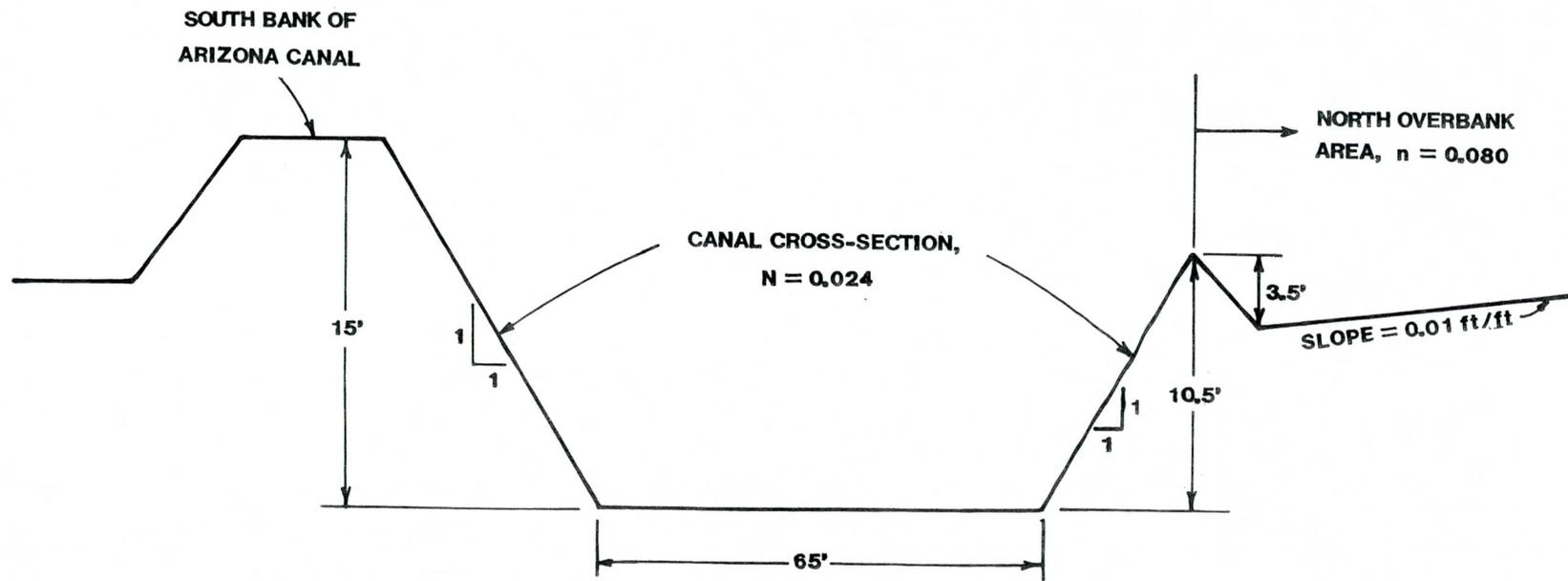
4.3.2 Channel Routing Along Arizona Canal

As previously discussed in Section 2.1, the Arizona Canal will intercept floodwaters and divert such flows to the west, where they will ultimately influence the hydraulic design of the Outer Loop Highway bridge over the Arizona Canal. For the purpose of this study, any flows intercepted by the canal east of the Evergreen Wasteway are assumed to be non-contributory to the Outer Loop intersection with the Arizona Canal (see Section 4.1 of this report).

Due to the existence of inlet locations along the north side of the canal embankment, some of the intercepted floodwaters will enter the canal and flow to the west through the canal cross-section. Some portion of the floodwaters will travel westerly along the north side of the canal embankment. The end result will be a combination of water flowing through the canal cross-section and along the roughly vegetated north canal bank.

The total length of flow diversion along the canal, from the Evergreen Wasteway to the proposed Outer Loop alignment, is approximately 4.6 miles. This length, combined with the referenced cross-section of flow, provides the potential for significant hydrograph attenuation. Accordingly, for the purpose of routing intercepted floodwaters along and through the Arizona Canal, the kinematic wave option was abandoned in favor of the "Normal Depth Storage Routing" option (Modified Puls) in HEC-1. This relatively simple option uses an 8-point cross-section (with variable "n" values) to conduct channel routing operations. Figure 4.2 illustrates the 8-point cross-section that was used for this analysis. This geometry is based on a typical cross-section described in the previously referenced Corps' report on the Indian Bend Wash Interceptor Channel analysis. As stated previously, this entire cross-section was used to

TYPICAL CROSS-SECTION USED FOR MODIFIED PULS ROUTING ALONG ARIZONA CANAL



sla SIMONS, LI & ASSOCIATES, INC.

FIGURE 4.2

route the intercepted floodwaters to the west, i.e., the canal is assumed to be drained of irrigation water (see Section 4.1).

For comparison purposes, a kinematic wave routing operation was also performed for those flows being diverted west along and through the canal. Based on similarity with the conveyance curve for the 8-point cross-section, a kinematic wave cross-section was chosen with a bottomwidth of 35 feet, 20:1 side-slopes, and a roughness value of 0.080. In order to provide a reasonable velocity match with previous Corps calculations, a bed-slope of 0.0045 feet/foot was used (i.e., this geometry and slope provide flow velocities in the 2 to 4 fps range for the majority of flows through the cross-section).

At the intersection with the Outer Loop alignment, the Modified Puls routing produced a peak 100-year discharge of 8120 cfs, while the kinematic wave option produced a peak discharge of 9803 cfs.

As a matter of technical interest, it should be noted that Modified Puls routing option produced a HEC-1 output message that the routed flows may be numerically unstable for discharges lying between 721 cfs and 2785 cfs. Such instability can be eliminated by decreasing the computation interval in the model or by increasing the routing reach length. Due to the limited number of hydrograph ordinates (300) that HEC-1 is capable of computing, it is not possible to decrease the computation interval to less than 5 minutes and still maintain the 24-hour hypothetical rainfall distribution. It is also infeasible to increase the routing reach lengths because of fixed concentration points used to combine flows in the model.

The routed hydrograph at Concentration Point 1986 was examined for oscillations and none were found. Accordingly, it is not believed that numerical instability is a major contributor to errors in the model.

4.3.3 Model Assumptions for Scottsdale Ranch Lake System

As discussed previously, the lake on Scottsdale Ranch is capable of providing a significant amount of flood control storage. In order to model the impact that this storage will have on downstream hydrographs, it was necessary to include the outflow hydrograph from the lake in the HEC-1 model. This outflow hydrograph is assumed to represent the total runoff contribution from all the

contributing drainage area upstream of the lake system (including cross-drainage at the CAP).

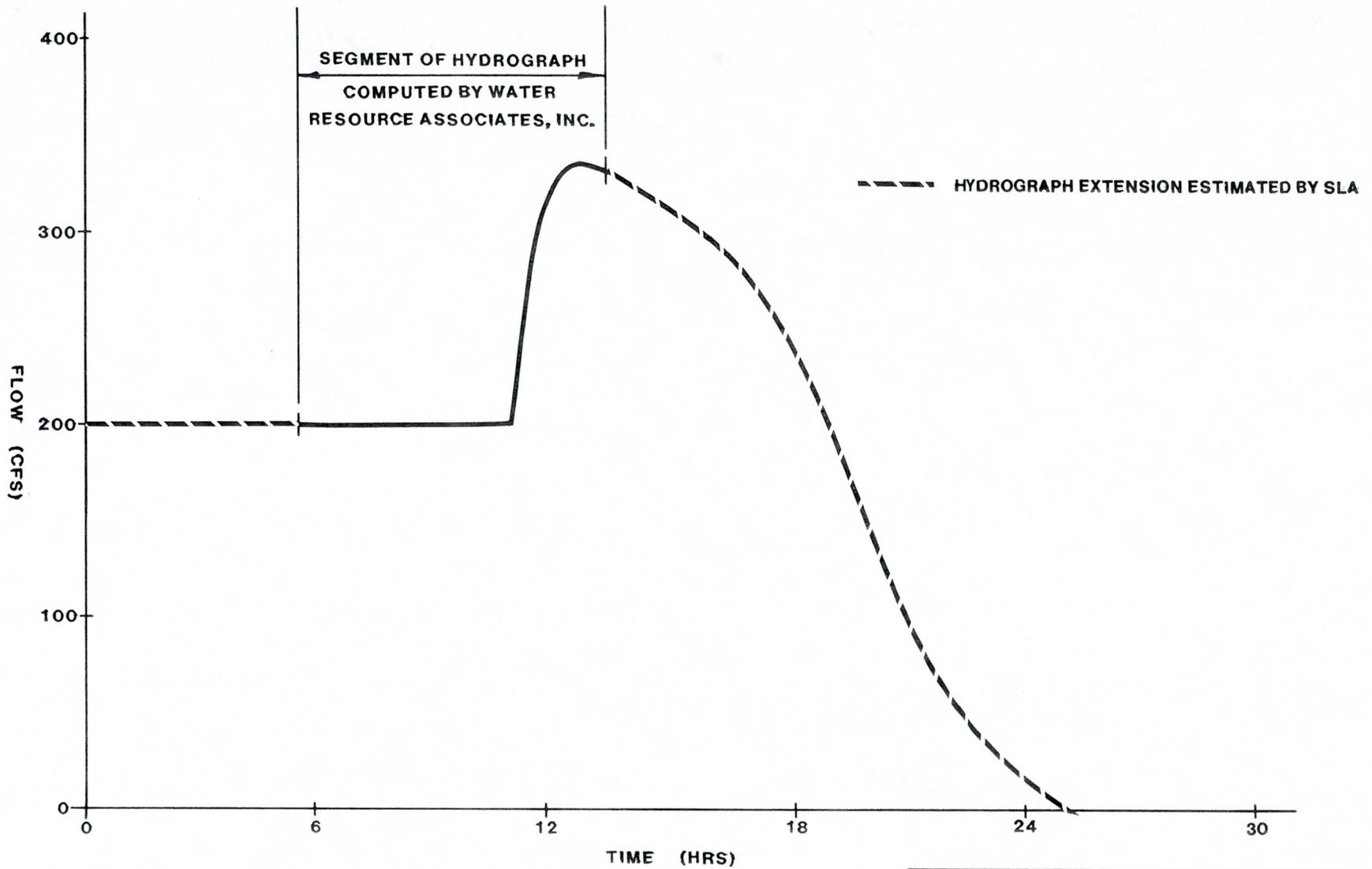
The outflow hydrograph that was used for this analysis was taken from Table 4 - Scottsdale Ranch, Detention Basin Operation Schedule for the 100-Year Frequency Flood, USCE, by Water Resources Associates, Inc., September 1979. This table was part of a report entitled Scottsdale Ranch, Offsite and Onsite Hydrology, prepared by Collar, Williams & White Engineering, Inc. This table only lists discharge ordinates for 32-15 minute time periods (total time base of 8-hours). Discussions with a former employee of Water Resources Associates were conducted in order to determine where the remainder of the hydrograph might be. The consensus of this discussion indicated the complete hydrograph was probably never computed since the peak outflow was all that was required. There was also no absolute confirmation as to what storm duration, or where within the storm duration, the hydrograph should be referenced.

For the purpose of this hydrology study, it was assumed that the outflow hydrograph was based on a 100-year, 24-hour storm and that the outflow hydrograph would be positioned within this storm so that its peak outflow would coincide with the peak rainfall intensity from the HEC-1 hypothetical 24-hour storm. Figure 4.3 is a graphical plot of this hydrograph as it was input into the HEC-1 model used for this study. This figure indicates the portion of the hydrograph that was taken from the Water Resources Associates study and the portion that was, judgementally, added by SLA. This hydrograph was input so that its outflow would be combined with the runoff from Sub-basin 300.

4.3.4 Model Assumptions for Onsite Detention/Retention

As was discussed in Section 2.1 of this report, there is a considerable amount of onsite retention/detention storage in the urbanized portions of the watershed. The attenuation impact of this storage on runoff hydrographs was simulated by inserting reservoir routing operations into the HEC-1 model. The storage volume assumptions used for these reservoir simulations are listed as follows:

1. Parcels on Scottsdale Ranch (east of 96th Street) - The Master Drainage and Flood Control Plan, August 1983, Collar, Williams, &



sla SIMONS, LI & ASSOCIATES, INC.

OUTFLOW HYDROGRAPH FROM
SCOTTSDALE RANCH LAKE

FIGURE 4.3

White Engineering, Inc. lists specific detention basin sites and basin volumes for eight locations. These locations are identified as DAM 1 through DAM 8 on Plate 3 and in the HEC-1 model. This information was used to develop the required storage relationship for a reservoir routing operation at each of these eight sites. An emergency spillway elevation and top of dam elevation was assigned to the dam elevation at which the allotted storage was obtained. Additional inflows to the reservoir (after the available storage was exceeded) were then routed over the top of the dam as weir flow.

2. Parcels west of 96th Street - Numerous discussions with representatives from the City of Scottsdale and many searches through available records failed to produce the drainage data needed for this area. Substantial onsite detention has been verified through field inspections conducted by SLA staff. Discussions with City staff indicated those parcels developed prior to 1984 should have onsite storage equivalent to the runoff from a 50-year, 24-hour storm. Parcels developed after 1984 were required to store the difference in runoff between pre- and post-development conditions, based on a 100-year, 1-hour storm. In the absence of having documented hydrology reports for these parcels, it was assumed that all parcels west of 96th Street were developed on the basis of providing detention storage for the increase in runoff between pre- and post-project conditions using the 100-year, 1-hour rainfall.

To provide a safety factor for the uncertainty that the required storage was actually provided as part of the development, only 50 percent of the incremental runoff from the 100-year, 1-hour rainfall was used to develop the storage/volume relationship used in the reservoir routing operation for the Outer Loop HEC-1 model. All runoff volume calculations were based on the existing or forecast land-uses shown on Plate 1 and the Scottsdale Ranch Master Plan.

The runoff volume calculations resulting from this procedure were used to develop a storage relationship for the reservoir routing operation used to simulate the detention basin performance. As with the Scottsdale Ranch parcels, the spillway and top of dam elevations

were set to coincide with the dam elevation at which the allotted detention storage was exhausted. Any inflows beyond this point were allowed to spill over the dam and continue to downstream areas of the watershed.

When reviewing the HEC-1 output data, these detention basins are identified by a "DAM" prefix in front of the sub-basin number.

4.4 Hydrologic Modeling Results

Table 4.3 presents the results of the HEC-1 modeling that was accomplished to predict peak discharge values for the design of an interceptor channel along the proposed Outer Loop Highway alignment.

To avoid confusion when reviewing Table 4.3, a clarification is warranted relative to Concentration Points 545, 546, and 547. Concentration Point 545 represents the peak discharge that would be expected at the Arizona Canal in response to water flowing south through an interceptor channel located along the east side of the proposed Outer Loop Highway. Concentration Point 546 represents the peak discharge that would occur at the highway in response to water flowing west, through and along the north side of the Arizona Canal, while CP 547 represents the peak discharge that would occur at the Arizona Canal and Outer Loop intersection through the combining of flows from the north/south interceptor channel (CP 545) with westerly flowing water through and along the north bank of the Arizona Canal (CP 546).

When reviewing the data in Table 4.3, it should be noted that certain limitations are associated with the peak discharge values for the 50-year event. These limitations result from the following factors:

1. No outflow hydrograph from the Scottsdale Ranch lake system was available for any event other than the 100-year storm. Accordingly, the 100-year outflow hydrograph was used for both the 50- and 100-year HEC-1 models for the Outer Loop.
2. The peak outflows from the CAP cross-drainage structures were obtained from the Bureau of Reclamation. The Bureau had no data on how these discharges might fluctuate with lesser storm events. As

a result, the same peak discharge was used for both 50- and 100-year events.

These limitations will provide some additional conservatism in the peak discharge values used for the 50-year event, i.e., the 50-year peaks along the Outer Loop may be slightly higher than normal since they reflect some 100-year inflows along the perimeter of the watershed.

4.5 Verification of Model Results

In order to establish confidence in the results from the HEC-1 model, it is important to utilize an independent procedure to calculate peak discharge values that can be compared to the computerized modeling results. Due to the extreme differences in land-use throughout the study area, several different techniques were used for model verification.

As will be shown in the following subsections of this report, the HEC-1 model is considered to be producing reasonable results from the various sub-basins comprising the watershed. The overall model results are also considered reasonable as a result of comparing peak discharge values at the intersection of the Outer Loop and Arizona Canal with those developed by the Corps of Engineers for the intersection of Pima Road and the Arizona Canal. The peak 100-year Corps' discharge at Pima Road was 7824 cfs (rounded to 8000 cfs for design purposes), while the peak 100-year discharge at the Outer Loop/Arizona Canal intersection generated from this study is 9326 cfs.

It should be noted that the Corps used a considerably different approach in developing their hydrology, i.e., they used a 7-hour storm distribution with direct runoff values computed as 45 percent of the Standard Project Flood values. The Corps also used the exponential loss rate function, rather than the SCS curve number, to simulate soil infiltration losses.

TABLE 4.3 Peak Discharge Summary Along Outer Loop Alignment		
Concentration Point	^{1/} Q100 (cfs)	^{2/} Q50 (cfs)
(North-south Interceptor Channel)		
103	514	428
360	507	433
410	549	457
422	755	618
402	997	788
501	996	782
503	1314	971
560	1378	1023
552	3234	2453
545	3239	2485
(Arizona Canal Bridge)		
^{3/} 546	8120	6417
^{4/} 547	9326	7449

^{1/} Model 2FD6.24I

^{2/} Model 2FD6.5I

^{3/} Peak discharge due to flows being routed west through the Arizona Canal.

^{4/} Combined peak discharge from flows routed through the north-south interceptor channel and flows routed west through the Arizona Canal, i.e., this combines the hydrograph from SUB 545 with that from CP 546.

4.5.1 Agricultural Fields

Model verification calculations were previously performed for this land-use category as part of the Final Outer Loop Hydrology Report for that study reach extending from the Arizona Canal to the Salt River (SLA, April 1989). Since the field dimensions and irrigation layout is the same for those fields north of the canal as for those south of the canal, identical kinematic wave modeling parameters were used in both cases. As a result, no additional verification calculations are required.

4.5.2 Undeveloped Desert

Due to the large size of the project study area, it is not practical to perform verification calculations for every concentration point in the HEC-1 model. Instead, a small number of "typical" sub-basins were selected from each land-use category to use in the verification procedure.

Sub-basins 440 and 1165 were selected as being typical of the undeveloped desert areas. Sub-basin 440 is representative of the smooth, relatively unincised desert region that lies west of Alma School Road, while Sub-basin 1165 is typical of the more incised, braided channel pattern that lies east of Alma School Road.

The following three procedures were used to verify the model results from those desert regions:

1. Peak discharge regression equations presented in Estimation of Magnitude and Frequency of Floods in Pima County, Arizona, With Comparisons of Alternative Methods, U.S.G.S. Water Resources Investigations report 84-4142, Table 2, J.H. Eychaner, August 1984.
2. Graphical peak discharge method presented in Urban Hydrology for Small Watersheds, Technical Release 55, Soil Conservation Service, USDA, June 1986.
3. Peak discharge methodology presented in Hydrology Manual for Engineering Design and Floodplain Management Within Pima County, Arizona, Pima County Department of Transportation and Flood Control District, September 1979.

Although the regression equations developed under Procedure 1 were based primarily on stream gage data in and around Pima County, their use in the Scottsdale area is justified on the basis of similar watershed characteristics in both areas.

Procedure 2 (TR55) is based on an SCS Type II rainfall distribution and uses a time of concentration that considers sheet flow, shallow concentrated flow, and open channel flow. Where applicable, the same overland flow and channel routing parameters that were used in the HEC-1 model were used in this procedure. The same SCS curve numbers were also used in the TR55 procedure as were used in the HEC-1 model.

Procedure 3 is a semi-empirical, peak discharge equation that acknowledges such watershed characteristics as watercourse length, mean slope, basin roughness, length to center of gravity, drainage area size, and infiltration rate (SCS curve number). Although this procedure was developed in Pima County, it is based on physical watershed characteristics that allow it to be used in any semi-arid environment. It should be noted, however, that the procedure is limited to individual sub-basins whose times of concentration are less than three hours. Since this procedure is based on short duration storms, all SCS curve numbers used for this method were taken from the Curve Number chart on page 107 of the Pima County Hydrology Manual.

Table 4.4 presents a summary of the independent peak discharge calculations that were performed for each of the two desert sub-basins. For comparison purposes, the peak discharge values from the HEC-1 model (using the 24-hour, hypothetical rainfall distribution) are also listed in this table.

Although Table 4.4 indicates considerable variation in the discharge values generated by the various methods, it is concluded that HEC-1 is providing results that are generally reasonable. The high value produced by the Eychaner regression equations for Sub-basin 440 is attributed to the fact that these

TABLE 4.4 Summary of Verification Calculations for HEC-1 Model					
Undeveloped Desert Parcels					
Sub-Basin	D.A. (sq.mi.)	Q100 (cfs)			
		USGS Regression (Eychaner)	TR55	Pima County Peak	HEC-1
440	0.1004	236	50	135	92
1165	0.3942	645	404	583	787

riverine based equations are not accurately simulating the shallow sheetflow conditions that exist in this sub-basin.

4.5.3 Urbanized Land

Sub-basins 255 and 270 were selected as typical basins to use for the verification of peak discharge from the urbanized parcels lying north of the Salt River Indian Reservation Boundary. Urban runoff based regression equations were used as the primary verification method for these basins. These equations, which include a "Basin Development Factor" to account for urbanization effects, are presented in U.S.G.S. Water Supply Paper 2207, Flood Characteristics of Urban Watersheds in the United States, Sauer 1983.

Due to the small size of these two typical sub-basins, the Rational Equation was also employed to provide a second estimate of peak discharge from urbanized basins. As a result of the sensitivity of this procedure to time of concentration (T_c), this parameter was computed using procedures in TR55, as well

as with the Kirpich equation. Peak discharge values using both Tc procedures are presented in Table 4.5.

A baseline runoff coefficient of 0.68 was used for both sub-basins. This value was increased by a factor of 1.25 to reflect the more severe runoff characteristics associated with a 100-year event. This adjustment produced a final runoff coefficient of 0.85.

A review of Table 4.5 indicates reasonable correlation between the HEC-1 results and those from the independent calculations. Accordingly, the HEC-1 modeling parameters used for the urban sub-basins are considered to be giving satisfactory results. It should be emphasized that the results listed in Table 4.5 do not reflect any impacts due to onsite detention storage.

TABLE 4.5 Summary of Verification Calculations for HEC-1 Model				
Urbanized Parcels				
Sub-Basin	D.A. (sq. mi.)	Q100 (cfs)		
		USGS WSP 2207 (Sauer)	Rational Method (variable Tc)	HEC-1
255	0.0290	112	54 ^{1/} 77 ^{2/}	78
270	0.0576	198	124 ^{3/} 160 ^{4/}	173

- ^{1/} Tc = 36 minutes
^{2/} Tc = 20.2 minutes
^{3/} Tc = 29.4 minutes
^{4/} Tc = 18.9 minutes

V. CONCLUSIONS AND RECOMMENDATIONS

This report presents a technical overview of the engineering parameters that were used to create a computerized rainfall/runoff model of the offsite drainage intercepted by that reach of the Outer Loop Highway extending from the Camelback Walk Channel to the Arizona Canal. The model simulates the runoff response that would be associated with both the 100- and 50-year, 24-hour precipitation applied to the 24-hour hypothetical rainfall distribution generated by HEC-1. When compared to peak discharge estimates generated from independent calculation methods, as well as previous Corps of Engineers studies, the model results were judged to be realistic.

Should future development occur in this watershed that would alter the existing drainage pattern used to create the routing structure of the model, the inflow points to the proposed highway interceptor channel could be significantly altered. Depending on how such alterations might occur, the proposed channel capacity might be subjected to either an under- or over-design.

It is also important to emphasize that any future land-use changes that might alter this watershed towards a more urbanized condition will undoubtedly generate a potential for increased runoff, as farmland and undeveloped desert areas are covered with more impervious surfaces such as asphalt streets, parking lots, rooftops, etc. If such changes are ever allowed to occur, it is important that effective drainage ordinances be enforced to insure that peak discharges are not increased along the Outer Loop Highway alignment.

In summary, SLA recommends that the HEC-1 model presented in this report be adopted for use in the concept design of offsite drainage structures for this reach of the Outer Loop Highway. The model was constructed in such a way that hydrology data is available for both the design of a north-south interceptor channel along the east side of the highway, and for the hydraulic design of a bridge structure to cross the Arizona Canal. Both 100- and 50-year storm frequencies should be considered for engineering and economic comparisons.

APPENDIX A

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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1 ID *****
2 ID *
3 ID * SIMONS, LI AND ASSOCIATES, INC. *
4 ID *
5 ID * OUTER LOOP FREEWAY *
6 ID * (NORTH OF THE ARIZONA CANAL) *
7 ID * PHASE 2 *
8 ID *
9 ID * HYDROLOGY ANALYSIS FOR OFFSITE DRAINAGE *
10 ID * 100 YEAR EVENT *
11 ID * 24 HOUR HYPOTHETICAL STORM DISTRIBUTION *
12 ID *
13 ID * USES % IMPERVIOUS COVER TO SIMULATE DEVELOPMENT *
14 ID * INCLUDES RETENTION/DETENTION BASINS FOR *
15 ID * RESIDENTIAL & COMMERCIAL AREAS *
16 ID *
17 ID *
18 ID * THIS MODEL INCLUDES THE ADDITIONAL AREA *
19 ID * PREVIOUSLY MODELED BY THE CORP OF ENGINEERS *
20 ID * N=.012 FOR THE HIGHWAY INTERCEPTOR CHANNEL *
21 ID * ALL CURVE NUMBERS ARE FOR 24-HOUR STORM DURATION *
22 ID * NORMAL DEPTH STORAGE ROUTING ALONG ARIZONA CANAL *
23 ID * INCLUDES CAP CROSS-DRAINAGE AT EAST BOUNDARY *
24 ID * USES APRIL 1989 HYDROLOGIC REVISIONS BY R. WARD *
25 ID * MARCH 1989 HIGHWAY ALIGNMENT *
26 ID *
27 ID * MODEL 2FD6.24I *
28 ID *****

```

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29 *DIAGRAM
IT 5 20APR89 0 289
30 IO 5

31 KK 1270 SUB
32 KM RUNOFF FROM SUB 1270
33 BA .0308
34 PH .1 .72 1.42 2.50 2.77 2.95 3.29 3.68 4.08
35 LS 80
36 UK 1300 .0028 .15 100
37 RK 660 .0029 .045 TRAP 10 100

38 KK 1280 SUB
39 KM RUNOFF FROM SUB 1280 AND ROUTE SUB 1270
40 BA .0308
41 LS 80
42 UK 1300 .0028 .15 100
43 RK 660 .0029 .045 TRAP 10 100 YES

44 KK 1350 SUB
45 KM RUNOFF FROM SUB 1350 AND ROUTE SUB 1280
46 BA .0308
47 LS 80
48 UK 1300 .0028 .15 100
49 RK 660 .0029 .045 TRAP 10 100 YES

```

LINE	ID	1	2	3	4	5	6	7	8	9	10
50	KK	1360									
51	KM										
52	BA	.0308									
53	LS		80								
54	UK	1300	.0028	.15	100						
55	RK	660	.0029	.045			TRAP	10	100	YES	
56	KK	1440									
57	KM										
58	BA	.0308									
59	LS		80								
60	UK	1300	.0028	.15	100						
61	RK	660	.0029	.045			TRAP	10	100	YES	
62	KK	1450									
63	KM										
64	BA	.0308									
65	LS		80								
66	UK	1300	.0028	.15	100						
67	RK	660	.0029	.045			TRAP	10	100	YES	
68	KK	1520									
69	KM										
70	BA	.0308									
71	LS		80								
72	UK	1300	.0028	.15	100						
73	RK	660	.0029	.045			TRAP	10	100	YES	
74	KK	1530									
75	KM										
76	BA	.0308									
77	LS		80								
78	UK	1300	.0028	.15	100						
79	RK	660	.0029	.045			TRAP	10	100	YES	
80	KK	1600									
81	KM										
82	BA	.0616									
83	LS		80								
84	UK	1300	.0028	.15	100						
85	RK	1320	.0029	.045			TRAP	10	100	YES	
86	KK	1591									
87	KM										
88	RK	1360	.0028	.045			TRAP	10	100		
89	KK	1260									
90	KM										
91	BA	.0320									
92	LS		80								
93	UK	1350	.0028	.15	100						
94	RK	660	.0029	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
95	KK	1290									
96	KM										
97	BA	.0320									
98	LS		80								
99	UK	1350	.0028	.15	100						
100	RK	660	.0029	.045			TRAP	10	100	YES	
101	KK	1340									
102	KM										
103	BA	.0320									
104	LS		80								
105	UK	1350	.0028	.15	100						
106	RK	660	.0029	.045			TRAP	10	100	YES	
107	KK	1370									
108	KM										
109	BA	.0320									
110	LS		80								
111	UK	1350	.0028	.15	100						
112	RK	660	.0029	.045			TRAP	10	100	YES	
113	KK	1430									
114	KM										
115	BA	.0320									
116	LS		80								
117	UK	1350	.0028	.15	100						
118	RK	660	.0029	.045			TRAP	10	100	YES	
119	KK	1460									
120	KM										
121	BA	.0320									
122	LS		80								
123	UK	1350	.0028	.15	100						
124	RK	660	.0029	.045			TRAP	10	100	YES	
125	KK	1510									
126	KM										
127	BA	.0320									
128	LS		80								
129	UK	1350	.0028	.15	100						
130	RK	660	.0029	.045			TRAP	10	100	YES	
131	KK	1540									
132	KM										
133	BA	.0320									
134	LS		80								
135	UK	1350	.0028	.15	100						
136	RK	660	.0029	.045			TRAP	10	100	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
137	KK	1590									
138	KM										
139	BA	.0639									
140	LS		80								
141	UK	1350	.0028	.15	100						
142	RK	1320	.0029	.045			TRAP	10	100	YES	
143	KK	1592									
144	KM										
145	HC	2									
146	KK	1581									
147	KM										
148	RK	1280	0.0028	.045			TRAP	10	100		
149	KK	1250									
150	KM										
151	BA	.0303									
152	LS		80								
153	UK	1280	.0028	.15	100						
154	RK	660	.0029	.045			TRAP	10	100		
155	KK	1300									
156	KM										
157	BA	.0303									
158	LS		80								
159	UK	1280	.0028	.15	100						
160	RK	660	.0029	.045			TRAP	10	100	YES	
161	KK	1330									
162	KM										
163	BA	.0303									
164	LS		80								
165	UK	1280	.0028	.15	100						
166	RK	660	.0029	.045			TRAP	10	100	YES	
167	KK	1380									
168	KM										
169	BA	.0303									
170	LS		80								
171	UK	1280	.0028	.15	100						
172	RK	660	.0029	.045			TRAP	10	100	YES	
173	KK	1420									
174	KM										
175	BA	.0303									
176	LS		80								
177	UK	1280	.0028	.15	100						
178	RK	660	.0029	.045			TRAP	10	100	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
179	KK	1470	SUB								
180	KM	RUNOFF FROM SUB 1470 AND ROUTE SUB 1420									
181	BA	.0303									
182	LS	80									
183	UK	1280	.0028	.15	100						
184	RK	660	.0029	.045		TRAP	10	100	YES		
185	KK	1500	SUB								
186	KM	RUNOFF FROM SUB 1500 AND ROUTE SUB 1470									
187	BA	.0303									
188	LS	80									
189	UK	1280	.0028	.15	100						
190	RK	660	.0029	.045		TRAP	10	100	YES		
191	KK	1550	SUB								
192	KM	RUNOFF FROM SUB 1550 AND ROUTE SUB 1500									
193	BA	.0303									
194	LS	80									
195	UK	1280	.0028	.15	100						
196	RK	660	.0029	.045		TRAP	10	100	YES		
197	KK	1580	SUB								
198	KM	RUNOFF FROM SUB 1580 AND ROUTE SUB 1550									
199	BA	.0606									
200	LS	80									
201	UK	1280	.0028	.15	100						
202	RK	660	.0029	.045		TRAP	10	100	YES		
203	KK	1582	CP								
204	KM	COMBINE SUB 1580 WITH CP 1581									
205	HC	2									
206	KK	1571	CP								
207	KM	ROUTE CP 1582 TO CP 1571									
208	RK	1330	0.0028	.045		TRAP	10	100			
209	KK	1240	SUB								
210	KM	RUNOFF FROM SUB 1240									
211	BA	.0315									
212	LS	80									
213	UK	1330	.0028	.15	100						
214	RK	660	.0029	.024		TRAP	5	10			
215	KK	1310	SUB								
216	KM	RUNOFF FROM SUB 1310 AND ROUTE SUB 1240									
217	BA	.0315									
218	LS	80									
219	UK	1330	.0028	.15	100						
220	RK	660	.0029	.024		TRAP	5	10	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
221	KK	1320		SUB							
222	KM			RUNOFF FROM SUB 1320 AND ROUTE SUB 1310							
223	BA	.0306									
224	LS		81								
225	UK	1330	.0028	.15	100						
226	RK	660	.0029	.024		TRAP	5	10	YES		
227	KK	1400		SUB							
228	KM			RUNOFF FROM SUB 1400 AND ROUTE SUB 1320							
229	BA	.0080									
230	LS		84								
231	UK	300	.0028	.15	100						
232	RK	700	.0029	.024		TRAP	5	10	YES		
233	KK	1390		SUB							
234	KM			RUNOFF FROM SUB 1390							
235	BA	.0239									
236	LS		80								
237	UK	1020	.0028	.15	100						
238	RK	660	.0029	.045		TRAP	10	100			
239	KK	1391		CP							
240	KM			ROUTE SUB 1390 TO CP 1391							
241	RK	300	0.0028	.045		TRAP	10	100			
242	KK	1392		CP							
243	KM			COMBINE SUB 1400 AND CP 1391							
244	HC	2									
245	KK	1410		SUB							
246	KM			RUNOFF FROM SUB 1410 AND ROUTE CP 1392							
247	BA	.0315									
248	LS		80								
249	UK	1330	.0028	.15	100						
250	RK	660	.0029	.024		TRAP	5	10	YES		
251	KK	1480		SUB							
252	KM			RUNOFF FROM SUB 1480 AND ROUTE SUB 1410							
253	BA	.0315									
254	LS		80								
255	UK	1330	.0028	.15	100						
256	RK	660	.0029	.024		TRAP	5	10	YES		
257	KK	1490		SUB							
258	KM			RUNOFF FROM SUB 1490 AND ROUTE SUB 1480							
259	BA	.0315									
260	LS		80								
261	UK	1330	.0028	.15	100						
262	RK	660	.0029	.024		TRAP	5	10	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
263	KK	1560		SUB							
264	KM			RUNOFF FROM SUB 1560 AND ROUTE SUB 1490							
265	BA	.0315									
266	LS		80								
267	UK	1330	.0028	.15	100						
268	RK	660	.0029	.024		TRAP	5	10	YES		
269	KK	1570		SUB							
270	KM			RUNOFF FROM SUB 1570 AND ROUTE SUB 1560							
271	BA	.0630									
272	LS		80								
273	UK	1330	.0028	.15	100						
274	RK	1320	.0029	.024		TRAP	5	10	YES		
275	KK	1572		CP							
276	KM			COMBINE CP 1572 WITH CP 1571							
277	HC	2									
278	KK	1611		CP							
279	KM			ROUTE CP 1572 TO CP 1611							
280	RK	1330	0.0029	.024		TRAP	5	10			
281	KK	1610		SUB							
282	KM			RUNOFF FROM SUB 1610							
283	BA	.2505									
284	LS		80								
285	UK	1330	.0029	.15	100						
286	RK	5250	.0028	.045		TRAP	10	100			
287	KK	1612		CP							
288	KM			COMBINE SUB 1610 AND CP 1611							
289	HC	2									
290	KK	1621		CP							
291	KM			ROUTE CP 1612 TO CP 1621							
292	RK	1350	0.0029	.024		TRAP	5	10			
293	KK	1620		SUB							
294	KM			RUNOFF FROM SUB 1620							
295	BA	.2542									
296	LS		82								
297	UK	1350	.0029	.15	100						
298	RK	5250	.0028	.045		TRAP	10	100			
299	KK	1630		SUB							
300	KM			RUNOFF FROM SUB 1630							
301	BA	.1107									
302	LS		84								
303	UK	1000	.0029	.15	100						
304	RK	2900	.0028	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
305	KK	1640									
306	KM										
307	BA	.0245									
308	LS		87								
309	UK	470	.0029	.15	100						
310	RK	2100	.0028	.045			TRAP	10	100	YES	
311	KK	1622									
312	KM										
313	HC	3									
314	KK	1887									
315	KM										
316	RS	1	FLOW	-1							
317	RC	.050	.024	.080	10500	.0007					
318	RX	0	50	100	115	180	190.5	194	544		
319	RY	400	400	400	385	385	395.5	392	395.5		
320	KK	1950									
321	KM										
322	BA	.0407									
323	LS		80								
324	UK	960	.0029	.15	100						
325	RK	1900	.0028	.045			TRAP	10	100		
326	KK	1945									
327	KM										
328	BA	.0822									
329	LS		82								
330	UK	1340	.0029	.15	100						
331	RK	1710	.0028	.045			TRAP	10	100		
332	KK	1951									
333	KM										
334	RK	500	0.0029	.045			TRAP	10	100		
335	KK	1952									
336	KM										
337	HC	2									
338	KK	1940									
339	KM										
340	BA	.1143									
341	LS		87								
342	UK	1200	.0029	.15	100						
343	RK	2770	.0028	.045			TRAP	10	100	YES	
344	KK	1920									
345	KM										
346	BA	.1959									
347	LS		86								
348	UK	1300	.0029	.15	100						
349	RK	4200	.0028	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
392	KK	1885									
393	KM										
394	BA	.0657									
395	LS		87								
396	UK	1100	.0029	.15	100						
397	RK	2400	.0028	.045			TRAP	10	100	YES	
398	KK	1850									
399	KM										
400	BA	.1392									
401	LS		86								
402	UK	1320	.0029	.15	100						
403	RK	2940	.0028	.045			TRAP	10	100		
404	KK	1860									
405	KM										
406	BA	.1170									
407	LS		87								
408	UK	1320	.0029	.15	100						
409	RK	2470	.0028	.045			TRAP	10	100	YES	
410	KK	1870									
411	KM										
412	BA	.1847									
413	LS		87								
414	UK	1320	.0029	.15	100						
415	RK	3900	.0028	.045			TRAP	10	100	YES	
416	KK	1886									
417	KM										
418	RK	300	0.0029	.045			TRAP	10	100		
419	KK	1880									
420	KM										
421	BA	.0546									
422	LS		87								
423	UK	1200	.0029	.15	100						
424	RK	1700	.0028	.045			TRAP	10	100		
425	KK	1888									
426	KM										
427	HC	4									
428	KK	1984									
429	KM										
430	RS	1	FLOW	-1							
431	RC	.050	.024	.080	1850	.0007					
432	RX	0	50	100	115	180	190.5	194	544		
433	RY	400	400	400	385	385	395.5	392	395.5		

LINE	ID	1	2	3	4	5	6	7	8	9	10
434	KK	1840		SUB							
435	KM			RUNOFF FROM SUB 1840							
436	BA	.0315									
437	LS			86							
438	UK	1630	.0029	.15	100						
439	RK	2550	.0028	.024			TRAP	5	1		
440	KK	1670		SUB							
441	KM			RUNOFF FROM SUB 1670							
442	BA	.0759									
443	LS			80							
444	UK	830	.0029	.15	100						
445	RK	2550	.0028	.045			TRAP	10	100		
446	KK	1681		CP							
447	KM			ROUTE SUB 1670 TO CP 1681							
448	RK	830	0.0029	.045			TRAP	10	100		
449	KK	1680		SUB							
450	KM			RUNOFF FROM SUB 1680							
451	BA	.0759									
452	LS			80							
453	UK	830	.0029	.15	100						
454	RK	2550	.0028	.045			TRAP	10	100		
455	KK	1682		CP							
456	KM			COMBINE SUB 1680 AND CP 1681							
457	HC	2									
458	KK	1751		CP							
459	KM			ROUTE CP 1682 TO 1751							
460	RK	860	0.0029	.024			TRAP	10	100		
461	KK	1226		CP							
462	KM			DISCHARGE FROM 2-66" CMP OVER THE CAP AT STA 332+75							
463	IN	60	20APR89								
464	BA	.00001									
465	QI	0	0	0	0	0	0	0	0	40	100
466	QI	200	300	437	437	437	300	250	200	150	100
467	QI	70	40	27	13	0					
468	KK	1227		DIV							
469	KM			DIVERT CROSS-DRAINAGE FROM CP 1226 OUT OF OUTER LOOP MODEL							
470	DT	1228									
471	DI	0	100	1000							
472	DQ	0	85	850							
473	KK	1225		SUB							
474	KM			RUNOFF FROM SUB 1225 & ROUTE NON-DIVERTED FLOW FROM DIV 1227							
475	BA	.0778									
476	LS			77							
477	UK	145	.0213	.10	100						
478	RK	3200	.0119	.045			TRAP	10	5	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
521	KK	1761	CP								
522	KM	ROUTE CP 1752 TO CP 1761									
523	RK	550	0.0029	.024			TRAP	10	100		
524	KK	1760	SUB								
525	KM	RUNOFF FROM SUB 1760									
526	BA	.0493									
527	LS	80									
528	UK	550	.0029	.15	100						
529	RK	2550	.0028	.045			TRAP	10	100		
530	KK	1762	CP								
531	KM	COMBINE SUB 1760 AND CP 1761									
532	HC	2									
533	KK	1771	CP								
534	KM	ROUTE CP 1762 TO CP 1771									
535	RK	400	0.0029	.045			TRAP	10	100		
536	KK	1770	SUB								
537	KM	RUNOFF FROM SUB 1770									
538	BA	.0359									
539	LS	80									
540	UK	400	.0029	.15	100						
541	RK	2550	.0028	.045			TRAP	10	100		
542	KK	1772	CP								
543	KM	COMBINE SUB 1770 AND CP 1771									
544	HC	2									
545	KK	1841	CP								
546	KM	ROUTE CP 1772 TO CP 1841									
547	RK	1600	0.0029	.045			TRAP	10	100		
548	KK	1842	CP								
549	KM	COMBINE CP 1841 AND SUB 1840									
550	HC	2									
551	KK	1830	SUB								
552	KM	RUNOFF FROM SUB 1830 AND ROUTE CP 1842									
553	BA	.1134									
554	LS	87									
555	UK	1230	.0029	.15	100						
556	RK	2570	.0028	.024			TRAP	5	1	YES	
557	KK	1660	SUB								
558	KM	RUNOFF FROM SUB 1660									
559	BA	.0903									
560	LS	80									
561	UK	980	.0029	.15	100						
562	RK	2570	.0028	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
563	KK	1691		CP							
564	KM			ROUTE SUB 1660 TO CP 1691							
565	RK	980	0.0029	.045			TRAP	10		100	
566	KK	1690		SUB							
567	KM			RUNOFF FROM SUB 1690							
568	BA	.0922									
569	LS			80							
570	UK	980	.0029	.15		100					
571	RK	2570	.0028	.045			TRAP	10		100	
572	KK	1692		CP							
573	KM			COMBINE SUB 1690 AND CP 1691							
574	HC	2									
575	KK	1741		CP							
576	KM			ROUTE CP 1692 TO CP 1741							
577	RK	980	0.0029	.045			TRAP	10		100	
578	KK	1740		SUB							
579	KM			RUNOFF FROM SUB 1740							
580	BA	.0913									
581	LS			80							
582	UK	980	.0029	.15		100					
583	RK	2570	.0028	.045			TRAP	10		100	
584	KK	1742		CP							
585	KM			COMBINE SUB 1740 AND CP 1741							
586	HC	2									
587	KK	1781		CP							
588	KM			ROUTE CP 1742 TO CP 1781							
589	RK	980	0.0029	.045			TRAP	10		100	
590	KK	1780		SUB							
591	KM			RUNOFF FROM SUB 1780							
592	BA	.0903									
593	LS			83							
594	UK	980	.0029	.15		100					
595	RK	2570	.0028	.045			TRAP	10		100	
596	KK	1782		CP							
597	KM			COMBINE SUB 1780 AND CP 1781							
598	HC	2									
599	KK	1831		CP							
600	KM			ROUTE CP 1782 TO CP 1831							
601	RK	1230	0.0029	.045			TRAP	10		100	

LINE	ID	1	2	3	4	5	6	7	8	9	10
602	KK	1832		CP							
603	KM			COMBINE CP 1831 AND SUB 1830							
604	HC		2								
605	KK	1820		SUB							
606	KM			RUNOFF FROM SUB 1820 AND ROUTE CP 1832							
607	BA	.1134									
608	LS		84								
609	UK	1230	.0029	.15	100						
610	RK	2570	.0028	.024		TRAP	5	1	YES		
611	KK	1650		SUB							
612	KM			RUNOFF FROM SUB 1650							
613	BA	.0903									
614	LS		80								
615	UK	980	.0029	.15	100						
616	RK	2570	.0028	.045		TRAP	10	100			
617	KK	1701		CP							
618	KM			ROUTE SUB 1650 TO CP 1701							
619	RK	980	0.0029	.045		TRAP	10	100			
620	KK	1700		SUB							
621	KM			RUNOFF FROM SUB 1700							
622	BA	.0922									
623	LS		80								
624	UK	980	.0029	.15	100						
625	RK	2570	.0028	.045		TRAP	10	100			
626	KK	1702		CP							
627	KM			COMBINE SUB 1700 AND CP 1701							
628	HC		2								
629	KK	1731		CP							
630	KM			ROUTE CP 1702 TO CP 1731							
631	RK	980	0.0029	.045		TRAP	10	100			
632	KK	1730		SUB							
633	KM			RUNOFF FROM SUB 1730							
634	BA	.0913									
635	LS		80								
636	UK	990	.0029	.15	100						
637	RK	2570	.0028	.045		TRAP	10	100			
638	KK	1732		CP							
639	KM			COMBINE SUB 1730 AND CP 1731							
640	HC		2								
641	KK	1791		CP							
642	KM			ROUTE CP 1732 TO CP 1791							
643	RK	980	0.0029	.045		TRAP	10	100			

LINE	ID	1	2	3	4	5	6	7	8	9	10
644	KK	1790		SUB							
645	KM			RUNOFF FROM SUB 1790							
646	BA	.0903									
647	LS		82								
648	UK	980	.0029	.15	100						
649	RK	2570	.0028	.045		TRAP	10	100			
650	KK	1792		CP							
651	KM			COMBINE SUB 1790 AND CP 1791							
652	HC	2									
653	KK	1822		CP							
654	KM			ROUTE CP 1792 TO CP 1822							
655	RK	1230	0.0029	.045		TRAP	10	100			
656	KK	1823		CP							
657	KM			COMBINE SUB 1820 AND CP 1822							
658	HC	2									
659	KK	1810		SUB							
660	KM			RUNOFF FROM SUB 1810 AND ROUTE CP 1823							
661	BA	.1156									
662	LS		87								
663	UK	1230	.0029	.15	100						
664	RK	2620	.0028	.024		TRAP	5	1	YES		
665	KK	1210		SUB							
666	KM			RUNOFF FROM SUB 1210							
667	BA	.9319									
668	LS		77								
669	UK	488	.0080	.10	100						
670	RK	9400	.0097	.045		TRAP	10	20			
671	KK	1201		CP							
672	KM			ROUTE SUB 1210 TO CP 1201							
673	RK	3650	.0044	.030		TRAP	20	10			
674	KK	1207		SUB							
675	KM			RUNOFF FROM SUB 1207							
676	BA	.1159									
677	LS		85	20							
678	UK	145	.0213	.10	100						
679	RK	2000	.0210	.045		TRAP	10	5			
680	KK	1204		SUB							
681	KM			RUNOFF FROM SUB 1204 & ROUTE SUB 1207							
682	BA	.6299									
683	LS		81								
684	UK	145	.0213	.10	100						
685	RK	7400	.0105	.045		TRAP	10	10	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
686	KK	1200		SUB							
687	KM			RUNOFF FROM SUB 1200 & ROUTE SUB 1204							
688	BA	.5624									
689	LS		77								
690	UK	610	.0080	.10	100						
691	RK	8480	.0083	.045		TRAP	10	20	YES		
692	KK	1202		CP							
693	KM			COMBINE SUB 1200 & CP 1201							
694	HC	2									
695	KK	1191		CP							
696	KM			ROUTE CP 1202 TO CP 1191							
697	RK	2400	.0054	.035		TRAP	25	15			
698	KK	1186		SUB							
699	KM			RUNOFF FROM SUB 1186							
700	BA	.1189									
701	LS		81	20							
702	UK	145	.0213	.10	100						
703	RK	3800	.0111	.045		TRAP	10	3			
704	KK	1187		SUB							
705	KM			RUNOFF FROM SUB 1187 & ROUTE SUB 1186							
706	BA	.1871									
707	LS		78								
708	UK	145	.0213	.10	100						
709	RK	5800	.0100	.045		TRAP	10	5	YES		
710	KK	1190		SUB							
711	KM			RUNOFF FROM SUB 1190 & ROUTE SUB 1187							
712	BA	.7228									
713	LS		77								
714	UK	610	.0080	.10	100						
715	RK	10600	.0081	.045		TRAP	10	20	YES		
716	KK	1192		CP							
717	KM			COMBINE SUB 1190 & CP 1191							
718	HC	2									
719	KK	1181		CP							
720	KM			ROUTE CP 1192 TO CP 1181							
721	RK	1850	.0059	.035		TRAP	25	15			
722	KK	1173		CP							
723	KM			DISCHARGE FROM 36" RCP OVER CAP AT STA 243+00							
724	IN	60	20APR89								
725	BA	.00001									
726	QI	0	0	0	0	0	0	0	0	0	0
727	QI	15	37	37	25	20	15	10	5	0	0
728	QI	0	0	0	0	0					

LINE	ID	1	2	3	4	5	6	7	8	9	10
729	KK	1174		SUB							
730	KM			RUNOFF FROM SUB 1174 & ROUTE CAP CROSS-DRAINAGE FROM CP 1173							
731	BA	.2122									
732	LS		78	20							
733	UK	145	.0213	.10	100						
734	RK	3400	.0103	.045		TRAP	10	4	YES		
735	KK	1177		SUB							
736	KM			RUNOFF FROM SUB 1177 & ROUTE SUB 1174							
737	BA	.2394									
738	LS		77								
739	UK	145	.0213	.10	100						
740	RK	5600	.0123	.045		TRAP	10	5	YES		
741	KK	1180		SUB							
742	KM			RUNOFF FROM SUB 1180 & ROUTE SUB 1177							
743	BA	.7011									
744	LS		77								
745	UK	575	.0080	.10	100						
746	RK	11000	.0078	.045		TRAP	10	20	YES		
747	KK	1000		SUB							
748	KM			RUNOFF FROM SUB 1000							
749	BA	.0166									
750	LS		77	60							
751	UK	725	.0063	.10	100						
752	RK	700	.0040	.024		TRAP	10	50			
753	KK	1042		CP							
754	KM			ROUTE SUB 1000 TO CP 1042							
755	RK	320	0.004	.024		TRAP	10	50			
756	KK	1060		SUB							
757	KM			RUNOFF FROM SUB 1060							
758	BA	.0399									
759	LS		77	60							
760	UK	210	.0014	.09	100						
761	RK	2400	.0001	.024		TRAP	5	25			
762	KK	1051		CP							
763	KM			ROUTE SUB 1060 TO CP 1051							
764	RK	800	.0040	.024		TRAP	10	50			
765	KK	1050		SUB							
766	KM			RUNOFF FROM SUB 1050							
767	BA	.0009									
768	LS		77	60							
769	UK	100	.0040	.09	100						
770	RK	450	.0040	.024	.0006	TRAP	5	25			
771	RK	200	.0040	.040		TRAP	5	10			

LINE	ID	1	2	3	4	5	6	7	8	9	10
772	KK	1052		CP							
773	KM			COMBINE SUB 1050 & CP 1051							
774	HC		2								
775	KK	1041		CP							
776	KM			ROUTE CP 1052 TO CP 1041							
777	RK	1820	.0040	.024		TRAP	10		50		
778	KK	1040		SUB							
779	KM			RUNOFF FROM SUB 1040							
780	BA	.0381									
781	LS		77	60							
782	UK	100	.0040	.09	100						
783	RK	2200	.0040	.025		TRAP	0		50		
784	KK	1043		CP							
785	KM			COMBINE SUB 1040 WITH CP 1041 AND CP 1442							
786	HC		3								
787	KK	DAM8									
788	KM			ROUTE CP 1043 THROUGH RETENTION BASIN 8							
789	RS	1	STOR								
790	SV	0	1.895	3.79							
791	SE	100	101	102							
792	SS	101.99	200	2.9	1.5						
793	ST	102	200	2.9	1.5						
794	KK	1011		CP							
795	KM			ROUTE OUTFLOW FROM DAM 8 TO CP 1011							
796	RK	1280	0.0040	.024		TRAP	10		50		
797	KK	1010		SUB							
798	KM			RUNOFF FROM SUB 1010							
799	BA	.0387									
800	LS		77	45							
801	UK	130	.0040	.09	100						
802	RK	680	.0040	.024	.0057	TRAP	5		25		
803	RK	1250	.0040	.024		TRAP	5		25		
804	KK	1012		CP							
805	KM			COMBINE SUB 1010 WITH CP 1011							
806	HC		2								
807	KK	1110		SUB							
808	KM			RUNOFF FROM SUB 1110 AND ROUTE CP 1012							
809	KM			ROUTING DOWN ALMA SCHOOL ROAD							
810	BA	.0109									
811	LS		77								
812	UK	450	.0080	.10	100						
813	RK	950	.0074	.030		TRAP	20	1	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
814	KK	1020									
			SUB								
815	KM										
			RUNOFF FROM SUB 1020								
816	BA	.0345									
817	LS		77	45							
818	UK	110	.0063	.10	100						
819	RK	1300	.0054	.024			TRAP	5	1		
820	KK	1120									
			SUB								
821	KM										
			RUNOFF FROM SUB 1120 AND ROUTE SUB 1020								
822	BA	.0352									
823	LS		77								
824	UK	355	.0080	.10	100						
825	RK	1200	.0083	.045			TRAP	5	3	YES	
826	KK	1121									
			CP								
827	KM										
			COMBINE SUB 1110 AND SUB 1120								
828	HC	2									
829	KK	2222									
			DIV								
830	KM										
			DIVERT FLOW TO SUB 480								
831	DT	2223									
832	DI	0	1000	10000							
833	DG	0	400	4000							
834	KK	1131									
			CP								
835	KM										
			ROUTE DIV 2222 TO CP 1131								
836	KM										
			ROUTING DOWN ALMA SCHOOL ROAD								
837	RK	750	.0027	.030			TRAP	20	1		
838	KK	1070									
			SUB								
839	KM										
			RUNOFF FROM SUB 1070								
840	BA	.0393									
841	LS		77	45							
842	UK	130	.0040	.10	100						
843	RK	300	.0040	.024	.0057		TRAP	0	50		
844	RK	1900	.004	.035			TRAP	5	10		
845	KK	1031									
			CP								
846	KM										
			ROUTE SUB 1070 TO CP 1031								
847	RK	400	0.0035	.035			TRAP	5	10		
848	KK	1030									
			SUB								
849	KM										
			RUNOFF FROM SUB 1030								
850	BA	.0308									
851	LS		77	45							
852	UK	160	.0040	.10	100						
853	RK	500	.0041	.025	.0045		TRAP	0	50		
854	RK	1800	.003	.035			TRAP	12	4		

LINE	ID	1	2	3	4	5	6	7	8	9	10
897	KK	4444									
898	KM										
899	DT	4445									
900	DI	0	100	500	1000	5000					
901	DQ	0	50	250	500	2500					
902	KK	1161									
903	KM										
904	KM										
905	RK	3350	.0048	.030			TRAP	20	5		
906	KK	1159									
	*	*****									
	*	DISCHARGE IN CFS RELEASED FROM 3-72" CMP ACROSS CAP AT STA 234+10									
	*	*****									
907	KM										
908	IN	10	20APR89	0							
909	BA	.0001									
910	QI	0	2	5	7	10	14	17	21	25	29
911	QI	33	37	41	46	51	57	63	69	75	82
912	QI	89	96	104	112	120	130	140	150	162	173
913	QI	185	198	212	225	239	252	266	283	300	317
914	QI	333	350	366	384	402	420	440	460	480	500
915	QI	520	540	557	573	590	607	623	640	655	670
916	QI	695	712	728	745	757	770	782	790	798	806
917	QI	811	817	822	825	827	830	831	833	834	834
918	QI	833	832	832	831	830	828	825	823	822	820
919	QI	819	817	814	812	810	809	807	804	802	799
920	QI	796	793	790	787	783	780	776	773	770	766
921	QI	763	760	756	752	748	743	738	733	728	724
922	QI	719	713	706	700	694	687	681	674	667	660
923	QI	653	645	638	628	618	608	597	589	580	570
924	QI	559	549	539	528	518					
	*	*****									
925	KK	1154									
926	KM										
927	BA	.1600									
928	LS		85	20							
929	UK	145	.0213	.10	100						
930	RK	4600	.0120	.045			TRAP	10	5	YES	
931	KK	1155									
932	KM										
933	BA	.4584									
934	LS		77								
935	UK	305	.0213	.10	100						
936	RK	6280	.0105	.045			TRAP	12	5	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
937	KK	1153		SUB							
938	KM			RUNOFF FROM SUB 1153							
939	BA	.0773									
940	LS		77	20							
941	UK	145	.0213	.10	100						
942	RK	1600	.0100	.035			TRAP	10	5		
943	KK	1101		CP							
944	KM			ROUTE SUB 1153 TO CP 1101							
945	RK	2500	.0104	.045			TRAP	12	5		
946	KK	1100		SUB							
947	KM			RUNOFF FROM SUB 1100							
948	BA	.0043									
949	LS		77	38							
950	UK	150	.0040	.10	100						
951	RK	450	.0030	.024			TRAP	5	25		
952	KK	1102		CP							
953	KM			COMBINE SUB 1100 & CP 1101							
954	HC	2									
955	KK	1156		CP							
956	KM			ROUTE CP 1102 TO CP 1156							
957	RK	2800	.0089	.045			TRAP	12	5		
958	KK	1157		CP							
959	KM			COMBINE SUB 1155 & CP 1156							
960	HC	2									
961	KK	1160		SUB							
962	KM			RUNOFF FROM SUB 1160 & ROUTE CP 1157							
963	BA	.3033									
964	LS		77								
965	UK	640	.0080	.10	100						
966	RK	7200	.0071	.045			TRAP	10	20	YES	
967	KK	1166		SUB							
968	KM			RUNOFF FROM SUB 1166							
969	BA	.1080									
970	LS		85	20							
971	UK	145	.0213	.10	100						
972	RK	2800	.0143	.045			TRAP	6	3		
973	KK	1165		SUB							
974	KM			RUNOFF FROM SUB 1165 & ROUTE SUB 1166							
975	BA	.3942									
976	LS		79								
977	UK	145	.0213	.10	100						
978	RK	5800	.0121	.045			TRAP	10	5	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
979	KK	1170									
980	KM										
981	BA	.2588									
982	LS		77								
983	UK	480	.0080	.10	100						
984	RK	6800	.0078	.045			TRAP	10	20	YES	
985	KK	1162									
986	KM										
987	RK	1850	.0070	.045			TRAP	10	20		
988	KK	1163									
989	KM										
990	HC	3									
991	KK	5555									
992	KM										
993	DT	5556									
994	DI	0	1000	4000							
995	DQ	0	500	2000							
996	KK	1182									
997	KM										
998	KM										
999	RK	1500	.0053	.030			TRAP	20	5		
1000	KK	1183									
1001	KM										
1002	HC	2									
1003	KK	1184									
1004	KM										
1005	KM										
1006	RK	1850	.0059	.030			TRAP	20	10		
1007	KK	1185									
1008	KM										
1009	HC	2									
1010	KK	1646									
1011	KM										
1012	KM										
1013	RK	980	.0054	.030			TRAP	20	10		
1014	KK	1645									
1015	KM										
1016	BA	.0921									
1017	LS		84								
1018	UK	980	.0054	.15	100						
1019	RK	2620	.0035	.045			TRAP	10	100		

LINE	ID	1	2	3	4	5	6	7	8	9	10
1059	KK	1811	CP								
1060	KM	ROUTE CP 1802 TO CP 1811									
1061	RK	1230	.0029	.030		TRAP	20	10			
1062	KK	1812	CP								
1063	KM	COMBINE CP 1811 AND SUB 1810									
1064	HC	2									
1065	KK	1813	CP								
1066	KM	ROUTE CP 1812 TO CP 1813									
1067	RK	700	.0029	.030		TRAP	20	10			
1068	KK	1985	CP								
1069	KM	COMBINE CP 1813 AND CP 1984									
1070	HC	2									
1071	KK	1986	CP								
1072	KM	ROUTE CP 1985 TO CP 1986									
1073	RS	1	FLOW	-1							
1074	RC	.050	.024	.080	2900	.0007					
1075	RX	0	50	100	115	180	190.5	194	544		
1076	RY	400	400	400	385	385	395.5	392	395.5		
1077	KK	3334	RET								
1078	KM	RETRIEVE DIVERT FROM CP 1132									
1079	DR	3334									
1080	KK	1135	SUB								
1081	KM	RUNOFF FROM SUB 1135 AND ROUTE RET 3334									
1082	BA	.1096									
1083	LS	80									
1084	UK	260	.0057	.10	100						
1085	RK	4400	.0070	.045		TRAP	5	10	YES		
1086	KK	1136	CP								
1087	KM	ROUTE SUB 1135 TO CP 1136 (ROUTE ALONG DIRT ROAD)									
1088	RK	1700	.0047	.030		TRAP	10	5			
1089	KK	4445	RET								
1090	KM	RETRIEVE DIVERT FROM CP 1152									
1091	DR	4445									
1092	KK	1140	SUB								
1093	KM	RUNOFF FROM SUB 1140 AND ROUTE RET 4445									
1094	BA	.1387									
1095	LS	80									
1096	UK	770	.0057	.10	100						
1097	RK	4000	.0063	.045		TRAP	5	10	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
1098	KK	1141	CP								
1099	KM	COMBINE CP 1136 AND SUB 1140									
1100	HC	2									
1101	KK	1971	CP								
1102	KM	ROUTE CP 1141 TO CP 1971 (ROUTE ALONG DIRT ROAD)									
1103	RK	4000	.0045	.030		TRAP	10	5			
1104	KK	5556	RET								
1105	KM	RETRIEVE DIVERT FROM CP 1163									
1106	DR	5556									
1107	KK	1972	CP								
1108	KM	ROUTE RET 5556 TO CP 1972									
1109	RK	5200	.0052	.045		TRAP	5	10			
1110	KK	1970	SUB								
1111	KM	RUNOFF FROM SUB 1970									
1112	BA	.5112									
1113	LS	81									
1114	UK	1200	.0057	.10	100						
1115	RK	6400	.0061	.045		TRAP	5	10			
1116	KK	1973	CP								
1117	KM	COMBINE SUB 1970, CP 1171, AND CP 1172									
1118	HC	3									
1119	KK	1980	SUB								
1120	KM	RUNOFF FROM SUB 1980 AND ROUTE CP 1973 (ROUTE ALONG DIRT ROAD)									
1121	BA	.4734									
1122	LS	85									
1123	UK	1200	.0057	.10	100						
1124	RK	4500	.0038	.045		TRAP	15	25	YES		
1125	KK	1987	CP								
1126	KM	COMBINE SUB 1980 WITH CP 1986									
1127	HC	2									
1128	KK	1991	CP								
1129	KM	ROUTE CP 1987 TO CP 1991									
1130	RS	1	FLOW	-1							
1131	RC	.050	.024	.080	2660	.0007					
1132	RX	0	50	100	115	180	190.5	194	544		
1133	RY	400	400	400	385	385	395.5	392	395.5		
1134	KK	1960	SUB								
1135	KM	RUNOFF FROM SUB 1960									
1136	BA	.3809									
1137	LS	85									
1138	UK	1200	.0057	.10	100						
1139	RK	6100	.0051	.045		TRAP	5	15			

LINE	ID	1	2	3	4	5	6	7	8	9	10
1140	KK	1990									
			SUB								
1141	KM										
1142	BA	.3793									
1143	LS		85								
1144	UK	1200	.0057	.10	100						
1145	RK	4200	.0038	.045			TRAP	10	20	YES	
1146	KK	1993									
			CP								
1147	KM										
1148	HC	2									
1149	KK	2001									
			CP								
1150	KM										
1151	RS	1	FLOW	-1							
1152	RC	.050	.024	.080	1550	.0007					
1153	RX	0	50	100	115	180	190.5	194	544		
1154	RY	400	400	400	385	385	395.5	392	395.5		
1155	KK	2000									
			SUB								
1156	KM										
1157	BA	.1153									
1158	LS		85								
1159	UK	1200	.0057	.10	100						
1160	RK	2500	.0048	.045			TRAP	5	15		
1161	KK	2002									
			CP								
1162	KM										
1163	HC	2									
1164	KK	541									
			CP								
1165	KM										
1166	RS	1	FLOW	-1							
1167	RC	.050	.024	.080	1100	.0007					
1168	RX	0	50	100	115	180	190.5	194	544		
1169	RY	400	400	400	385	385	395.5	392	395.5		
1170	KK	540									
			SUB								
1171	KM										
1172	BA	.2064									
1173	LS		77								
1174	UK	1200	.0057	.10	100						
1175	RK	4400	.0045	.045			TRAP	5	15		
1176	KK	542									
			CP								
1177	KM										
1178	HC	2									
1179	KK	546									
			CP								
1180	KM										
1181	RS	1	FLOW	-1							
1182	RC	.050	.024	.080	850	.0007					
1183	RX	0	50	100	115	180	190.5	194	544		
1184	RY	400	400	400	385	385	395.5	392	395.5		
	*	*****									
	*	BEGIN DRAINAGE AREA CONTRIBUTING DIRECTLY TO HIGHWAY DRAINAGE CHANNEL									
	*	*****									

LINE	ID	1	2	3	4	5	6	7	8	9	10
1185	KK	165									
1186	KM										
1187	BA	.0133									
1188	LS		77	85							
1189	UK	230	.005	.09	100						
1190	RK	320	.004	.024			TRAP	10	25		
1191	KK	DAM165									
1192	KM										
1193	RS	1	STOR								
1194	SV	0	.265	.53							
1195	SE	100	101	102							
1196	SS	101.99	200	2.9	1.5						
1197	ST	102	200	2.9	1.5						
1198	KK	156									
1199	KM										
1200	RK	840	.004	.024			TRAP	10	25		
1201	KK	160									
1202	KM										
1203	BA	.0234									
1204	LS		77	85							
1205	UK	230	.005	.09	100						
1206	RK	880	.004	.024			TRAP	10	25		
1207	KK	DAM160									
1208	KM										
1209	RS	1	STOR								
1210	SV	0	.465	.93							
1211	SE	100	101	102							
1212	SS	101.99	200	2.9	1.5						
1213	ST	102	200	2.9	1.5						
1214	KK	161									
1215	KM										
1216	RK	740	.004	.025			TRAP	50	2		
1217	KK	155									
1218	KM										
1219	BA	.0256									
1220	LS		77	85							
1221	UK	230	.005	.09	100						
1222	RK	1120	.004	.024			TRAP	10	25		
1223	KK	DAM155									
1224	KM										
1225	RS	1	STOR								
1226	SV	0	.51	1.02							
1227	SE	100	101	102							
1228	SS	101.99	200	2.9	1.5						
1229	ST	102	200	2.9	1.5						

LINE	ID	1	2	3	4	5	6	7	8	9	10
1230	KK	157		CP							
1231	KM	COMBINE DAM 155 WITH CP 156 AND CP 161									
1232	HC	3									
1233	KK	151		CP							
1234	KM	ROUTE CP 157 TO CP 151									
1235	RK	1140	.004	.024		TRAP	10		25		
1236	KK	150		SUB							
1237	KM	RUNOFF FROM SUB 150									
1238	BA	.0427									
1239	LS		77	85							
1240	UK	230	.005	.09	100						
1241	RK	1200	.004	.024		TRAP	10		25		
1242	KK	DAM150									
1243	KM	ROUTE SUB 150 THROUGH RETENTION BASIN									
1244	RS	1		STOR							
1245	SV	0	.85	1.70							
1246	SE	100	101	102							
1247	SS	101.99	200	2.9	1.5						
1248	ST	102	200	2.9	1.5						
1249	KK	152		CP							
1250	KM	COMBINE DAM 150 WITH CP 151									
1251	HC	2									
1252	KK	121		CP							
1253	KM	ROUTE CP 152 TO CP 121									
1254	RK	720	.004	.024		TRAP	10		25		
1255	KK	70		SUB							
1256	KM	RUNOFF FROM SUB 70									
1257	BA	.0108									
1258	LS		77								
1259	UK	270	.00625	.1	100						
1260	RK	760	.004	.024		TRAP	10		50		
1261	KK	122		CP							
1262	KM	ROUTE CP SUB 70 TO CP 122									
1263	RK	520	.004	.024		TRAP	10		25		
1264	KK	120		SUB							
1265	KM	RUNOFF FROM SUB 120									
1266	BA	.0149									
1267	LS		77	85							
1268	UK	270	.005	.09	100						
1269	RK	800	.004	.024		TRAP	0		25		

LINE	ID	1	2	3	4	5	6	7	8	9	10
1270	KK	DAM120									
1271	KM	ROUTE SUB 120 THROUGH RETENTION BASIN									
1272	RS	1	STOR								
1273	SV	0	.295	.59							
1274	SE	100	101	102							
1275	SS	101.99	200	2.9	1.5						
1276	ST	102	200	2.9	1.5						
1277	KK	123	CP								
1278	KM	COMBINE DAM 120 WITH CP 121 AND CP 122									
1279	HC	3									
1280	KK	75	SUB								
1281	KM	RUNOFF FROM SUB 75 AND ROUTE CP 123									
1282	BA	.0025									
1283	LS		77	85							
1284	UK	100	.005	.09	100						
1285	RK	200	.004	.024	TRAP	10	50	YES			
1286	KK	DAM75									
1287	KM	ROUTE SUB 75 THROUGH RETENTION BASIN									
1288	RS	1	STOR								
1289	SV	0	.05	.10							
1290	SE	100	101	102							
1291	SS	101.99	200	2.9	1.5						
1292	ST	102	200	2.9	1.5						
1293	KK	85	SUB								
1294	KM	RUNOFF FROM SUB 85 AND ROUTE DAM 75									
1295	BA	.0027									
1296	LS		77	85							
1297	UK	70	.005	.09	100						
1298	RK	285	.004	.024	TRAP	10	25	YES			
1299	KK	DAM85									
1300	KM	ROUTE SUB 85 THROUGH RETENTION BASIN									
1301	RS	1	STOR								
1302	SV	0	.055	.11							
1303	SE	100	101	102							
1304	SS	101.99	200	2.9	1.5						
1305	ST	102	200	2.9	1.5						
1306	KK	80	SUB								
1307	KM	RUNOFF FROM SUB 80									
1308	BA	.0066									
1309	LS		77	85							
1310	UK	105	.005	.09	100						
1311	RK	240	.004	.024	TRAP	0	25				

LINE	ID	1	2	3	4	5	6	7	8	9	10
1312	KK	DAM80									
1313	KM	ROUTE SUB 80 THROUGH RETENTION BASIN									
1314	RS	1	STOR								
1315	SV	0	.13	.26							
1316	SE	100	101	102							
1317	SS	101.99	200	2.9	1.5						
1318	ST	102	200	2.9	1.5						
1319	KK	87	CP								
1320	KM	ROUTE DAM 80 TO CP 87									
1321	RK	240	.004	.024	TRAP	10	50				
1322	KK	88	CP								
1323	KM	COMBINE CP 87 AND DAM 85									
1324	HC	2									
1325	KK	96	CP								
1326	KM	ROUTE CP 88 TO CP 96									
1327	RK	600	.004	.024	TRAP	10	25				
1328	KK	95	SUB								
1329	KM	RUNOFF FROM SUB 95									
1330	BA	.0124									
1331	LS		77	95							
1332	UK	315	.005	.09	100						
1333	RK	1020	.004	.035	TRAP	6	3.6				
1334	KK	DAM95									
1335	KM	ROUTE SUB 95 THROUGH RETENTION BASIN									
1336	RS	1	STOR								
1337	SV	0	.275	.55							
1338	SE	100	101	102							
1339	SS	101.99	200	2.9	1.5						
1340	ST	102	200	2.9	1.5						
1341	KK	97	CP								
1342	KM	COMBINE DAM 95 WITH CP 96									
1343	HC	2									
1344	KK	91	CP								
1345	KM	ROUTE CP 97 TO CP 91									
1346	RK	520	.004	.024	TRAP	10	25				
1347	KK	90	SUB								
1348	KM	RUNOFF FROM SUB 90									
1349	BA	.0151									
1350	LS		77	60							
1351	UK	150	.005	.09	100						
1352	RK	1000	.004	.035	TRAP	30	3				

LINE	ID	1	2	3	4	5	6	7	8	9	10
1353	KK	DAM90									
1354	KM	ROUTE SUB 90 THROUGH RETENTION BASIN									
1355	RS	1	STOR								
1356	SV	0	.26	.52							
1357	SE	100	101	102							
1358	SS	101.99	200	2.9	1.5						
1359	ST	102	200	2.9	1.5						
1360	KK	92	CP								
1361	KM	COMBINE DAM 90 WITH CP 91									
1362	HC	2									
1363	KK	110	SUB								
1364	KM	RUNOFF FROM SUB 110									
1365	BA	.0158									
1366	LS		77	85							
1367	UK	120	.005	.09	100						
1368	RK	1280	.004	.024	TRAP		10	25			
1369	KK	DAM110									
1370	KM	ROUTE SUB 110 THROUGH RETENTION BASIN									
1371	RS	1	STOR								
1372	SV	0	.315	.63							
1373	SE	100	101	102							
1374	SS	101.99	200	2.9	1.5						
1375	ST	102	200	2.9	1.5						
1376	KK	101	CP								
1377	KM	ROUTE DAM 110 TO CP 101									
1378	RK	680	.004	.024	TRAP		10	25			
1379	KK	100	SUB								
1380	KM	RUNOFF FROM SUB 100									
1381	BA	.0032									
1382	LS		77	85							
1383	UK	200	.005	.09	100						
1384	RK	640	.004	.024	TRAP		10	25			
1385	KK	DAM100									
1386	KM	ROUTE SUB 100 THROUGH RETENTION BASIN									
1387	RS	1	STOR								
1388	SV	0	.11	.22							
1389	SE	100	101	102							
1390	SS	101.99	200	2.9	1.5						
1391	ST	102	200	2.9	1.5						
1392	KK	102	CP								
1393	KM	COMBINE CP 101 WITH DAM 100 AND CP 92									
1394	HC	3									

LINE	ID	1	2	3	4	5	6	7	8	9	10
1395	KK	103		CP							
1396	KM			ROUTE CP 102 TO CP 103							
1397	RK	250	.004	.018		TRAP	10		1		
1398	KK	360		SUB							
1399	KM			RUNOFF FROM SUB 360 AND ROUTE CP 103							
1400	BA	.0253									
1401	LS			77							
1402	UK	1000	.0057	.10	100						
1403	RK	1200	.0042	.012		TRAP	15		1	YES	
1404	KK	410		SUB							
1405	KM			RUNOFF FROM SUB 410 & ROUTE SUB 360							
1406	BA	.0330									
1407	LS			77							
1408	UK	400	.0057	.10	100						
1409	RK	1500	.0067	.045	.0330	TRAP	5		5		
1410	RK	1000	.0060	.012		TRAP	15		1	YES	
1411	KK	421		CP							
1412	KM			ROUTE SUB 410 TO CP 421							
1413	RK	200	.010	.012		TRAP	15		1		
1414	KK	140		SUB							
1415	KM			RUNOFF FROM SUB 140							
1416	BA	.0263									
1417	LS			77	85						
1418	UK	230	.005	.09	100						
1419	RK	2000	.004	.024		TRAP	10		50		
1420	KK	DAM140									
1421	KM			ROUTE SUB 140 THROUGH RETENTION BASIN							
1422	RS	1		STOR							
1423	SV	0	.525	1.05							
1424	SE	100	101	102							
1425	SS	101.99	200	2.9	1.5						
1426	ST	102	200	2.9	1.5						
1427	KK	130		SUB							
1428	KM			RUNOFF FROM SUB 130 AND ROUTE DAM 140							
1429	BA	.0317									
1430	LS			77	85						
1431	UK	510	.005	.09	100						
1432	RK	200	.004	.025		TRAP	5		2	YES	
1433	KK	DAM130									
1434	KM			ROUTE SUB 130 THROUGH RETENTION BASIN							
1435	RS	1		STOR							
1436	SV	0	.505	1.01							
1437	SE	100	101	102							
1438	SS	101.99	200	2.9	1.5						
1439	ST	102	200	2.9	1.5						

LINE	ID	1	2	3	4	5	6	7	8	9	10
1440	KK	420									
			SUB								
1441	KM										
			RUNOFF FROM SUB 420 AND ROUTE DAM 130								
1442	BA	.0940									
1443	LS		77								
1444	UK	1200	.0057	.10	100						
1445	RK	2400	.0058	.045			TRAP	5	5	YES	
1446	KK	422									
			CP								
1447	KM										
			COMBINE SUB 420 WITH CP 421								
1448	HC	2									
1449	KK	401									
			CP								
1450	KM										
			ROUTE CP 422 TO CP 401								
1451	RK	800	.0013	.012			TRAP	15	1		
1452	KK	240									
			SUB								
1453	KM										
			RUNOFF FROM SUB 240								
1454	BA	.0739									
1455	LS		77	33							
1456	UK	140	.004	.09	100						
1457	RK	1720	.004	.024	.034		TRAP	10	25		
1458	KK	246									
			CP								
1459	KM										
			ROUTE SUB 240 TO CP 246								
1460	RK	960	.0077	.035			TRAP	3.5	2.6		
1461	KK	245									
			SUB								
1462	KM										
			RUNOFF FROM SUB 245								
1463	BA	.0235									
1464	LS		77	36							
1465	UK	130	.004	.09	100						
1466	RK	1320	.004	.024			TRAP	10	25		
1467	KK	247									
			CP								
1468	KM										
			COMBINE CP 246 WITH SUB 245								
1469	HC	2									
1470	KK	248									
			CP								
1471	KM										
			ROUTE CP 247 TO CP 248								
1472	RK	580	.004	.035			TRAP	3.5	2.6		
1473	KK	180									
			SUB								
1474	KM										
			RUNOFF FROM SUB 180								
1475	BA	.0659									
1476	LS		77								
1477	UK	400	.0082	.1	100						
1478	RK	1600	.004	.035			TRAP	5	2		
1479	KK	255									
			SUB								
1480	KM										
			RUNOFF FROM SUB 255								
1481	BA	.0290									
1482	LS		77	36							
1483	UK	135	.004	.09	100						
1484	RK	780	.004	.024	.0079		TRAP	10	25		

LINE	ID	1	2	3	4	5	6	7	8	9	10
1525	KK	188	CP								
1526	KM	ROUTE CP 187 TO CP 188									
1527	RK	680	.004	.024			TRAP	10	50		
1528	KK	189	CP								
1529	KM	COMBINE CP 188 & CP 191									
1530	HC	2									
1531	KK	DAM1									
1532	KM	ROUTE CP 189 THROUGH RETENTION BASIN 1									
1533	RS	1	STOR								
1534	SV	0	5.6	11.2							
1535	SE	100	101	102							
1536	SS	101.99	200	2.9	1.5						
1537	ST	102	200	2.9	1.5						
1538	KK	190	SUB								
1539	KM	RUNOFF FROM SUB 190									
1540	BA	.0321									
1541	LS		77								
1542	UK	1230	.00625	.1	100						
1543	RK	600	.004	.035			TRAP	10	100		
1544	KK	192	CP								
1545	KM	COMBINE OUTFLOW FROM DAM 1 WITH SUB 190									
1546	HC	2									
1547	KK	211	CP								
1548	KM	ROUTE CP 192 TO CP 211									
1549	RK	1320	.004	.035			TRAP	10	2		
1550	KK	210	SUB								
1551	KM	RUNOFF FROM SUB 210									
1552	BA	.0224									
1553	LS		77	45							
1554	UK	153	.004	.09	100						
1555	RK	1200	.004	.024			TRAP	10	25		
1556	KK	212	CP								
1557	KM	COMBINE SUB 210 WITH CP 211									
1558	HC	2									
1559	KK	213	DIV								
1560	KM	DIVERT CP 212 TO SUB 430									
1561	DT	429									
1562	DI	2	50	500	1000						
1563	DQ	1	25	250	500						
1564	KK	405	SUB								
1565	KM	RUNOFF FROM SUB 405 AND ROUTE DIV 213									
1566	BA	.0133									
1567	LS		77								
1568	UK	200	.0057	.10	100						
1569	RK	1200	.0050	.045			TRAP	5	10	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
1570	KK	200	SUB								
1571	KM		RUNOFF FROM SUB 200								
1572	BA	.0416									
1573	LS		77	13							
1574	UK	260	.005	.10	100						
1575	RK	1640	.004	.024			TRAP	10	50		
1576	KK	DAM200									
1577	KM		ROUTE SUB 200 THROUGH RETENTION BASIN								
1578	RS	1	STOR								
1579	SV	0	.125	.25							
1580	SE	100	101	102							
1581	SS	101.99	200	2.9	1.5						
1582	ST	102	200	2.9	1.5						
1583	KK	415	SUB								
1584	KM		RUNOFF FROM SUB 415 AND ROUTE DAM 200								
1585	BA	.0072									
1586	LS		77								
1587	UK	200	.0057	.10	100						
1588	RK	600	.0100	.045			TRAP	5	1	YES	
1589	KK	135	SUB								
1590	KM		RUNOFF FROM SUB 135								
1591	BA	.0253									
1592	LS		77	85							
1593	UK	390	.005	.09	100						
1594	RK	1160	.004	.024			TRAP	10	50		
1595	KK	DAM135									
1596	KM		ROUTE SUB 135 THROUGH RETENTION BASIN								
1597	RS	1	STOR								
1598	SV	0	.505	1.01							
1599	SE	100	101	102							
1600	SS	101.99	200	2.9	1.5						
1601	ST	102	200	2.9	1.5						
1602	KK	425	SUB								
1603	KM		RUNOFF FROM SUB 425 AND ROUTE DAM 135								
1604	BA	.0108									
1605	LS		77								
1606	UK	200	.0057	.10	100						
1607	RK	600	.0083	.045			TRAP	5	1	YES	
1608	KK	426	CP								
1609	KM		COMBINE SUB 405, SUB 415 AND SUB 425								
1610	HC	3									
1611	KK	400	SUB								
1612	KM		RUNOFF FROM SUB 400 AND ROUTE CP 426								
1613	BA	.1483									
1614	LS		81								
1615	UK	1000	.0057	.10	100						
1616	RK	3250	.0046	.045			TRAP	5	10	YES	

LINE	ID	1	2	3	4	5	6	7	8	9	10
1617	KK	402	CP								
1618	KM	COMBINE SUB 400 WITH CP 401									
1619	HC	2									
1620	KK	501	CP								
1621	KM	ROUTE CP 402 TO CP 501									
1622	RK	790	.0038	.012		TRAP	20	1			
1623	KK	500	SUB								
1624	KM	RUNOFF FROM SUB 500 AND ROUTE CP 501									
1625	BA	.1205									
1626	LS		84								
1627	UK	1000	.0057	.10	100						
1628	RK	3160	.0038	.012		TRAP	25	1	YES		
1629	KK	429	RET								
1630	KM	RETRIEVE DIVERT FROM CP 212									
1631	DR	429									
1632	KK	430	SUB								
1633	KM	RUNOFF FROM SUB 430 AND ROUTE RET 429									
1634	BA	.1849									
1635	LS		82								
1636	UK	1025	.0057	.10	100						
1637	RK	5500	.0053	.045		TRAP	5	7	YES		
1638	KK	440	SUB								
1639	KM	RUNOFF FROM SUB 440									
1640	BA	.1004									
1641	LS		83								
1642	UK	1200	.0057	.10	100						
1643	RK	3100	.0055	.045		TRAP	5	5			
1644	KK	441	CP								
1645	KM	COMBINE SUB 430 WITH SUB 440									
1646	HC	2									
1647	KK	502	CP								
1648	KM	ROUTE CP 441 TO CP 502									
1649	RK	1550	.0045	.045		TRAP	5	10			
1650	KK	503	CP								
1651	KM	COMBINE SUB 500 WITH CP 502									
1652	HC	2									
1653	KK	560	SUB								
1654	KM	RUNOFF FROM SUB 560 & ROUTE CP 503									
1655	BA	.0890									
1656	LS		78								
1657	UK	500	.0057	.10	100						
1658	RK	1700	.0047	.045	.0890	TRAP	5	5			
1659	RK	2000	.0045	.012		TRAP	25	1	YES		

LINE	ID	1	2	3	4	5	6	7	8	9	10
1660	KK	551	CP								
1661	KM	ROUTE SUB 560 TO CP 551									
1662	RK	2120	.0047	.012		TRAP	25	1			
1663	KK	304	CP								
	*	*****									
	*	DISCHARGE IN CFS RELEASED FROM THE LAKES									
	*	*****									
1664	KM	FLOW RELEASED FROM THE LAKES									
1665	IN	10	20APR89	0							
1666	BA	.0001									
1667	QI	200	200	200	200	200	200	200	200	200	200
1668	QI	200	200	200	200	200	200	200	200	200	200
1669	QI	200	200	200	200	200	200	200	200	200	200
1670	QI	200	200	200	200	200	200	200	200	200	200
1671	QI	200	200	200	200	200	200	200	200	200	200
1672	QI	200	200	200	200	200	200	200	200	200	200
1673	QI	200	200	200	200	200	200	200	207	230	260
1674	QI	288	305	317	324	330	333	336	336	335	334
1675	QI	333	332	330	328	327	326	324	322	319	317
1676	QI	313	311	308	306	303	301	298	295	291	287
1677	QI	282	278	272	268	263	258	250	244	240	234
1678	QI	227	219	211	203	194	186	179	170	161	152
1679	QI	140	132	124	116	109	102	96	88	79	72
1680	QI	67	62	58	54	49	44	39	35	31	28
1681	QI	24	20	18	15	12					
	*	*****									
1682	KK	300	SUB								
1683	KM	RUNOFF FROM SUB 300 & ROUTE CP 304									
1684	BA	.0319									
1685	LS		77	31							
1686	UK	270	.0033	.10	100						
1687	RK	1360	.0059	.035		TRAP	20	2	YES		
1688	KK	DAM7									
1689	KM	ROUTE SUB 300 THROUGH RETENTION BASIN 7									
1690	RS	1	STOR								
1691	SV	0	.325	.65							
1692	SE	100	101	102							
1693	SS	101.99	200	2.9	1.5						
1694	ST	102	200	2.9	1.5						
1695	KK	291	CP								
1696	KM	ROUTE OUTFLOW FROM DAM 7 TO CP 291									
1697	RK	650	.004	.035		TRAP	12	2			
1698	KK	290	SUB								
1699	KM	RUNOFF FROM SUB 290									
1700	BA	.0154									
1701	LS		77	92							
1702	UK	100	.004	.09	100						
1703	RK	640	.004	.035		TRAP	12	2			

LINE	ID	1	2	3	4	5	6	7	8	9	10
1704	KK	DAM6									
1705	KM	ROUTE SUB 290 THROUGH RETENTION BASIN 6									
1706	RS	1	STOR								
1707	SV	0	.8	1.6							
1708	SE	100	101	102							
1709	SS	101.99	200	2.9	1.5						
1710	ST	102	200	2.9	1.5						
1711	KK	292	CP								
1712	KM	COMBINE OUTFLOW FROM DAM 6 WITH CP 291									
1713	HC	2									
1714	KK	331	CP								
1715	KM	ROUTE CP 292 TO CP 331									
1716	RK	680	.004	.035	TRAP	12	2				
1717	KK	330	SUB								
1718	KM	RUNOFF FROM SUB 330									
1719	BA	.0179									
1720	LS		77	92							
1721	UK	100	.004	.09	100						
1722	RK	1080	.004	.035	TRAP	12	2				
1723	KK	DAM2									
1724	KM	ROUTE SUB 330 THROUGH RETENTION BASIN 2									
1725	RS	1	STOR								
1726	SV	0	.8	1.6							
1727	SE	100	101	102							
1728	SS	101.99	200	2.9	1.5						
1729	ST	102	200	2.9	1.5						
1730	KK	332	CP								
1731	KM	COMBINE OUTFLOW FROM DAM 2 WITH CP 331									
1732	HC	2									
1733	KK	231	CP								
1734	KM	ROUTE CP 332 TO CP 231									
1735	RK	480	.004	.035	TRAP	12	2				
1736	KK	230	SUB								
1737	KM	RUNOFF FROM SUB 230									
1738	BA	.0419									
1739	LS		77	60							
1740	UK	110	.004	.09	100						
1741	RK	1840	.004	.035	TRAP	12	2				
1742	KK	232	CP								
1743	KM	COMBINE SUB 230 WITH CP 231									
1744	HC	2									

LINE	ID	1	2	3	4	5	6	7	8	9	10
1786	KK	DAM3									
1787	KM	ROUTE CP 322 THROUGH RETENTION BASIN 3									
1788	RS	1	STOR								
1789	SV	0	.8	1.6							
1790	SE	100	101	102							
1791	SS	101.99	200	2.9	1.5						
1792	ST	102	200	2.9	1.5						
1793	KK	260	SUB								
1794	KM	RUNOFF FROM SUB 260 AND ROUTE OUTFLOW FROM DAM 3									
1795	BA	.0448									
1796	LS		77	36							
1797	UK	120	.004	.09	100						
1798	RK	1000	.004	.024	.0256	TRAP	10	25			
1799	RK	800	.004	.035		TRAP	8	2	YES		
1800	KK	451	CP								
1801	KM	ROUTE SUB 260 TO CP 451									
1802	RK	760	.004	.035		TRAP	8	2			
1803	KK	450	SUB								
1804	KM	RUNOFF FROM SUB 450 AND ROUTE CP 451									
1805	BA	.0255									
1806	LS		77								
1807	UK	350	.0057	.10	100						
1808	RK	1400	.0057	.045		TRAP	5	2	YES		
1809	KK	452	CP								
1810	KM	COMBINE SUB 450 WITH SUB 445									
1811	HC	2									
1812	KK	455	SUB								
1813	KM	RUNOFF FROM SUB 455 AND ROUTE CP 452									
1814	BA	.0643									
1815	LS		81								
1816	UK	520	.0057	.10	100						
1817	RK	2450	.0057	.045		TRAP	5	10	YES		
1818	KK	280	SUB								
1819	KM	RUNOFF FROM SUB 280									
1820	BA	.0599									
1821	LS		77	85							
1822	UK	165	.004	.09	100						
1823	RK	1600	.004	.024		TRAP	10	50			
1824	KK	DAM5									
1825	KM	ROUTE SUB 280 THROUGH RETENTION BASIN 5									
1826	RS	1	STOR								
1827	SV	0	2.9	5.8							
1828	SE	100	101	102							
1829	SS	101.99	200	2.9	1.5						
1830	ST	102	200	2.9	1.5						

LINE	ID	1	2	3	4	5	6	7	8	9	10
1831	KK	250									
1832	KM										
1833	BA	.0488									
1834	LS		77	36							
1835	UK	150	.004	.09	100						
1836	RK	1840	.004	.024			TRAP	10	50		YES
1837	KK	460									
1838	KM										
1839	BA	.1292									
1840	LS		79								
1841	UK	700	.0057	.10	100						
1842	RK	4000	.0058	.045			TRAP	5	7		YES
1843	KK	461									
1844	KM										
1845	HC	2									
1846	KK	495									
1847	KM										
1848	BA	.1559									
1849	LS		85								
1850	UK	800	.0057	.10	100						
1851	RK	4000	.0043	.045			TRAP	5	15		YES
1852	KK	270									
1853	KM										
1854	BA	.0576									
1855	LS		77	45							
1856	UK	130	.004	.09	100						
1857	RK	1440	.004	.024			TRAP	10	25		
1858	KK	470									
1859	KM										
1860	BA	.0770									
1861	LS		77								
1862	UK	425	.0057	.10	100						
1863	RK	3400	.0062	.045			TRAP	5	5		YES
1864	KK	2223									
1865	KM										
1866	DR	2223									
1867	KK	480									
1868	KM										
1869	BA	.0753									
1870	LS		77								
1871	UK	560	.0057	.10	100						
1872	RK	3200	.0069	.045			TRAP	5	5		YES

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
31	1270	
	V	
	V	
38	1280 ***	
	V	
	V	
44	1350 ***	
	V	
	V	
50	1360 ***	
	V	
	V	
56	1440 ***	
	V	
	V	
62	1450 ***	
	V	
	V	
68	1520 ***	
	V	
	V	
74	1530 ***	
	V	
	V	
80	1600 ***	
	V	
	V	
86	1591	
	.	
	.	
89	.	1260
	.	V
	.	V
95	.	1290 ***
	.	V
	.	V
101	.	1340 ***
	.	V
	.	V
107	.	1370 ***
	.	V
	.	V
113	.	1430 ***
	.	V
	.	V
119	.	1460 ***
	.	V
	.	V
125	.	1510 ***
	.	V
	.	V
131	.	1540 ***
	.	V
	.	V
137	.	1590 ***
	.	.

	V	
	V	
146	1581	
	.	
	.	
149	1250	
	V	
	V	
155	1300 ***	
	V	
	V	
161	1330 ***	
	V	
	V	
167	1380 ***	
	V	
	V	
173	1420 ***	
	V	
	V	
179	1470 ***	
	V	
	V	
185	1500 ***	
	V	
	V	
191	1550 ***	
	V	
	V	
197	1580 ***	
	.	
	.	
203	1582.....	
	V	
	V	
206	1571	
	.	
	.	
209	1240	
	V	
	V	
215	1310 ***	
	V	
	V	
221	1320 ***	
	V	
	V	
227	1400 ***	
	.	
	.	
233	.	1390
	.	V
	.	V
239	.	1391
	.	.
	.	.
242	1392.....	
	V	
	V	
245	1410 ***	
	V	
	V	
251	1480 ***	
	V	

	.	V	
	.	V	
263	.	1560	***
	.	V	
	.	V	
269	.	1570	***
	.	.	
	.	.	
275	1572	
	.	V	
	.	V	
278	1611		
	.	.	
	.	.	
281	.	1610	
	.	.	
	.	.	
287	1612	
	.	V	
	.	V	
290	1621		
	.	.	
	.	.	
293	.	1620	
	.	.	
	.	.	
299	.	.	1630
	.	.	V
	.	.	V
305	.	.	1640 ***
	.	.	.
	.	.	.
311	1622	
	.	V	
	.	V	
314	1887		
	.	.	
	.	.	
320	.	1950	
	.	.	
	.	.	
326	.	.	1945
	.	.	V
	.	.	V
332	.	.	1951
	.	.	.
	.	.	.
335	.	1952
	.	V	
	.	V	
338	.	1940	***
	.	.	
	.	.	
344	.	.	1920
	.	.	V
	.	.	V
350	.	.	1941
	.	.	.
	.	.	.
353	.	1942
	.	V	
	.	V	
356	.	1925	***
	.	V	

368	.	.	1910	
	.	.	V	
	.	.	V	
374	.	.	1900 ***	
	.	.	V	
	.	.	V	
380	.	.	1890 ***	
	.	.	V	
	.	.	V	
386	.	.	1931	
	.	.	.	
	.	.	.	
389	.	1932.....	.	
	.	V	.	
	.	V	.	
392	.	1885 ***	.	
	.	.	.	
	.	.	.	
398	.	.	1850	
	.	.	V	
	.	.	V	
404	.	.	1860 ***	
	.	.	V	
	.	.	V	
410	.	.	1870 ***	
	.	.	V	
	.	.	V	
416	.	.	1886	
	.	.	.	
	.	.	.	
419	.	.	.	1880

425	1888.....	.	.	.
	V	.	.	.
	V	.	.	.
428	1984	.	.	.

434	.	1840	.	.

440	.	.	1670	
	.	.	V	
	.	.	V	
446	.	.	1681	
	.	.	.	
	.	.	.	
449	.	.	.	1680

455	.	.	1682.....	
	.	.	V	
	.	.	V	
458	.	.	1751	
	.	.	.	
	.	.	.	
461	.	.	.	1226

470	.	.	.	-----) 1228
468	.	.	.	1227

473	.	.	.	1225 ***	
	.	.	.	V	
	.	.	.	V	
479	.	.	.	1230 ***	
	.	.	.	V	
	.	.	.	V	
485	.	.	.	1221	
	
	
488	1214
	V
	V
494	1217 ***
	V
	V
500	1220 ***
	V
	V
506	1222

509	.	.	.	1223.....	
	.	.	.	V	
	.	.	.	V	
512	.	.	.	1750 ***	
	
	
518	.	.	.	1752.....	
	.	.	.	V	
	.	.	.	V	
521	.	.	.	1761	
	
	
524	.	.	.	1760	
	
	
530	.	.	.	1762.....	
	.	.	.	V	
	.	.	.	V	
533	.	.	.	1771	
	
	
536	.	.	.	1770	
	
	
542	.	.	.	1772.....	
	.	.	.	V	
	.	.	.	V	
545	.	.	.	1841	
	
	
548	.	.	.	1842.....	
	.	.	.	V	
	.	.	.	V	
551	.	.	.	1830 ***	
	
	
557	.	.	.	1660	
	.	.	.	V	
	.	.	.	V	
563	.	.	.	1691	
	
	
566	.	.	.	1690	

572	.	.	1692.....	
	.	.	V	
	.	.	V	
575	.	.	1741	
	.	.	.	
578	.	.	.	1740

584	.	.	1742.....	
	.	.	V	
	.	.	V	
587	.	.	1781	
	.	.	.	
	.	.	.	
590	.	.	.	1780

596	.	.	1782.....	
	.	.	V	
	.	.	V	
599	.	.	1831	
	.	.	.	
	.	.	.	
602	.	1832.....		
	.	V		
	.	V		
605	.	1820 ***		
	.	.		
	.	.		
611	.	.	1650	
	.	.	V	
	.	.	V	
617	.	.	1701	
	.	.	.	
	.	.	.	
620	.	.	.	1700

626	.	.	1702.....	
	.	.	V	
	.	.	V	
629	.	.	1731	
	.	.	.	
	.	.	.	
632	.	.	.	1730

638	.	.	1732.....	
	.	.	V	
	.	.	V	
641	.	.	1791	
	.	.	.	
	.	.	.	
644	.	.	.	1790

650	.	.	1792.....	
	.	.	V	
	.	.	V	
653	.	.	1822	
	.	.	.	
	.	.	.	
656	.	1823.....		

659	.	1810 ***		
	.	.		
665	.	.	1210	
	.	.	V	
	.	.	V	
671	.	.	1201	
	.	.	.	
674	.	.	.	1207
	.	.	.	V
	.	.	.	V
680	.	.	.	1204 ***
	.	.	.	V
	.	.	.	V
686	.	.	.	1200 ***

692	.	.	1202.....	
	.	.	V	
	.	.	V	
695	.	.	1191	
	.	.	.	
698	.	.	.	1186
	.	.	.	V
	.	.	.	V
704	.	.	.	1187 ***
	.	.	.	V
	.	.	.	V
710	.	.	.	1190 ***

716	.	.	1192.....	
	.	.	V	
	.	.	V	
719	.	.	1181	
	.	.	.	
722	.	.	.	1173
	.	.	.	V
	.	.	.	V
729	.	.	.	1174 ***
	.	.	.	V
	.	.	.	V
735	.	.	.	1177 ***
	.	.	.	V
	.	.	.	V
741	.	.	.	1180 ***

747	.	.	.	1000
	.	.	.	V
	.	.	.	V
753	.	.	.	1042

756	.	.	.	1060
	.	.	.	V
	.	.	.	V
762	.	.	.	1051

765	.	.	.	1050

772	1052.....
	V
	V
775	1041

778	1040

784	1043.....
	V
	V
787	DAM8
	V
	V
794	1011

797	1010

804	1012.....
	V
	V
807	1110 ***

814	1020
	V
	V
820	1120 ***

826	1121.....

831	-----> 2223
829	2222
	V
	V
834	1131

838	1070
	V
	V
845	1031

848	1030

855	1032.....
	V
	V
858	1130 ***

864	1132.....

869	-----> 3334
867	3333
	V

876

1080

V

V

882

1090 ***

V

V

888

1150 ***

894

1152.....

899

-----> 4445

897

4444

V

V

902

1161

906

1159

V

V

925

1154 ***

V

V

931

1155 ***

937

1153

V

V

943

1101

946

1100

952

1102.....

V

V

955

1156

958

1157.....

V

V

961

1160 ***

967

1166

V

V

973

1165 ***

V

V

979

1170 ***

V

V

985

1162

988

1163.....

993----->	5556
991	5555	
	V	
	V	
996	1182	
	
	
1000	1183.....	
	V	
	V	
1003	1184	
	
	
1007	1185.....	
	V	
	V	
1010	1646	
	
	
1014	1645	
	
	
1020	1647.....	
	V	
	V	
1023	1711	
	
	
1026	1710	
	
	
1032	1712.....	
	V	
	V	
1035	1721	
	
	
1038	1720	
	
	
1044	1722.....	
	V	
	V	
1047	1801	
	
	
1050	1800	
	
	
1056	1802.....	
	V	
	V	
1059	1811	
	
	
1062	1812.....	
	V	
	V	
1065	1813	
	
	
1068	1985.....	
	V	
	V	

1079	.	.	.(-----	3334
1077	.	3334		
	.	V		
	.	V		
1080	.	1135	***	
	.	V		
	.	V		
1086	.	1136		
	.	.		
	.	.		
1091	.	.	.(-----	4445
1089	.	.	4445	
	.	V		
	.	V		
1092	.	.	1140	***
	.	.		
	.	.		
1098	.	1141	
	.	V		
	.	V		
1101	.	1971		
	.	.		
	.	.		
1106	.	.	.(-----	5556
1104	.	.	5556	
	.	V		
	.	V		
1107	.	.	1972	
	.	.		
	.	.		
1110	.	.	.	1970

1116	.	1973	
	.	V		
	.	V		
1119	.	1980	***	
	.	.		
	.	.		
1125	1987		
	V			
	V			
1128	1991			
	.			
	.			
1134	.	1960		
	.	V		
	.	V		
1140	.	1990	***	
	.	.		
	.	.		
1146	1993		
	V			
	V			
1149	2001			
	.			
	.			
1155	.	2000		
	.	.		
	.	.		
1161	2002		
	V			
	V			

1170	.	540		
	.	.		
	.	.		
1176	542.....			
	V			
	V			
1179	546			
	.			
	.			
1185	.	165		
	.	V		
	.	V		
1191	.	DAM165		
	.	V		
	.	V		
1198	.	156		
	.	.		
	.	.		
1201	.	.	160	
	.	.	V	
	.	.	V	
1207	.	.	DAM160	
	.	.	V	
	.	.	V	
1214	.	.	161	
	.	.	.	
	.	.	.	
1217	.	.	.	155
	.	.	.	V
	.	.	.	V
1223	.	.	.	DAM155

1230	.	157.....		
	.	V		
	.	V		
1233	.	151		
	.	.		
	.	.		
1236	.	.	150	
	.	.	V	
	.	.	V	
1242	.	.	DAM150	
	.	.	.	
	.	.	.	
1249	.	152.....		
	.	V		
	.	V		
1252	.	121		
	.	.		
	.	.		
1255	.	.	70	
	.	.	V	
	.	.	V	
1261	.	.	122	
	.	.	.	
	.	.	.	
1264	.	.	.	120
	.	.	.	V
	.	.	.	V
1270	.	.	.	DAM120

1277	.	127		

1280	.	V	
	.	75 ***	
	.	V	
	.	V	
1286	.	DAM75	
	.	V	
	.	V	
1293	.	85 ***	
	.	V	
	.	V	
1299	.	DAM85	
	.	.	
	.	.	
1306	.	.	80
	.	V	
	.	V	
1312	.	.	DAM80
	.	V	
	.	V	
1319	.	.	87
	.	.	.
	.	.	.
1322	.	88.....	
	.	V	
	.	V	
1325	.	96	
	.	.	
	.	.	
1328	.	.	95
	.	V	
	.	V	
1334	.	.	DAM95
	.	.	.
	.	.	.
1341	.	97.....	
	.	V	
	.	V	
1344	.	91	
	.	.	
	.	.	
1347	.	.	90
	.	V	
	.	V	
1353	.	.	DAM90
	.	.	.
	.	.	.
1360	.	92.....	
	.	.	
	.	.	
1363	.	.	110
	.	V	
	.	V	
1369	.	.	DAM110
	.	V	
	.	V	
1376	.	.	101
	.	.	.
	.	.	.
1379	.	.	100
	.	.	V
	.	.	V
1385	.	.	DAM100
	.	.	.
	.	.	.

1395	.	V		
	.	103		
	.	V		
	.	V		
1398	.	360 ***		
	.	V		
	.	V		
1404	.	410 ***		
	.	V		
	.	V		
1411	.	421		
	.	.		
	.	.		
1414	.	.	140	
	.	.	V	
	.	.	V	
1420	.	.	DAM140	
	.	.	V	
	.	.	V	
1427	.	.	130 ***	
	.	.	V	
	.	.	V	
1433	.	.	DAM130	
	.	.	V	
	.	.	V	
1440	.	.	420 ***	
	.	.	.	
	.	.	.	
1446	.	422.....		
	.	V		
	.	V		
1449	.	401		
	.	.		
	.	.		
1452	.	.	240	
	.	.	V	
	.	.	V	
1458	.	.	246	
	.	.	.	
	.	.	.	
1461	.	.	.	245

1467	.	.	247.....	
	.	.	V	
	.	.	V	
1470	.	.	248	
	.	.	.	
	.	.	.	
1473	.	.	.	180

1479	.	.	.	255
	.	.	.	V
	.	.	.	V
1486	.	.	.	236

1489	.	.	.	235

1495	.	.	.	237.....
	.	.	.	V
	.	.	.	V
1498	.	.	.	181

1501	.	.	182.....	.
	.	.	V	.
	.	.	V	.
1504	.	.	191	.

1507	.	.	195	.
	.	.	V	.
	.	.	V	.
1513	.	.	186	.

1516	.	.	.	185

1522	.	.	187.....	.
	.	.	V	.
	.	.	V	.
1525	.	.	188	.

1528	.	.	189.....	.
	.	.	V	.
	.	.	V	.
1531	.	.	DAM1	.

1538	.	.	190	.

1544	.	.	192.....	.
	.	.	V	.
	.	.	V	.
1547	.	.	211	.

1550	.	.	210	.

1556	.	.	212.....	.

1561	.	.	----->	429
1559	.	.	213	.
	.	.	V	.
	.	.	V	.
1564	.	.	405 ***	.

1570	.	.	200	.
	.	.	V	.
	.	.	V	.
1576	.	.	DAM200	.
	.	.	V	.
	.	.	V	.
1583	.	.	415 ***	.

1589	.	.	.	135
	.	.	.	V
	.	.	.	V
1595	.	.	.	DAM135
	.	.	.	V

1608
	.	.	426
	.	.	V		.
	.	.	V		.
1611	.	.	400	***	.

1617	.	402
	.	V			.
	.	V			.
1620	.	501			.
	.	V			.
	.	V			.
1623	.	500	***		.
	.	.			.
1631	.	.	.	-----	429
1629	.	.	429		.
	.	.	V		.
	.	.	V		.
1632	.	.	430	***	.

1638	.	.	.		440

1644	.	.	441
	.	.	V		.
	.	.	V		.
1647	.	.	502		.

1650	.	503
	.	V			.
	.	V			.
1653	.	560	***		.
	.	V			.
	.	V			.
1660	.	551			.
	.	.			.
1663	.	.	304		.
	.	.	V		.
	.	.	V		.
1682	.	.	300	***	.
	.	.	V		.
	.	.	V		.
1688	.	.	DAM7		.
	.	.	V		.
	.	.	V		.
1695	.	.	291		.

1698	.	.	.		290
	.	.	.		V
	.	.	.		V
1704	.	.	.		DAM6

1711	.	.	292
	.	.	V		.
	.	.	V		.
1714	.	.	331		.

1717	.	.	.	330	
	.	.	.	V	
	.	.	.	V	
1723	.	.	.	DAM2	
	
	
1730	.	.	332.....		
	.	.	V		
	.	.	V		
1733	.	.	231		
	.	.	.		
	.	.	.		
1736	.	.	.	230	
	
	
1742	.	.	232.....		
	.	.	V		
	.	.	V		
1745	.	.	233		
	.	.	V		
	.	.	V		
1748	.	.	220 ***		
	.	.	V		
	.	.	V		
1755	.	.	445 ***		
	.	.	.		
	.	.	.		
1761	.	.	.	310	
	.	.	.	V	
	.	.	.	V	
1767	.	.	.	DAM4	
	.	.	.	V	
	.	.	.	V	
1774	.	.	.	312	
	
	
1777	320

1783	.	.	.	322.....	
	.	.	.	V	
	.	.	.	V	
1786	.	.	.	DAM3	
	.	.	.	V	
	.	.	.	V	
1793	.	.	.	260 ***	
	.	.	.	V	
	.	.	.	V	
1800	.	.	.	451	
	.	.	.	V	
	.	.	.	V	
1803	.	.	.	450 ***	
	
	
1809	.	.	.	452.....	
	.	.	.	V	
	.	.	.	V	
1812	.	.	.	455 ***	
	
	
1818	.	.	.	280	
	.	.	.	V	
	.	.	.	V	
1824	.	.	.	DAM5	

1831	.	.	.	250 ***	
	.	.	.	V	
	.	.	.	V	
1837	.	.	.	460 ***	
	
	
1843	.	.	461.....		
	.	.	V		
	.	.	V		
1846	.	.	495 ***		
	.	.	.		
	.	.	.		
1852	.	.	.	270	
	.	.	.	V	
	.	.	.	V	
1858	.	.	.	470 ***	
	
	
1866(----- 2223
1864	2223
	V
	V
1867	480 ***

1873	.	.	.	481.....	
	.	.	.	V	
	.	.	.	V	
1876	.	.	.	490 ***	
	
	
1882	.	.	491.....		
	.	.	V		
	.	.	V		
1885	.	.	550 ***		
	.	.	.		
	.	.	.		
1891	.	552.....			
	.	V			
	.	V			
1894	.	545 ***			
	.	.			
	.	.			
1900	.	547.....			

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	1270	12.	13.33	5.	2.	2.	.03		
HYDROGRAPH AT	1280	23.	13.42	11.	3.	3.	.06		
HYDROGRAPH AT	1350	35.	13.50	16.	5.	5.	.09		
HYDROGRAPH AT	1360	47.	13.50	21.	6.	6.	.12		
HYDROGRAPH AT	1440	58.	13.58	27.	8.	8.	.15		
HYDROGRAPH AT	1450	69.	13.58	32.	9.	9.	.18		
HYDROGRAPH AT	1520	80.	13.67	38.	11.	11.	.22		
HYDROGRAPH AT	1530	90.	13.67	43.	13.	13.	.25		
HYDROGRAPH AT	1600	110.	13.75	53.	16.	16.	.31		
ROUTED TO	1591	110.	13.92	53.	15.	15.	.31		
HYDROGRAPH AT	1260	12.	13.42	5.	2.	2.	.03		
HYDROGRAPH AT	1290	24.	13.42	11.	3.	3.	.06		
HYDROGRAPH AT	1340	35.	13.50	17.	5.	5.	.10		
HYDROGRAPH AT	1370	47.	13.58	22.	7.	7.	.13		
HYDROGRAPH AT	1430	58.	13.58	28.	8.	8.	.16		
HYDROGRAPH AT	1460	69.	13.58	33.	10.	10.	.19		
HYDROGRAPH AT	1510	80.	13.67	39.	11.	11.	.22		
HYDROGRAPH AT	1540	91.	13.75	44.	13.	13.	.26		
HYDROGRAPH AT	1590	112.	13.83	55.	16.	16.	.32		
2 COMBINED AT	1592	221.	13.92	107.	32.	32.	.63		
ROUTED TO	1581	220.	14.00	107.	31.	31.	.63		
HYDROGRAPH AT	1250	12.	13.33	5.	2.	2.	.03		
HYDROGRAPH AT	1300	23.	13.33	11.	3.	3.	.06		
HYDROGRAPH AT	1330	35.	13.42	16.	5.	5.	.09		
HYDROGRAPH AT	1380	46.	13.50	21.	6.	6.	.12		
HYDROGRAPH AT	1420	58.	13.42	26.	8.	8.	.15		
HYDROGRAPH AT	1470	69.	13.58	32.	9.	9.	.18		
HYDROGRAPH AT	1500	79.	13.58	37.	11.	11.	.21		

HYDROGRAPH AT	1580	110.	13.75	52.	15.	15.	.30		
2 COMBINED AT	1582	327.	13.92	158.	47.	47.	.93		
ROUTED TO	1571	326.	14.00	157.	46.	46.	.93		
HYDROGRAPH AT	1240	12.	13.33	6.	2.	2.	.03		
HYDROGRAPH AT	1310	24.	13.33	11.	3.	3.	.06		
HYDROGRAPH AT	1320	36.	13.33	17.	5.	5.	.09		
HYDROGRAPH AT	1400	38.	13.33	19.	5.	5.	.10		
HYDROGRAPH AT	1390	11.	13.08	4.	1.	1.	.02		
ROUTED TO	1391	11.	13.17	4.	1.	1.	.02		
2 COMBINED AT	1392	49.	13.25	23.	7.	7.	.13		
HYDROGRAPH AT	1410	61.	13.33	28.	8.	8.	.16		
HYDROGRAPH AT	1480	73.	13.33	34.	10.	10.	.19		
HYDROGRAPH AT	1490	85.	13.33	39.	11.	11.	.22		
HYDROGRAPH AT	1560	97.	13.42	45.	13.	13.	.25		
HYDROGRAPH AT	1570	120.	13.42	56.	16.	16.	.31		
2 COMBINED AT	1572	430.	13.92	211.	63.	63.	1.25		
ROUTED TO	1611	429.	14.00	211.	63.	63.	1.25		
HYDROGRAPH AT	1610	86.	13.83	42.	13.	13.	.25		
2 COMBINED AT	1612	515.	14.00	253.	75.	75.	1.50		
ROUTED TO	1621	515.	14.00	253.	75.	75.	1.50		
HYDROGRAPH AT	1620	99.	13.75	47.	14.	14.	.25		
HYDROGRAPH AT	1630	64.	13.17	23.	7.	7.	.11		
HYDROGRAPH AT	1640	67.	13.50	28.	8.	8.	.14		
3 COMBINED AT	1622	668.	13.92	327.	97.	97.	1.89		
ROUTED TO	1887	532.	14.75	303.	92.	92.	1.89	387.61	14.75
HYDROGRAPH AT	1950	20.	13.17	7.	2.	2.	.04		
HYDROGRAPH AT	1945	35.	13.33	15.	4.	4.	.08		
ROUTED TO	1951	35.	13.50	15.	4.	4.	.08		
2 COMBINED AT	1952	54.	13.33	23.	7.	7.	.12		
HYDROGRAPH AT	1940	106.	13.50	48.	14.	14.	.24		
HYDROGRAPH AT	1920	103.	13.50	43.	12.	12.	.20		
ROUTED TO	1941	103.	13.58	43.	12.	12.	.20		

HYDROGRAPH AT	1925	239.	13.67	105.	31.	31.	.50		
HYDROGRAPH AT	1930	257.	13.92	119.	35.	35.	.57		
HYDROGRAPH AT	1910	71.	13.42	30.	9.	9.	.14		
HYDROGRAPH AT	1900	127.	13.58	57.	17.	17.	.27		
HYDROGRAPH AT	1890	167.	13.75	79.	23.	23.	.36		
ROUTED TO	1931	167.	13.75	78.	23.	23.	.36		
2 COMBINED AT	1932	423.	13.83	197.	58.	58.	.93		
HYDROGRAPH AT	1885	434.	14.08	208.	62.	62.	1.00		
HYDROGRAPH AT	1850	75.	13.33	31.	9.	9.	.14		
HYDROGRAPH AT	1860	130.	13.58	57.	17.	17.	.26		
HYDROGRAPH AT	1870	190.	13.92	96.	28.	28.	.44		
ROUTED TO	1886	190.	14.00	96.	28.	28.	.44		
HYDROGRAPH AT	1880	34.	13.08	13.	4.	4.	.05		
4 COMBINED AT	1888	1106.	14.33	612.	186.	186.	3.38		
ROUTED TO	1984	1103.	14.42	611.	184.	184.	3.38	389.07	14.42
HYDROGRAPH AT	1840	15.	13.33	7.	2.	2.	.03		
HYDROGRAPH AT	1670	39.	13.17	14.	4.	4.	.08		
ROUTED TO	1681	39.	13.33	14.	4.	4.	.08		
HYDROGRAPH AT	1680	39.	13.17	14.	4.	4.	.08		
2 COMBINED AT	1682	78.	13.25	27.	8.	8.	.15		
ROUTED TO	1751	77.	13.33	27.	8.	8.	.15		
HYDROGRAPH AT	1226	437.	12.00	357.	129.	129.	.00		
DIVERSION TO	1228	371.	12.00	304.	110.	110.	.00		
HYDROGRAPH AT	1227	66.	12.00	54.	19.	19.	.00		
HYDROGRAPH AT	1225	244.	12.25	67.	23.	23.	.08		
HYDROGRAPH AT	1230	721.	12.50	175.	53.	53.	.72		
ROUTED TO	1221	717.	12.67	179.	54.	54.	.72		
HYDROGRAPH AT	1214	49.	12.17	4.	1.	1.	.01		
HYDROGRAPH AT	1217	1348.	12.25	129.	36.	36.	.74		
HYDROGRAPH AT	1220	1534.	12.50	241.	67.	67.	1.41		
ROUTED TO	1222	1492.	12.58	241.	67.	67.	1.41		
2 COMBINED AT	1223	2173.	12.58	420.	121.	121.	2.13		

2 COMBINED AT	1752	2064.	12.75	459.	132.	132.	2.36
ROUTED TO	1761	2040.	12.83	459.	132.	132.	2.36
HYDROGRAPH AT	1760	31.	13.00	9.	3.	3.	.05
2 COMBINED AT	1762	2067.	12.83	468.	135.	135.	2.41
ROUTED TO	1771	2046.	12.83	468.	135.	135.	2.41
HYDROGRAPH AT	1770	24.	13.00	7.	2.	2.	.04
2 COMBINED AT	1772	2069.	12.83	474.	137.	137.	2.44
ROUTED TO	1841	2057.	13.00	485.	139.	139.	2.44
2 COMBINED AT	1842	2071.	13.00	492.	141.	141.	2.47
HYDROGRAPH AT	1830	2102.	13.08	514.	148.	148.	2.59
HYDROGRAPH AT	1660	42.	13.25	16.	5.	5.	.09
ROUTED TO	1691	42.	13.50	16.	5.	5.	.09
HYDROGRAPH AT	1690	43.	13.25	16.	5.	5.	.09
2 COMBINED AT	1692	84.	13.42	32.	9.	9.	.18
ROUTED TO	1741	83.	13.50	32.	9.	9.	.18
HYDROGRAPH AT	1740	42.	13.25	16.	5.	5.	.09
2 COMBINED AT	1742	124.	13.42	48.	14.	14.	.27
ROUTED TO	1781	124.	13.58	48.	14.	14.	.27
HYDROGRAPH AT	1780	51.	13.17	18.	5.	5.	.09
2 COMBINED AT	1782	169.	13.42	66.	19.	19.	.36
ROUTED TO	1831	168.	13.58	65.	19.	19.	.36
2 COMBINED AT	1832	2189.	13.08	580.	167.	167.	2.95
HYDROGRAPH AT	1820	2222.	13.08	600.	174.	174.	3.06
HYDROGRAPH AT	1650	42.	13.25	16.	5.	5.	.09
ROUTED TO	1701	42.	13.50	16.	5.	5.	.09
HYDROGRAPH AT	1700	43.	13.25	16.	5.	5.	.09
2 COMBINED AT	1702	84.	13.42	32.	9.	9.	.18
ROUTED TO	1731	83.	13.50	32.	9.	9.	.18
HYDROGRAPH AT	1730	42.	13.25	16.	5.	5.	.09
2 COMBINED AT	1732	124.	13.42	48.	14.	14.	.27
ROUTED TO	1791	123.	13.58	48.	14.	14.	.27
HYDROGRAPH AT	1790	47.	13.17	17.	5.	5.	.09

ROUTED TO	1822	166.	13.58	65.	19.	19.	.36		
2 COMBINED AT	1823	2319.	13.17	665.	193.	193.	3.43		
HYDROGRAPH AT	1810	2373.	13.17	689.	200.	200.	3.54		
HYDROGRAPH AT	1210	725.	12.58	155.	44.	44.	.93		
ROUTED TO	1201	725.	12.75	157.	44.	44.	.93		
HYDROGRAPH AT	1207	387.	12.17	30.	9.	9.	.12		
HYDROGRAPH AT	1204	1332.	12.33	159.	44.	44.	.75		
HYDROGRAPH AT	1200	1247.	12.67	251.	70.	70.	1.31		
2 COMBINED AT	1202	1920.	12.67	408.	114.	114.	2.24		
ROUTED TO	1191	1917.	12.75	406.	114.	114.	2.24		
HYDROGRAPH AT	1186	352.	12.25	27.	8.	8.	.12		
HYDROGRAPH AT	1187	563.	12.33	62.	17.	17.	.31		
HYDROGRAPH AT	1190	673.	12.75	180.	51.	51.	1.03		
2 COMBINED AT	1192	2590.	12.75	585.	165.	165.	3.27		
ROUTED TO	1181	2574.	12.83	589.	165.	165.	3.27		
HYDROGRAPH AT	1173	37.	11.00	25.	7.	7.	.00		
HYDROGRAPH AT	1174	602.	12.25	70.	20.	20.	.21		
HYDROGRAPH AT	1177	986.	12.33	114.	32.	32.	.45		
HYDROGRAPH AT	1180	880.	12.75	229.	65.	65.	1.15		
HYDROGRAPH AT	1000	30.	12.33	5.	1.	1.	.02		
ROUTED TO	1042	30.	12.42	5.	1.	1.	.02		
HYDROGRAPH AT	1060	41.	12.92	11.	3.	3.	.04		
ROUTED TO	1051	41.	13.00	11.	3.	3.	.04		
HYDROGRAPH AT	1050	3.	12.25	0.	0.	0.	.00		
2 COMBINED AT	1052	41.	13.00	11.	3.	3.	.04		
ROUTED TO	1041	41.	13.08	11.	3.	3.	.04		
HYDROGRAPH AT	1040	104.	12.25	10.	3.	3.	.04		
3 COMBINED AT	1043	131.	12.25	26.	8.	8.	.10		
ROUTED TO	DAM8	120.	12.42	21.	6.	6.	.10	102.22	12.42
ROUTED TO	1011	125.	12.50	22.	6.	6.	.10		
HYDROGRAPH AT	1010	111.	12.25	10.	3.	3.	.04		
2 COMBINED AT	1012	162.	12.50	31.	9.	9.	.13		

HYDROGRAPH AT	1020	108.	12.17	9.	3.	3.	.03
HYDROGRAPH AT	1120	143.	12.25	15.	4.	4.	.07
2 COMBINED AT	1121	250.	12.33	47.	14.	14.	.21
DIVERSION TO	2223	100.	12.33	19.	5.	5.	.21
HYDROGRAPH AT	2222	150.	12.33	28.	8.	8.	.21
ROUTED TO	1131	150.	12.33	28.	8.	8.	.21
HYDROGRAPH AT	1070	105.	12.25	10.	3.	3.	.04
ROUTED TO	1031	100.	12.33	10.	3.	3.	.04
HYDROGRAPH AT	1030	77.	12.33	7.	2.	2.	.03
2 COMBINED AT	1032	177.	12.33	17.	5.	5.	.07
HYDROGRAPH AT	1130	254.	12.33	29.	8.	8.	.13
2 COMBINED AT	1132	404.	12.33	57.	16.	16.	.35
DIVERSION TO	3334	202.	12.33	29.	8.	8.	.35
HYDROGRAPH AT	3333	202.	12.33	29.	8.	8.	.35
ROUTED TO	1151	185.	12.50	29.	8.	8.	.35
HYDROGRAPH AT	1080	238.	12.25	19.	6.	6.	.08
HYDROGRAPH AT	1090	313.	12.25	32.	9.	9.	.14
HYDROGRAPH AT	1150	416.	12.50	72.	21.	21.	.37
2 COMBINED AT	1152	602.	12.50	101.	29.	29.	.72
DIVERSION TO	4445	301.	12.50	50.	14.	14.	.72
HYDROGRAPH AT	4444	301.	12.50	50.	14.	14.	.72
ROUTED TO	1161	298.	12.67	52.	15.	15.	.72
HYDROGRAPH AT	1159	834.	13.00	817.	538.	538.	.00
HYDROGRAPH AT	1154	1328.	12.25	859.	547.	547.	.16
HYDROGRAPH AT	1155	2170.	12.25	942.	565.	565.	.62
HYDROGRAPH AT	1153	199.	12.17	16.	5.	5.	.08
ROUTED TO	1101	220.	12.25	18.	5.	5.	.08
HYDROGRAPH AT	1100	13.	12.25	1.	0.	0.	.00
2 COMBINED AT	1102	233.	12.25	19.	5.	5.	.08
ROUTED TO	1156	239.	12.33	21.	6.	6.	.08
2 COMBINED AT	1157	2284.	12.33	963.	571.	571.	.70
HYDROGRAPH AT	1160	2153.	12.50	1014.	574.	574.	1.00

HYDROGRAPH AT	1165	1165.	12.25	103.	29.	29.	.50		
HYDROGRAPH AT	1170	845.	12.58	146.	41.	41.	.76		
ROUTED TO	1162	839.	12.67	147.	41.	41.	.76		
3 COMBINED AT	1163	3213.	12.58	1212.	629.	629.	2.48		
DIVERSION TO	5556	1607.	12.58	606.	315.	315.	2.48		
HYDROGRAPH AT	5555	1607.	12.58	606.	315.	315.	2.48		
ROUTED TO	1182	1585.	12.67	606.	314.	314.	2.48		
2 COMBINED AT	1183	2454.	12.67	835.	379.	379.	3.63		
ROUTED TO	1184	2425.	12.75	835.	378.	378.	3.63		
2 COMBINED AT	1185	4980.	12.75	1423.	543.	543.	6.90		
ROUTED TO	1646	4916.	12.83	1423.	542.	542.	6.90		
HYDROGRAPH AT	1645	67.	12.92	20.	6.	6.	.09		
2 COMBINED AT	1647	4981.	12.83	1442.	548.	548.	6.99		
ROUTED TO	1711	4978.	12.83	1442.	547.	547.	6.99		
HYDROGRAPH AT	1710	53.	13.17	19.	5.	5.	.09		
2 COMBINED AT	1712	5020.	12.83	1461.	552.	552.	7.09		
ROUTED TO	1721	4969.	12.83	1461.	551.	551.	7.09		
HYDROGRAPH AT	1720	59.	13.08	20.	6.	6.	.09		
2 COMBINED AT	1722	5019.	12.83	1481.	557.	557.	7.18		
ROUTED TO	1801	4999.	12.92	1481.	556.	556.	7.18		
HYDROGRAPH AT	1800	65.	13.08	22.	6.	6.	.09		
2 COMBINED AT	1802	5060.	12.92	1502.	562.	562.	7.27		
ROUTED TO	1811	5023.	12.92	1502.	561.	561.	7.27		
2 COMBINED AT	1812	7025.	13.08	2190.	761.	761.	10.82		
ROUTED TO	1813	7018.	13.08	2190.	760.	760.	10.82		
2 COMBINED AT	1985	7241.	13.08	2786.	944.	944.	14.20		
ROUTED TO	1986	6352.	13.33	2780.	938.	938.	14.20	395.88	13.33
HYDROGRAPH AT	3334	202.	12.33	29.	8.	8.	.00		
HYDROGRAPH AT	1135	267.	12.58	50.	14.	14.	.11		
ROUTED TO	1136	266.	12.58	50.	14.	14.	.11		
HYDROGRAPH AT	4445	301.	12.50	50.	14.	14.	.00		
HYDROGRAPH AT	1140	400.	12.67	76.	22.	22.	.14		

ROUTED TO	1971	652.	12.83	129.	36.	36.	.25		
HYDROGRAPH AT	5556	1607.	12.58	606.	315.	315.	.00		
ROUTED TO	1972	1530.	12.75	606.	310.	310.	.00		
HYDROGRAPH AT	1970	375.	12.83	98.	28.	28.	.51		
3 COMBINED AT	1973	2544.	12.75	832.	374.	374.	.76		
HYDROGRAPH AT	1980	2781.	12.92	937.	398.	398.	1.23		
2 COMBINED AT	1987	8384.	13.25	3701.	1336.	1336.	15.43		
ROUTED TO	1991	7881.	13.50	3697.	1328.	1328.	15.43	396.89	13.50
HYDROGRAPH AT	1960	326.	12.83	85.	24.	24.	.38		
HYDROGRAPH AT	1990	591.	12.92	169.	48.	48.	.76		
2 COMBINED AT	1993	8248.	13.50	3859.	1376.	1376.	16.19		
ROUTED TO	2001	8123.	13.58	3857.	1372.	1372.	16.19	397.04	13.58
HYDROGRAPH AT	2000	112.	12.67	26.	7.	7.	.12		
2 COMBINED AT	2002	8160.	13.58	3879.	1379.	1379.	16.31		
ROUTED TO	541	8095.	13.67	3878.	1375.	1375.	16.31	397.02	13.67
HYDROGRAPH AT	540	116.	12.92	33.	10.	10.	.21		
2 COMBINED AT	542	8154.	13.67	3910.	1385.	1385.	16.51		
ROUTED TO	546	8120.	13.75	3910.	1382.	1382.	16.51	397.03	13.75
HYDROGRAPH AT	165	51.	12.25	4.	1.	1.	.01		
ROUTED TO	DAM165	53.	12.25	4.	1.	1.	.01	102.12	12.25
ROUTED TO	156	47.	12.33	4.	1.	1.	.01		
HYDROGRAPH AT	160	89.	12.25	7.	2.	2.	.02		
ROUTED TO	DAM160	86.	12.25	7.	2.	2.	.02	102.17	12.25
ROUTED TO	161	72.	12.33	7.	2.	2.	.02		
HYDROGRAPH AT	155	96.	12.25	8.	2.	2.	.03		
ROUTED TO	DAM155	91.	12.25	7.	2.	2.	.03	102.18	12.25
3 COMBINED AT	157	202.	12.25	18.	5.	5.	.06		
ROUTED TO	151	198.	12.25	18.	5.	5.	.06		
HYDROGRAPH AT	150	161.	12.25	14.	4.	4.	.04		
ROUTED TO	DAM150	155.	12.25	12.	3.	3.	.04	102.26	12.25
2 COMBINED AT	152	353.	12.25	30.	8.	8.	.10		
ROUTED TO	121	329.	12.33	30.	8.	8.	.10		

ROUTED TO	122	17.	12.42	2.	1.	1.	.01		
HYDROGRAPH AT	120	49.	12.25	5.	1.	1.	.01		
ROUTED TO	DAM120	47.	12.17	4.	1.	1.	.01	102.11	12.17
3 COMBINED AT	123	384.	12.33	36.	10.	10.	.13		
HYDROGRAPH AT	75	384.	12.33	37.	10.	10.	.13		
ROUTED TO	DAM75	384.	12.33	37.	10.	10.	.13	102.47	12.33
HYDROGRAPH AT	85	378.	12.33	38.	10.	10.	.14		
ROUTED TO	DAM85	377.	12.33	38.	10.	10.	.14	102.47	12.33
HYDROGRAPH AT	80	28.	12.17	2.	1.	1.	.01		
ROUTED TO	DAM80	29.	12.17	2.	1.	1.	.01	102.08	12.17
ROUTED TO	87	24.	12.25	2.	1.	1.	.01		
2 COMBINED AT	88	394.	12.33	40.	11.	11.	.14		
ROUTED TO	96	369.	12.33	40.	11.	11.	.14		
HYDROGRAPH AT	95	41.	12.25	4.	1.	1.	.01		
ROUTED TO	DAM95	41.	12.17	4.	1.	1.	.01	102.10	12.17
2 COMBINED AT	97	405.	12.33	44.	12.	12.	.15		
ROUTED TO	91	387.	12.42	44.	12.	12.	.15		
HYDROGRAPH AT	90	50.	12.25	4.	1.	1.	.02		
ROUTED TO	DAM90	52.	12.25	4.	1.	1.	.02	102.12	12.25
2 COMBINED AT	92	408.	12.42	48.	13.	13.	.17		
HYDROGRAPH AT	110	59.	12.25	5.	2.	2.	.02		
ROUTED TO	DAM110	63.	12.25	5.	1.	1.	.02	102.14	12.25
ROUTED TO	101	56.	12.33	5.	1.	1.	.02		
HYDROGRAPH AT	100	120.	12.25	10.	3.	3.	.00		
ROUTED TO	DAM100	120.	12.25	10.	3.	3.	.00	102.22	12.25
3 COMBINED AT	102	518.	12.33	62.	17.	17.	.19		
ROUTED TO	103	514.	12.33	62.	17.	17.	.19		
HYDROGRAPH AT	360	507.	12.33	66.	18.	18.	.21		
HYDROGRAPH AT	410	549.	12.42	72.	20.	20.	.25		
ROUTED TO	421	548.	12.42	72.	20.	20.	.25		
HYDROGRAPH AT	140	81.	12.25	8.	2.	2.	.03		
ROUTED TO	DAM140	88.	12.25	7.	2.	2.	.03	102.17	12.25

ROUTED TO	DAM130	163.	12.25	17.	5.	5.	.06	102.27	12.25
HYDROGRAPH AT	420	208.	12.33	34.	9.	9.	.15		
2 COMBINED AT	422	755.	12.42	106.	29.	29.	.40		
ROUTED TO	401	750.	12.42	106.	29.	29.	.40		
HYDROGRAPH AT	240	206.	12.25	17.	5.	5.	.07		
ROUTED TO	246	182.	12.25	17.	5.	5.	.07		
HYDROGRAPH AT	245	67.	12.25	5.	2.	2.	.02		
2 COMBINED AT	247	249.	12.25	22.	6.	6.	.10		
ROUTED TO	248	226.	12.33	22.	6.	6.	.10		
HYDROGRAPH AT	180	92.	12.33	11.	3.	3.	.07		
HYDROGRAPH AT	255	78.	12.25	7.	2.	2.	.03		
ROUTED TO	236	72.	12.33	7.	2.	2.	.03		
HYDROGRAPH AT	235	39.	12.25	3.	1.	1.	.01		
2 COMBINED AT	237	98.	12.33	10.	3.	3.	.04		
ROUTED TO	181	93.	12.42	10.	3.	3.	.04		
3 COMBINED AT	182	404.	12.33	43.	13.	13.	.21		
ROUTED TO	191	375.	12.33	43.	13.	13.	.21		
HYDROGRAPH AT	195	161.	12.17	13.	4.	4.	.04		
ROUTED TO	186	146.	12.25	13.	4.	4.	.04		
HYDROGRAPH AT	185	96.	12.17	8.	2.	2.	.02		
2 COMBINED AT	187	229.	12.25	20.	6.	6.	.06		
ROUTED TO	188	206.	12.33	21.	6.	6.	.06		
2 COMBINED AT	189	581.	12.33	63.	19.	19.	.27		
ROUTED TO	DAM1	449.	12.50	47.	13.	13.	.27	102.53	12.50
HYDROGRAPH AT	190	20.	12.67	5.	2.	2.	.03		
2 COMBINED AT	192	467.	12.50	52.	15.	15.	.30		
ROUTED TO	211	430.	12.58	54.	15.	15.	.30		
HYDROGRAPH AT	210	67.	12.25	5.	2.	2.	.02		
2 COMBINED AT	212	449.	12.50	59.	17.	17.	.32		
DIVERSION TO	429	224.	12.50	29.	8.	8.	.32		
HYDROGRAPH AT	213	224.	12.50	29.	8.	8.	.32		
HYDROGRAPH AT	405	221.	12.67	32.	9.	9.	.34		

ROUTED TO	DAM200	67.	12.33	8.	2.	2.	.04	102.14	12.33
HYDROGRAPH AT	415	75.	12.33	9.	2.	2.	.05		
HYDROGRAPH AT	135	77.	12.33	8.	2.	2.	.03		
ROUTED TO	DAM135	78.	12.25	7.	2.	2.	.03	102.16	12.25
HYDROGRAPH AT	425	87.	12.33	9.	2.	2.	.04		
3 COMBINED AT	426	287.	12.58	50.	14.	14.	.42		
HYDROGRAPH AT	400	401.	12.75	78.	22.	22.	.57		
2 COMBINED AT	402	997.	12.50	184.	51.	51.	.97		
ROUTED TO	501	996.	12.50	184.	51.	51.	.97		
HYDROGRAPH AT	500	1103.	12.50	211.	59.	59.	1.09		
HYDROGRAPH AT	429	224.	12.50	29.	8.	8.	.00		
HYDROGRAPH AT	430	305.	12.75	67.	19.	19.	.18		
HYDROGRAPH AT	440	89.	12.67	21.	6.	6.	.10		
2 COMBINED AT	441	392.	12.75	87.	25.	25.	.29		
ROUTED TO	502	387.	12.92	88.	25.	25.	.29		
2 COMBINED AT	503	1314.	12.58	298.	83.	83.	1.38		
HYDROGRAPH AT	560	1378.	12.58	313.	88.	88.	1.46		
ROUTED TO	551	1374.	12.67	313.	88.	88.	1.46		
HYDROGRAPH AT	304	336.	12.67	309.	206.	206.	.00		
HYDROGRAPH AT	300	389.	12.33	316.	208.	208.	.03		
ROUTED TO	DAM7	391.	12.33	316.	208.	208.	.03	102.48	12.33
ROUTED TO	291	388.	12.33	316.	208.	208.	.03		
HYDROGRAPH AT	290	65.	12.17	5.	2.	2.	.02		
ROUTED TO	DAM6	55.	12.33	3.	1.	1.	.02	102.13	12.33
2 COMBINED AT	292	443.	12.33	319.	209.	209.	.05		
ROUTED TO	331	426.	12.33	319.	209.	209.	.05		
HYDROGRAPH AT	330	71.	12.17	6.	2.	2.	.02		
ROUTED TO	DAM2	56.	12.33	4.	1.	1.	.02	102.13	12.33
2 COMBINED AT	332	482.	12.33	323.	210.	210.	.07		
ROUTED TO	231	470.	12.33	323.	210.	210.	.07		
HYDROGRAPH AT	230	138.	12.25	11.	3.	3.	.04		
2 COMBINED AT	232	561.	12.33	334.	214.	214.	.11		

HYDROGRAPH AT	220	713.	12.25	354.	220.	220.	.19		
HYDROGRAPH AT	445	727.	12.33	358.	221.	221.	.21		
HYDROGRAPH AT	310	40.	12.42	7.	2.	2.	.03		
ROUTED TO	DAM4	1.	20.25	0.	0.	0.	.03	102.00	20.33
ROUTED TO	312	1.	20.42	0.	0.	0.	.03		
HYDROGRAPH AT	320	67.	12.17	6.	2.	2.	.02		
2 COMBINED AT	322	67.	12.17	6.	2.	2.	.05		
ROUTED TO	DAM3	56.	12.33	3.	1.	1.	.05	102.13	12.33
HYDROGRAPH AT	260	133.	12.25	14.	4.	4.	.09		
ROUTED TO	451	130.	12.33	14.	4.	4.	.09		
HYDROGRAPH AT	450	151.	12.42	18.	5.	5.	.12		
2 COMBINED AT	452	869.	12.33	376.	227.	227.	.33		
HYDROGRAPH AT	455	927.	12.50	389.	230.	230.	.39		
HYDROGRAPH AT	280	224.	12.25	19.	6.	6.	.06		
ROUTED TO	DAM5	102.	12.42	10.	3.	3.	.06	102.19	12.42
HYDROGRAPH AT	250	143.	12.50	22.	6.	6.	.11		
HYDROGRAPH AT	460	241.	12.67	45.	13.	13.	.24		
2 COMBINED AT	461	1130.	12.50	434.	243.	243.	.63		
HYDROGRAPH AT	495	1256.	12.67	470.	252.	252.	.79		
HYDROGRAPH AT	270	173.	12.25	14.	4.	4.	.06		
HYDROGRAPH AT	470	227.	12.33	27.	8.	8.	.13		
HYDROGRAPH AT	2223	100.	12.33	19.	5.	5.	.00		
HYDROGRAPH AT	480	166.	12.42	32.	9.	9.	.08		
2 COMBINED AT	481	387.	12.42	59.	17.	17.	.21		
HYDROGRAPH AT	490	474.	12.67	105.	30.	30.	.41		
2 COMBINED AT	491	1729.	12.67	575.	282.	282.	1.20		
HYDROGRAPH AT	550	1878.	12.75	620.	294.	294.	1.41		
2 COMBINED AT	552	3234.	12.75	933.	381.	381.	2.87		
HYDROGRAPH AT	545	3239.	12.75	947.	385.	385.	2.96		
2 COMBINED AT	547	9326.	13.67	4838.	1768.	1768.	19.48		

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM8

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	4.	4.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.22	.22	4.	120.	11.75	12.42	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM165

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.12	.12	1.	53.	5.00	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM160

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.17	.17	1.	86.	8.08	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM155

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.18	.18	1.	91.	8.92	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM150

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.26	.26	2.	155.	12.00	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM120

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.11	.11	1.	47.	5.58	12.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM75

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.47	.47	0.	384.	12.08	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM85

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.47	.47	0.	377.	12.08	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM80

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.08	.08	0.	29.	2.42	12.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM95

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S. ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.10	.10	1.	41.	4.75	12.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM90

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.12	.12	1.	52.	5.08	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM110

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.14	.14	1.	63.	5.67	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM100

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.22	.22	0.	120.	15.25	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM140

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.17	.17	1.	88.	9.33	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM130

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.27	.27	1.	163.	12.08	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	11.	11.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.53	.53	14.	449.	11.75	12.50	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM200

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.14	.14	0.	67.	11.92	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM135

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.16	.16	1.	78.	9.08	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM7

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.48	.48	1.	391.	24.00	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM6

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.13	.13	2.	55.	5.33	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM2

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.13	.13	2.	56.	6.25	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM4

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	4.	4.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.00	.00	4.	1.	2.50	20.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM3

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.13	.13	2.	56.	10.50	12.33	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM5

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	6.	6.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.19	.19	6.	102.	11.75	12.42	.00

*** NORMAL END OF HEC-1 ***

APPENDIX B

+	HYDROGRAPH AT	1530	66.	13.92	34.	10.	10.	.25
+	HYDROGRAPH AT	1600	81.	14.00	42.	13.	13.	.31
+	ROUTED TO	1591	80.	14.17	42.	12.	12.	.31
+	HYDROGRAPH AT	1260	8.	13.58	4.	1.	1.	.03
+	HYDROGRAPH AT	1290	17.	13.67	9.	3.	3.	.06
+	HYDROGRAPH AT	1340	26.	13.67	13.	4.	4.	.10
+	HYDROGRAPH AT	1370	34.	13.75	18.	5.	5.	.13
+	HYDROGRAPH AT	1430	42.	13.83	22.	7.	7.	.16
+	HYDROGRAPH AT	1460	50.	13.75	26.	8.	8.	.19
+	HYDROGRAPH AT	1510	58.	13.83	31.	9.	9.	.22
+	HYDROGRAPH AT	1540	66.	13.92	35.	11.	11.	.26
+	HYDROGRAPH AT	1590	81.	14.00	43.	13.	13.	.32
+	2 COMBINED AT	1592	161.	14.08	85.	26.	26.	.63
+	ROUTED TO	1581	161.	14.25	85.	25.	25.	.63
+	HYDROGRAPH AT	1250	8.	13.50	4.	1.	1.	.03
+	HYDROGRAPH AT	1300	17.	13.58	8.	3.	3.	.06
+	HYDROGRAPH AT	1330	25.	13.58	13.	4.	4.	.09
+	HYDROGRAPH AT	1380	34.	13.67	17.	5.	5.	.12
+	HYDROGRAPH AT	1420	42.	13.67	21.	6.	6.	.15
+	HYDROGRAPH AT	1470	50.	13.75	25.	8.	8.	.18
+	HYDROGRAPH AT	1500	58.	13.83	29.	9.	9.	.21
	HYDROGRAPH AT							

+	HYDROGRAPH AT	1580	80.	13.92	42.	13.	13.	.30
+	2 COMBINED AT	1582	239.	14.17	125.	38.	38.	.93
+	ROUTED TO	1571	239.	14.25	125.	38.	38.	.93
+	HYDROGRAPH AT	1240	9.	13.50	4.	1.	1.	.03
+	HYDROGRAPH AT	1310	17.	13.58	9.	3.	3.	.06
+	HYDROGRAPH AT	1320	26.	13.58	13.	4.	4.	.09
+	HYDROGRAPH AT	1400	27.	13.58	15.	4.	4.	.10
+	HYDROGRAPH AT	1390	8.	13.25	3.	1.	1.	.02
+	ROUTED TO	1391	8.	13.33	3.	1.	1.	.02
+	2 COMBINED AT	1392	35.	13.50	18.	5.	5.	.13
+	HYDROGRAPH AT	1410	44.	13.50	23.	7.	7.	.16
+	HYDROGRAPH AT	1480	52.	13.58	27.	8.	8.	.19
+	HYDROGRAPH AT	1490	61.	13.58	32.	9.	9.	.22
+	HYDROGRAPH AT	1560	69.	13.58	36.	11.	11.	.25
+	HYDROGRAPH AT	1570	86.	13.67	45.	13.	13.	.31
+	2 COMBINED AT	1572	315.	14.17	167.	51.	51.	1.25
+	ROUTED TO	1611	315.	14.17	167.	51.	51.	1.25
+	HYDROGRAPH AT	1610	63.	14.08	33.	10.	10.	.25
+	2 COMBINED AT	1612	377.	14.17	200.	61.	61.	1.50
+	ROUTED TO	1621	377.	14.25	200.	61.	61.	1.50
+	HYDROGRAPH AT	1620	72.	14.00	37.	11.	11.	.25
	HYDROGRAPH AT							

+	HYDROGRAPH AT	1640	50.	13.67	23.	7.	7.	.14		
+	3 COMBINED AT	1622	491.	14.17	260.	79.	79.	1.89		
+	ROUTED TO	1887	390.	15.00	239.	74.	74.	1.89		
+									387.16	15.00
+	HYDROGRAPH AT	1950	14.	13.33	6.	2.	2.	.04		
+	HYDROGRAPH AT	1945	25.	13.58	12.	4.	4.	.08		
+	ROUTED TO	1951	25.	13.67	13.	4.	4.	.08		
+	2 COMBINED AT	1952	39.	13.50	18.	5.	5.	.12		
+	HYDROGRAPH AT	1940	80.	13.67	39.	12.	12.	.24		
+	HYDROGRAPH AT	1920	77.	13.58	35.	10.	10.	.20		
+	ROUTED TO	1941	77.	13.75	35.	10.	10.	.20		
+	2 COMBINED AT	1942	157.	13.75	75.	22.	22.	.43		
+	HYDROGRAPH AT	1925	179.	13.83	86.	25.	25.	.50		
+	HYDROGRAPH AT	1930	194.	14.08	97.	29.	29.	.57		
+	HYDROGRAPH AT	1910	54.	13.58	25.	7.	7.	.14		
+	HYDROGRAPH AT	1900	97.	13.75	47.	14.	14.	.27		
+	HYDROGRAPH AT	1890	128.	13.92	65.	19.	19.	.36		
+	ROUTED TO	1931	127.	13.92	65.	19.	19.	.36		
+	2 COMBINED AT	1932	321.	14.00	161.	48.	48.	.93		
+	HYDROGRAPH AT	1885	329.	14.25	171.	51.	51.	1.00		
+	HYDROGRAPH AT	1850	57.	13.50	25.	7.	7.	.14		
+	HYDROGRAPH AT	1860	99.	13.75	47.	14.	14.	.26		

+		1750	1536.	12.83	358.	105.	105.	2.21
	2 COMBINED AT							
+		1752	1556.	12.83	380.	111.	111.	2.36
	ROUTED TO							
+		1761	1533.	12.92	379.	111.	111.	2.36
	HYDROGRAPH AT							
+		1760	22.	13.17	7.	2.	2.	.05
	2 COMBINED AT							
+		1762	1553.	12.92	387.	113.	113.	2.41
	ROUTED TO							
+		1771	1545.	12.92	387.	113.	113.	2.41
	HYDROGRAPH AT							
+		1770	18.	13.08	5.	2.	2.	.04
	2 COMBINED AT							
+		1772	1562.	12.92	392.	115.	115.	2.44
	ROUTED TO							
+		1841	1557.	13.08	398.	116.	116.	2.44
	2 COMBINED AT							
+		1842	1568.	13.08	404.	117.	117.	2.47
	HYDROGRAPH AT							
+		1830	1597.	13.17	423.	123.	123.	2.59
	HYDROGRAPH AT							
+		1660	30.	13.42	13.	4.	4.	.09
	ROUTED TO							
+		1691	30.	13.58	13.	4.	4.	.09
	HYDROGRAPH AT							
+		1690	31.	13.42	13.	4.	4.	.09
	2 COMBINED AT							
+		1692	61.	13.50	26.	8.	8.	.18
	ROUTED TO							
+		1741	60.	13.67	26.	8.	8.	.18
	HYDROGRAPH AT							
+		1740	31.	13.42	13.	4.	4.	.09
	2 COMBINED AT							
+		1742	90.	13.58	38.	11.	11.	.27
	ROUTED TO							
+		1781	90.	13.75	38.	11.	11.	.27
	HYDROGRAPH AT							
+		1780	37.	13.33	15.	4.	4.	.09
	2 COMBINED AT							
+		1782	123.	13.67	53.	16.	16.	.36
	ROUTED TO							
+		1831	122.	13.75	52.	15.	15.	.36

+		1832	1654.	13.17	475.	139.	139.	2.95
	HYDROGRAPH AT							
+		1820	1677.	13.17	492.	144.	144.	3.06
	HYDROGRAPH AT							
+		1650	30.	13.42	13.	4.	4.	.09
	ROUTED TO							
+		1701	30.	13.58	13.	4.	4.	.09
	HYDROGRAPH AT							
+		1700	31.	13.42	13.	4.	4.	.09
	2 COMBINED AT							
+		1702	61.	13.50	26.	8.	8.	.18
	ROUTED TO							
+		1731	60.	13.67	26.	8.	8.	.18
	HYDROGRAPH AT							
+		1730	30.	13.42	13.	4.	4.	.09
	2 COMBINED AT							
+		1732	90.	13.58	38.	11.	11.	.27
	ROUTED TO							
+		1791	90.	13.75	38.	11.	11.	.27
	HYDROGRAPH AT							
+		1790	35.	13.33	14.	4.	4.	.09
	2 COMBINED AT							
+		1792	121.	13.67	52.	15.	15.	.36
	ROUTED TO							
+		1822	121.	13.83	52.	15.	15.	.36
	2 COMBINED AT							
+		1823	1744.	13.25	543.	159.	159.	3.43
	HYDROGRAPH AT							
+		1810	1784.	13.25	563.	166.	166.	3.54
	HYDROGRAPH AT							
+		1210	552.	12.67	124.	35.	35.	.93
	ROUTED TO							
+		1201	552.	12.75	125.	35.	35.	.93
	HYDROGRAPH AT							
+		1207	318.	12.17	25.	7.	7.	.12
	HYDROGRAPH AT							
+		1204	1039.	12.33	131.	37.	37.	.75
	HYDROGRAPH AT							
+		1200	916.	12.75	204.	58.	58.	1.31
	2 COMBINED AT							
+		1202	1469.	12.75	328.	93.	93.	2.24
	ROUTED TO							
+		1191	1449.	12.83	327.	92.	92.	2.24

+	HYDROGRAPH AT	1110	106.	12.67	27.	8.	8.	.15
+	HYDROGRAPH AT	1020	89.	12.17	7.	2.	2.	.03
+	HYDROGRAPH AT	1120	113.	12.33	12.	4.	4.	.07
+	2 COMBINED AT	1121	205.	12.33	39.	11.	11.	.21
+	DIVERSION TO	2223	82.	12.33	16.	5.	5.	.21
+	HYDROGRAPH AT	2222	123.	12.33	24.	7.	7.	.21
+	ROUTED TO	1131	119.	12.33	24.	7.	7.	.21
+	HYDROGRAPH AT	1070	84.	12.25	8.	2.	2.	.04
+	ROUTED TO	1031	80.	12.33	8.	2.	2.	.04
+	HYDROGRAPH AT	1030	58.	12.33	6.	2.	2.	.03
+	2 COMBINED AT	1032	138.	12.33	14.	4.	4.	.07
+	HYDROGRAPH AT	1130	195.	12.42	24.	7.	7.	.13
+	2 COMBINED AT	1132	305.	12.42	47.	14.	14.	.35
+	DIVERSION TO	3334	152.	12.42	24.	7.	7.	.35
+	HYDROGRAPH AT	3333	152.	12.42	24.	7.	7.	.35
+	ROUTED TO	1151	148.	12.50	24.	7.	7.	.35
+	HYDROGRAPH AT	1080	200.	12.25	16.	5.	5.	.08
+	HYDROGRAPH AT	1090	257.	12.33	27.	8.	8.	.14
+	HYDROGRAPH AT	1150	312.	12.58	59.	17.	17.	.37
+	2 COMBINED AT	1152	449.	12.58	83.	24.	24.	.72
+	DIVERSION TO	4445	224.	12.58	41.	12.	12.	.72
	HYDROGRAPH AT							

+	ROUTED TO	1161	224.	12.75	42.	12.	12.	.72
+	HYDROGRAPH AT	1159	834.	13.00	817.	538.	538.	.00
+	HYDROGRAPH AT	1154	1253.	12.25	852.	545.	545.	.16
+	HYDROGRAPH AT	1155	1812.	12.25	917.	558.	558.	.62
+	HYDROGRAPH AT	1153	160.	12.25	13.	4.	4.	.08
+	ROUTED TO	1101	169.	12.25	15.	4.	4.	.08
+	HYDROGRAPH AT	1100	9.	12.25	1.	0.	0.	.00
+	2 COMBINED AT	1102	179.	12.25	16.	5.	5.	.08
+	ROUTED TO	1156	162.	12.42	17.	5.	5.	.08
+	2 COMBINED AT	1157	1959.	12.33	934.	563.	563.	.70
+	HYDROGRAPH AT	1160	1825.	12.58	976.	563.	563.	1.00
+	HYDROGRAPH AT	1166	285.	12.25	24.	7.	7.	.11
+	HYDROGRAPH AT	1165	858.	12.25	85.	24.	24.	.50
+	HYDROGRAPH AT	1170	631.	12.58	119.	33.	33.	.76
+	ROUTED TO	1162	615.	12.75	120.	34.	34.	.76
+	3 COMBINED AT	1163	2580.	12.67	1137.	609.	609.	2.48
+	DIVERSION TO	5556	1290.	12.67	568.	305.	305.	2.48
+	HYDROGRAPH AT	5555	1290.	12.67	568.	305.	305.	2.48
+	ROUTED TO	1182	1286.	12.67	568.	304.	304.	2.48
+	2 COMBINED AT	1183	1898.	12.75	755.	357.	357.	3.63
+	ROUTED TO	1184	1893.	12.75	755.	356.	356.	3.63
	2 COMBINED AT							

+	ROUTED TO	1646	3745.	12.83	1227.	489.	489.	6.90		
+	HYDROGRAPH AT	1645	50.	13.08	16.	5.	5.	.09		
+	2 COMBINED AT	1647	3790.	12.83	1243.	494.	494.	6.99		
+	ROUTED TO	1711	3786.	12.92	1242.	493.	493.	6.99		
+	HYDROGRAPH AT	1710	39.	13.33	15.	4.	4.	.09		
+	2 COMBINED AT	1712	3816.	12.92	1258.	497.	497.	7.09		
+	ROUTED TO	1721	3785.	12.92	1258.	496.	496.	7.09		
+	HYDROGRAPH AT	1720	44.	13.25	17.	5.	5.	.09		
+	2 COMBINED AT	1722	3822.	12.92	1274.	501.	501.	7.18		
+	ROUTED TO	1801	3805.	13.00	1274.	500.	500.	7.18		
+	HYDROGRAPH AT	1800	50.	13.17	18.	5.	5.	.09		
+	2 COMBINED AT	1802	3853.	13.00	1292.	505.	505.	7.27		
+	ROUTED TO	1811	3829.	13.00	1292.	504.	504.	7.27		
+	2 COMBINED AT	1812	5358.	13.17	1854.	670.	670.	10.82		
+	ROUTED TO	1813	5349.	13.17	1854.	669.	669.	10.82		
+	2 COMBINED AT	1985	5515.	13.17	2327.	820.	820.	14.20		
+	ROUTED TO	1986	4958.	13.42	2322.	814.	814.	14.20		
+									394.73	13.42
+	HYDROGRAPH AT	3334	152.	12.42	24.	7.	7.	.00		
+	HYDROGRAPH AT	1135	209.	12.58	41.	12.	12.	.11		
+	ROUTED TO	1136	208.	12.67	41.	12.	12.	.11		
+	HYDROGRAPH AT	4445	224.	12.58	41.	12.	12.	.00		

+	2 COMBINED AT	1141	500.	12.67	104.	30.	30.	.25		
	ROUTED TO									
+		1971	498.	12.83	105.	30.	30.	.25		
	HYDROGRAPH AT									
+		5556	1290.	12.67	568.	305.	305.	.00		
	ROUTED TO									
+		1972	1235.	12.83	568.	300.	300.	.00		
	HYDROGRAPH AT									
+		1970	269.	12.92	79.	23.	23.	.51		
	3 COMBINED AT									
+		1973	1998.	12.83	752.	352.	352.	.76		
	HYDROGRAPH AT									
+		1980	2181.	13.00	839.	372.	372.	1.23		
	2 COMBINED AT									
+		1987	6622.	13.33	3146.	1185.	1185.	15.43		
	ROUTED TO									
+		1991	6232.	13.58	3143.	1178.	1178.	15.43		
+									395.79	13.58
	HYDROGRAPH AT									
+		1960	261.	12.83	71.	20.	20.	.38		
	HYDROGRAPH AT									
+		1990	463.	13.00	141.	40.	40.	.76		
	2 COMBINED AT									
+		1993	6533.	13.58	3277.	1218.	1218.	16.19		
	ROUTED TO									
+		2001	6424.	13.67	3275.	1213.	1213.	16.19		
+									395.93	13.67
	HYDROGRAPH AT									
+		2000	85.	12.75	21.	6.	6.	.12		
	2 COMBINED AT									
+		2002	6455.	13.67	3294.	1219.	1219.	16.31		
	ROUTED TO									
+		541	6402.	13.83	3293.	1216.	1216.	16.31		
+									395.92	13.83
	HYDROGRAPH AT									
+		540	82.	13.00	26.	8.	8.	.21		
	2 COMBINED AT									
+		542	6447.	13.83	3318.	1224.	1224.	16.51		
	ROUTED TO									
+		546	6417.	13.92	3318.	1221.	1221.	16.51		
+									395.93	13.92
	HYDROGRAPH AT									
+		165	39.	12.17	4.	1.	1.	.01		

+		DAM165	45.	12.17	3.	1.	1.	.01		
+									102.11	12.17
		ROUTED TO								
+			156	12.25	3.	1.	1.	.01		
		HYDROGRAPH AT								
+			160	12.25	6.	2.	2.	.02		
		ROUTED TO								
+			DAM160	12.25	6.	2.	2.	.02		
+									102.14	12.25
		ROUTED TO								
+			161	12.25	6.	2.	2.	.02		
		HYDROGRAPH AT								
+			155	12.25	7.	2.	2.	.03		
		ROUTED TO								
+			DAM155	12.25	6.	2.	2.	.03		
+									102.16	12.25
		3 COMBINED AT								
+			157	12.25	16.	4.	4.	.06		
		ROUTED TO								
+			151	12.25	16.	4.	4.	.06		
		HYDROGRAPH AT								
+			150	12.25	12.	4.	4.	.04		
		ROUTED TO								
+			DAM150	12.25	10.	3.	3.	.04		
+									102.23	12.25
		2 COMBINED AT								
+			152	12.25	26.	7.	7.	.10		
		ROUTED TO								
+			121	12.33	26.	7.	7.	.10		
		HYDROGRAPH AT								
+			70	12.33	1.	0.	0.	.01		
		ROUTED TO								
+			122	12.42	2.	0.	0.	.01		
		HYDROGRAPH AT								
+			120	12.25	4.	1.	1.	.01		
		ROUTED TO								
+			DAM120	12.25	4.	1.	1.	.01		
+									102.10	12.25
		3 COMBINED AT								
+			123	12.33	32.	9.	9.	.13		
		HYDROGRAPH AT								
+			75	12.33	32.	9.	9.	.13		
		ROUTED TO								
+			DAM75	12.33	32.	9.	9.	.13		
+									102.42	12.33

+		85	315.	12.33	33.	9.	9.	.14		
	ROUTED TO									
+		DAM85	314.	12.33	33.	9.	9.	.14		
+									102.41	12.33
	HYDROGRAPH AT									
+		80	24.	12.17	2.	1.	1.	.01		
	ROUTED TO									
+		DAM80	22.	12.25	2.	0.	0.	.01		
+									102.07	12.25
	ROUTED TO									
+		87	22.	12.25	2.	0.	0.	.01		
	2 COMBINED AT									
+		88	329.	12.33	35.	9.	9.	.14		
	ROUTED TO									
+		96	307.	12.42	35.	9.	9.	.14		
	HYDROGRAPH AT									
+		95	35.	12.25	4.	1.	1.	.01		
	ROUTED TO									
+		DAM95	39.	12.25	3.	1.	1.	.01		
+									102.10	12.25
	2 COMBINED AT									
+		97	332.	12.42	38.	10.	10.	.15		
	ROUTED TO									
+		91	331.	12.42	38.	10.	10.	.15		
	HYDROGRAPH AT									
+		90	42.	12.25	4.	1.	1.	.02		
	ROUTED TO									
+		DAM90	59.	12.25	3.	1.	1.	.02		
+									102.13	12.25
	2 COMBINED AT									
+		92	357.	12.42	41.	11.	11.	.17		
	HYDROGRAPH AT									
+		110	52.	12.25	4.	1.	1.	.02		
	ROUTED TO									
+		DAM110	52.	12.17	4.	1.	1.	.02		
+									102.12	12.17
	ROUTED TO									
+		101	51.	12.25	4.	1.	1.	.02		
	HYDROGRAPH AT									
+		100	105.	12.25	9.	3.	3.	.00		
	ROUTED TO									
+		DAM100	106.	12.25	9.	3.	3.	.00		
+									102.20	12.25
	3 COMBINED AT									
+		102	429.	12.42	54.	15.	15.	.19		

+		103	428.	12.42	54.	15.	15.	.19		
	HYDROGRAPH AT									
+		360	433.	12.42	57.	16.	16.	.21		
	HYDROGRAPH AT									
+		410	457.	12.42	61.	17.	17.	.25		
	ROUTED TO									
+		421	456.	12.42	61.	17.	17.	.25		
	HYDROGRAPH AT									
+		140	63.	12.25	7.	2.	2.	.03		
	ROUTED TO									
+		DAM140	74.	12.25	6.	2.	2.	.03	102.15	12.25
+										
	HYDROGRAPH AT									
+		130	133.	12.25	15.	4.	4.	.06		
	ROUTED TO									
+		DAM130	130.	12.33	14.	4.	4.	.06	102.23	12.33
+										
	HYDROGRAPH AT									
+		420	164.	12.50	28.	8.	8.	.15		
	2 COMBINED AT									
+		422	618.	12.42	89.	25.	25.	.40		
	ROUTED TO									
+		401	602.	12.42	89.	25.	25.	.40		
	HYDROGRAPH AT									
+		240	171.	12.25	14.	4.	4.	.07		
	ROUTED TO									
+		246	146.	12.33	14.	4.	4.	.07		
	HYDROGRAPH AT									
+		245	56.	12.25	5.	1.	1.	.02		
	2 COMBINED AT									
+		247	199.	12.25	19.	6.	6.	.10		
	ROUTED TO									
+		248	190.	12.33	19.	5.	5.	.10		
	HYDROGRAPH AT									
+		180	64.	12.33	9.	3.	3.	.07		
	HYDROGRAPH AT									
+		255	64.	12.25	6.	2.	2.	.03		
	ROUTED TO									
+		236	59.	12.33	6.	2.	2.	.03		
	HYDROGRAPH AT									
+		235	33.	12.25	3.	1.	1.	.01		
	2 COMBINED AT									
+		237	80.	12.33	8.	2.	2.	.04		

+	HYDROGRAPH AT	455	789.	12.50	375.	226.	226.	.39		
+	HYDROGRAPH AT	280	195.	12.25	16.	5.	5.	.06		
+	ROUTED TO	DAM5	51.	12.50	7.	2.	2.	.06		
									102.12	12.50
+	HYDROGRAPH AT	250	101.	12.25	17.	5.	5.	.11		
+	HYDROGRAPH AT	460	149.	12.67	36.	10.	10.	.24		
+	2 COMBINED AT	461	926.	12.50	411.	237.	237.	.63		
+	HYDROGRAPH AT	495	1018.	12.67	440.	244.	244.	.79		
+	HYDROGRAPH AT	270	147.	12.25	12.	4.	4.	.06		
+	HYDROGRAPH AT	470	173.	12.42	23.	7.	7.	.13		
+	HYDROGRAPH AT	2223	82.	12.33	16.	5.	5.	.00		
+	HYDROGRAPH AT	480	122.	12.50	26.	7.	7.	.08		
+	2 COMBINED AT	481	289.	12.42	49.	14.	14.	.21		
+	HYDROGRAPH AT	490	358.	12.75	87.	25.	25.	.41		
+	2 COMBINED AT	491	1362.	12.67	527.	269.	269.	1.20		
+	HYDROGRAPH AT	550	1474.	12.83	564.	278.	278.	1.41		
+	2 COMBINED AT	552	2453.	12.83	821.	351.	351.	2.87		
+	HYDROGRAPH AT	545	2485.	12.83	832.	354.	354.	2.96		
+	2 COMBINED AT	547	7449.	13.83	4131.	1575.	1575.	19.48		
1	SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM8									

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	4.	4.
OUTFLOW	0.	0.	1.

RATIO MAXIMUM MAXIMUM MAXIMUM MAXIMUM DURATION TIME OF TIME OF

1 1.00 102.17 .17 4. 82. 11.67 12.50 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM165

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 102.11 .11 1. 45. 4.58 12.17 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM160

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 102.14 .14 1. 65. 7.33 12.25 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM155

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 102.16 .16 1. 78. 8.00 12.25 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM150

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1 1.00 102.23 .23 2. 131. 11.92 12.25 .00
 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM120

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.10	.10	1.	42.	5.00	12.25	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM75

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.42	.42	0.	324.	12.00	12.33	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM85

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.41	.41	0.	314.	12.08	12.33	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM80

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.07	.07	0.	22.	2.17	12.25	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM95

PLAN 1	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.10	.10	1.	39.	4.33	12.25	.00

1 SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM90

ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.13	.13	1.	59.	4.67	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM110

1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.12	.12	1.	52.	5.17	12.17	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM100

1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.20	.20	0.	106.	13.75	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM140

1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.15	.15	1.	74.	8.42	12.25	.00

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM130

1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
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1

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM1

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	11.	11.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.40	.40	13.	299.	11.67	12.50	.00

1

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM200

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	0.	0.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.13	.13	0.	55.	10.75	12.42	.00

1

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM135

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.16	.16	1.	81.	8.33	12.25	.00

1

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM7

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	1.	1.
OUTFLOW	0.	0.	1.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1.00	102.47	.47	1.	377.	24.00	12.33	.00

1

SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM6

PLAN 1

	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
ELEVATION	100.00	101.99	102.00
STORAGE	0.	2.	2.
OUTFLOW	0.	0.	1.

	OF PMF	RESERVOIR W.S.ELEV	DEPTH OVER DAM	STORAGE AC-FT	OUTFLOW CFS	OVER TOP HOURS	MAX OUTFLOW HOURS	FAILURE HOURS
1	1.00	102.07	.07	2.	24.	4.83	12.42	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM2						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	100.00	101.99	102.00
	STORAGE	0.	2.	2.
	OUTFLOW	0.	0.	1.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	102.13	.13	2.	60.	5.75	12.33	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM4						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	100.00	101.99	102.00
	STORAGE	0.	4.	4.
	OUTFLOW	0.	0.	1.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	101.81	.00	3.	0.	.00	.00	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM3						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	100.00	101.99	102.00
	STORAGE	0.	2.	2.
	OUTFLOW	0.	0.	1.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	102.10	.10	2.	42.	5.25	12.33	.00
		SUMMARY OF DAM OVERTOPPING/BREACH ANALYSIS FOR STATION DAM5						

PLAN 1		INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	ELEVATION	100.00	101.99	102.00
	STORAGE	0.	6.	6.
	OUTFLOW	0.	0.	1.

	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
1	1.00	102.12	.12	6.	51.	11.67	12.50	.00

HYDROLOGIC MODEL DATA SHEET

Date: _____

By: _____

Area Modelled: Camelback Walk Channel to Az. Canal

~~FCD~~ File Code: ADOT

Report Title: Hydrology Report: Outer Loop Highway Camelback Walk Channel to Az. Canal

Author: Simons, Li, & Assoc

Date: May 1989, final copy

~~FCD~~ Staff Contact: _____

Hydrologic Model Summary

Computer Model Used: HEC-1
version/date: _____

Total Area: 2.9 mile reach of Outer Loop highway affected.

No. of Subbasins: _____

Rainfall:

freq. & duration: _____

temporal dist.: _____

~~rainfall depth:~~ _____

areal reduction: _____

Excess: _____

Hydrograph: _____

Routing: _____

Significant Structures
and Drainage Features:

Comments:



MORRISON-KNUDSEN ENGINEERS, INC.
A MORRISON KNUDSEN COMPANY

LETTER OF TRANSMITTAL

10240 N. 31st AVE., SUITE 213
PHOENIX, ARIZONA 85051-9565
PHONE: (602) 997-4050
TELECOPY: (602) 943-3072

TO: Maricopa County Flood Control Dist.
3335 West Durango Street
Phoenix, AZ

DATE	3-12-90	PROJ. No.	3863
ATTENTION Mr. Dick Perreault			
RE: Contract No. 89-34			
Pima Freeway			
90th Street - Indian School Road			
101 L MA H2447 01D			

GENTLEMEN:

WE ARE SENDING YOU Attached Under separate cover via Messenger the following items

- Shop drawings Prints Plans Samples Specifications
 Copy of letter Change order See Below

COPIES	DATE	NO.	DESCRIPTION
3		1	Meeting Notes from 3-8-90 9:30 a.m. meeting re: Discharge for drainage water from Arizona Canal to MCFCD

THESE ARE TRANSMITTED as checked below:

- For approval Approved as submitted Resubmit _____ copies for approval
 For your use Approved as noted Submit _____ copies for distribution
 As requested Returned for corrections Return _____ corrected prints
 For review and comment _____
 FOR BIDS DUE _____ 19 _____ PRINTS RETURNED AFTER LOAN TO US

REMARKS: _____

FLOOD CONTROL DISTRICT RECEIVED	
MAR 02 1990	
CH ENG	P & PM
DEP	HYDRO
ADMIN	LMGT
FINANCE	FILE
C & O	
ENGR	
REMARKS	

FLOOD CONTROL DISTRICT RECEIVED	
MAR 12 1990	
CH ENG	P & PM
DEP	HYDRO
ADMIN	LMGT
FINANCE	FILE
C & O	
ENGR	
REMARKS	

COPY TO: _____

SIGNED: *Dawn M Speight*

PIMA FREEWAY
90th STREET TO INDIAN SCHOOL ROAD
MEETING NOTES

DATE: March 8, 1990
TIME: 0930 Hrs
LOCATION: MCFCD

ADOT PROJECT NO.: 89-34
TRACS NO.: 101 L MA H2447 01D
MKE PROJECT NO.: 3863
SERIAL NO.: G66.mtg

ATTENDEES: Dick Perreault MCFCD
 Dave Johnson MCFCD
 Ed Raleigh MCFCD
 Mike Karpuk DCCO
 Stan Polasik DCCO
 Ray Jordan ADOT
 Frank Medina ADOT
 Rich DeBoer ADOT
 Norm Gutcher MKE

This meeting was held specifically as a vehicle for DCCO and ADOT to present the current proposal for discharge of drainage water from the area North of the Arizona Canal to the Maricopa County Flood Control District (MCFCD).

Stan Polasik made the initial presentation. He explained that two concepts had been evaluated. One that used the existing flood plain east on Pima Road and north of the Arizona Canal as it currently functions and provided a bridge across the flood plain to avoid increasing the 100 year flood elevation. The other alternate, referred to as number 2, utilizes a lined channel parallel to the Az. Canal on the north side draining to the west under Pima Road through a new multi-barrel box culvert and into MCFCD facilities which ultimately empty into the Indian Bend Wash.

The concept presented in the Arizona Canal Crossing Study which took all of the flow south across the canal and to the Salt River was not mentioned.

The drainage channel north of the Arizona Canal along the freeway will be an unlined channel. Only that portion turning the corner at the Az. Canal and through the new box under Pima Road will be lined.

Conjecture was that the COE studies may not have accounted for the effect of the bar screen in the Arizona Canal which exists just upstream of Pima Road.

Downstream of Pima Road the Az. Canal capacity is thought to be

2500 cfs. There was considerable discussion on this point. The spillway provided by the COE west of Pima Road is a side weir spillway which is supposed to have a capacity of 5500 cfs. This is the same as the flow proposed to come from the project drainage channel. Flood flow above that would go to the Arizona Canal.

The statement was made by Ray Jordan that the entire analysis assumed "responsible" operation of the Evergreen Wasteway Gate by SRP. This means dumping the normal flow of the Canal to accept flood flow.

The statement was made by several of the participants that there was no flood plain definition by FEMA east of Pima Road because the area was Indian Land.

Ray Jordan, in response to a question from Dick Perreault, said that the drainage area north of the Arizona Canal was approximately 26 square miles.

It was stated that this alternate is now made feasible because the Indians have agreed to make ROW available. Ray Jordan said that it was not yet clear whether the ROW would be in "fee" or as an easement.

Dick Perreault asked who would maintain the drainage facility. Ray Jordan answered that ADOT would maintain the portion that they construct, which would include the portion from east of the freeway through the Pima Road box and box discharge area.

According to Mike Karpuk DCCO has estimated the capacity of the Arizona Canal at 3700 cfs without overtopping the south bank. (In previous conversations SRP staff has said 1900 cfs) There is a low section of the south bank near the bar screen just east on Pima Road which, according to Mike Karpuk, should be raised. This improvement, he says, should raise the capacity of the Arizona Canal to 5000 cfs.

Note: MKE should obtain a copy of the COE study for Indian Bend Wash.

There was discussion of the design Q. The COE studies use 8000 cfs while the SLI studies show 9326 cfs. ^{SCA} Stan Polasik explained that the difference was explained in that SLI had used HEC-1 and the Kenimatic Wave formula to predict the 9326 and felt that their number was conservatively high. Both Stan and Ray said that the 8000 cfs flow would be used for design with the higher flow used only to check and be sure that the design would not create a hazard to private property.

Mike Karpuk gave some estimated flow velocities downstream of the Pima Road Box Culvert. At the end of the culvert $V = 13.5$ fps, at the end of the apron $V = 11.3$ fps and 200 downstream $V = 6.7$ fps.

There was discussion of the extent of improvements downstream of the box to be provided by ADOT. While there was no definite limit set all agreed that some work would be needed to allow the flow velocity to reduce to a point where erosion of the downstream drainage channel would not be a problem.

Over the last two years a developer has been attempting to build a golf course in the drainage way downstream of Pima Road. Apparently he has given up. His license from MCFCD has expired. Any subsequent such development will require a new approval process and will, according to MCFCD have to accommodate the drainage way.

Regarding permits, no one seemed sure if a COE section 404 permit would be required. The consensus was to apply for on at the earliest opportunity and let the COE either issue the permit or state in writing that one was not needed. Water Quality seemed to be of more concern than wetlands encroachment. Degradation of receiving waters was the concern here.

Per Ray Jordan, the next step in the process is to go to the City of Scottsdale. Ray made the statement that he assumed SRP would be no problem.

Through the design process, MCFCD agreed to be the point of contact with the COE.

DCCO will attempt to set up a meeting with the City of Scottsdale as soon as possible. Dick Perreault pointed out that within the City of Scottsdale there are four individuals that will be part of the decision making process. 1) Bill Erickson, Drainage, 2) John Ferimelli (spelling unknown), Development, 3) Mike Millilo (spelling unknown), Development and 4) Bill ????, Parks Department.

Respectfully Submitted,


Norman K. Gutcher, P. E.

cc: In house only
Ron Holmes
Bob Ferrese
Norm Gutcher

NOTE: Should any participant in the above conversation have any comments regarding these notes, notify the signer, in writing, within ten (10) days from the date above. Otherwise, these notes will remain as prepared. 3

**DeLEUW
CATHER**

De Leuw, Cather & Company
One Gateway Center 426 N. 44th Street Suite 252
Phoenix, Arizona 85008 (602) 244-9096

LETTER OF TRANSMITTAL

DATE 3/9/90

JOB NO. 03731

ATTENTION Richard G. Perreault
RE: Pima Freeway Crossing
Arizona Canal

3/12/90

DR-47A-6-24

TO Flood Control District
3335 West Durango
Phoenix, AZ

COPIES	DATE	NO.	DESCRIPTION
1			Final Hydrology Report-OLH-Camelback Walk Channel to the Arizona Canal

THESE ARE TRANSMITTED as checked below:

For approval

As requested

For your information

For your use

For review and comment

REMARKS:

90 MAR 12 PM 1:37

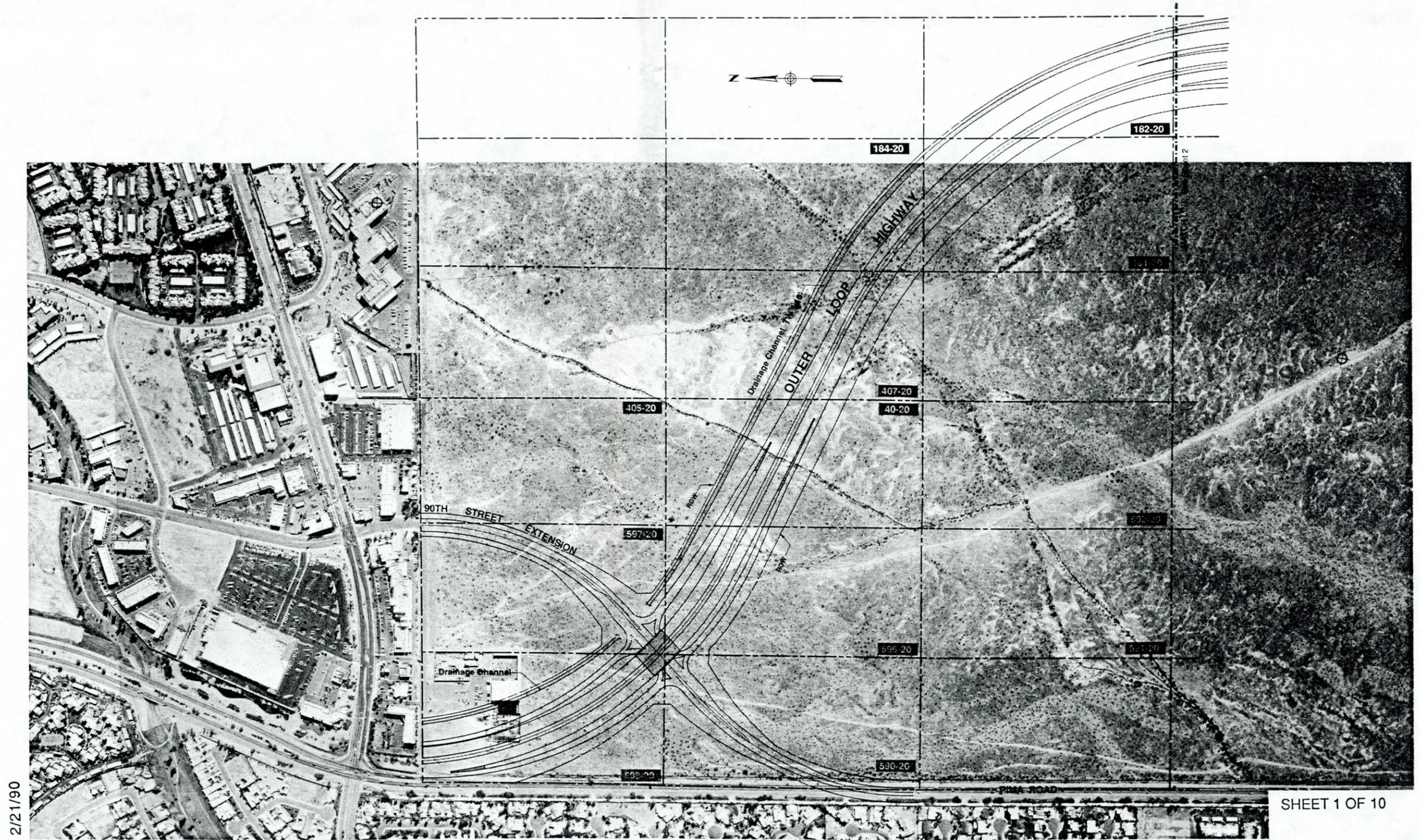
cc: R. Simeon
R. Jordan

OFFICE OF MARICOPA COUNTY
FLOOD CONTROL DISTRICT

SIGNED: _____

Stan Polak *AM*

BGP
3/8/90

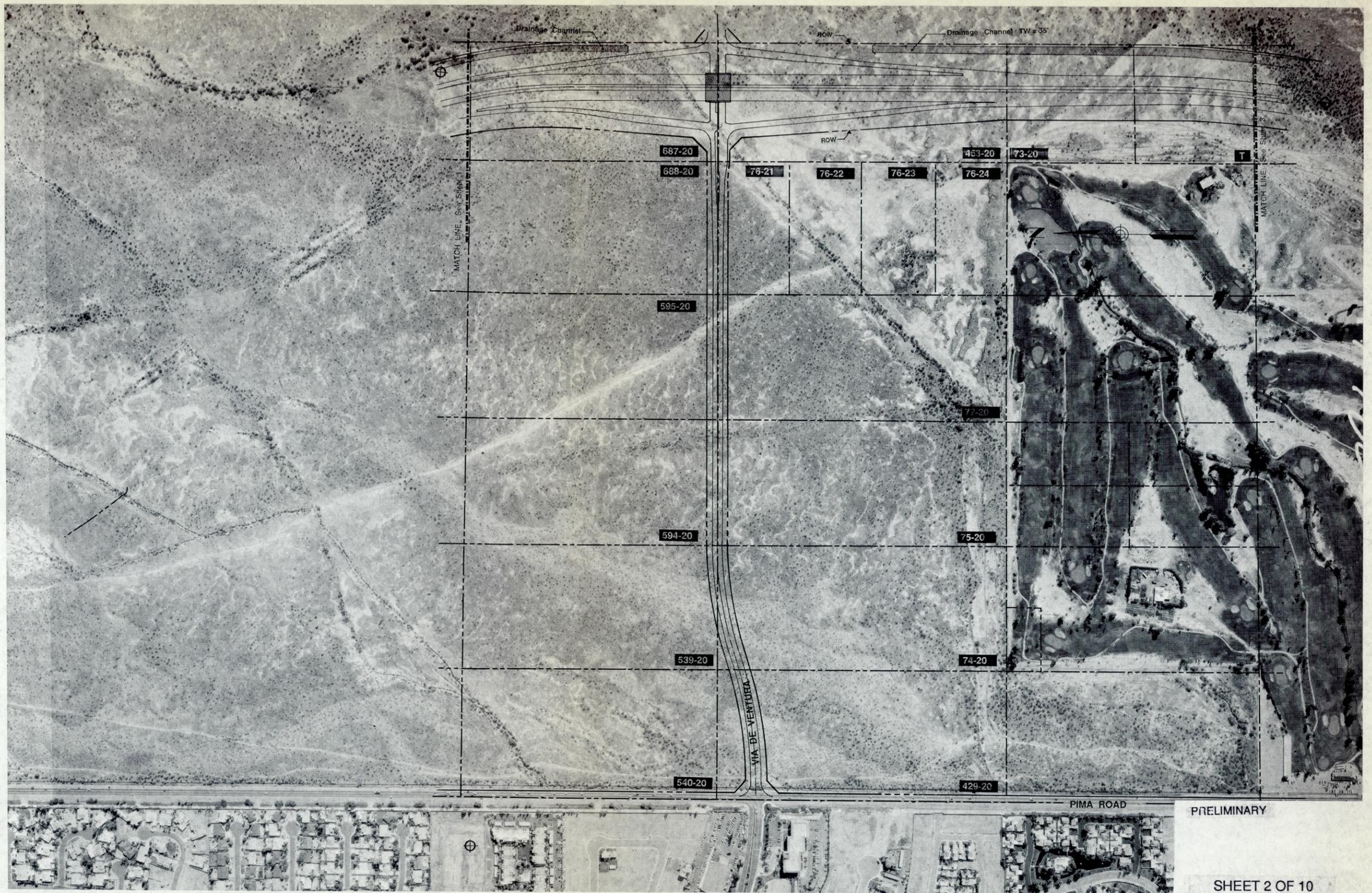


2/21/90

SHEET 1 OF 10

Outer Loop Highway

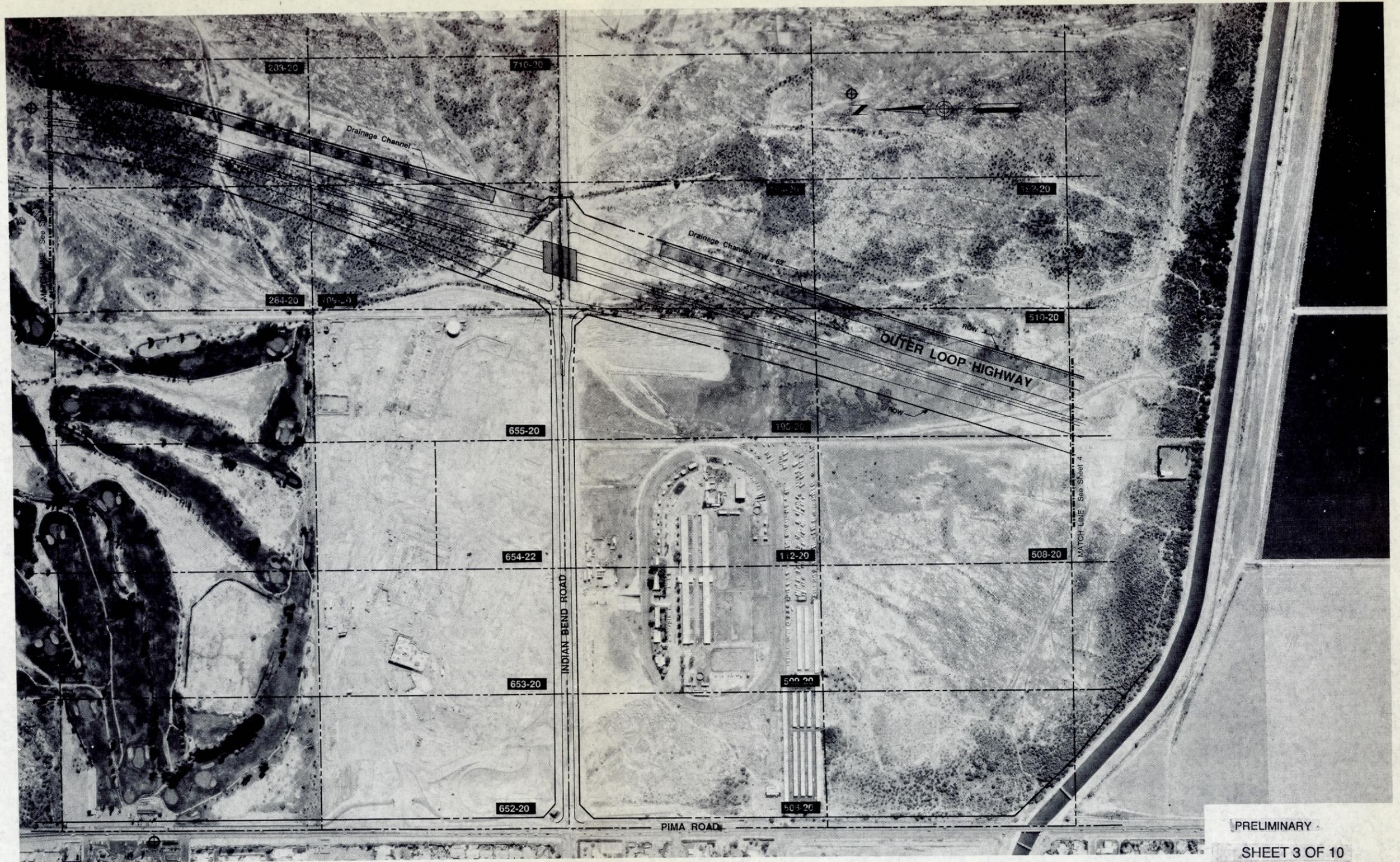
prepared by DeLeuw, Cather & Company, Management Consultant for the
ARIZONA DEPARTMENT OF TRANSPORTATION



Outer Loop Highway

prepared by DeLeuw, Cather & Company, Management Consultant for the

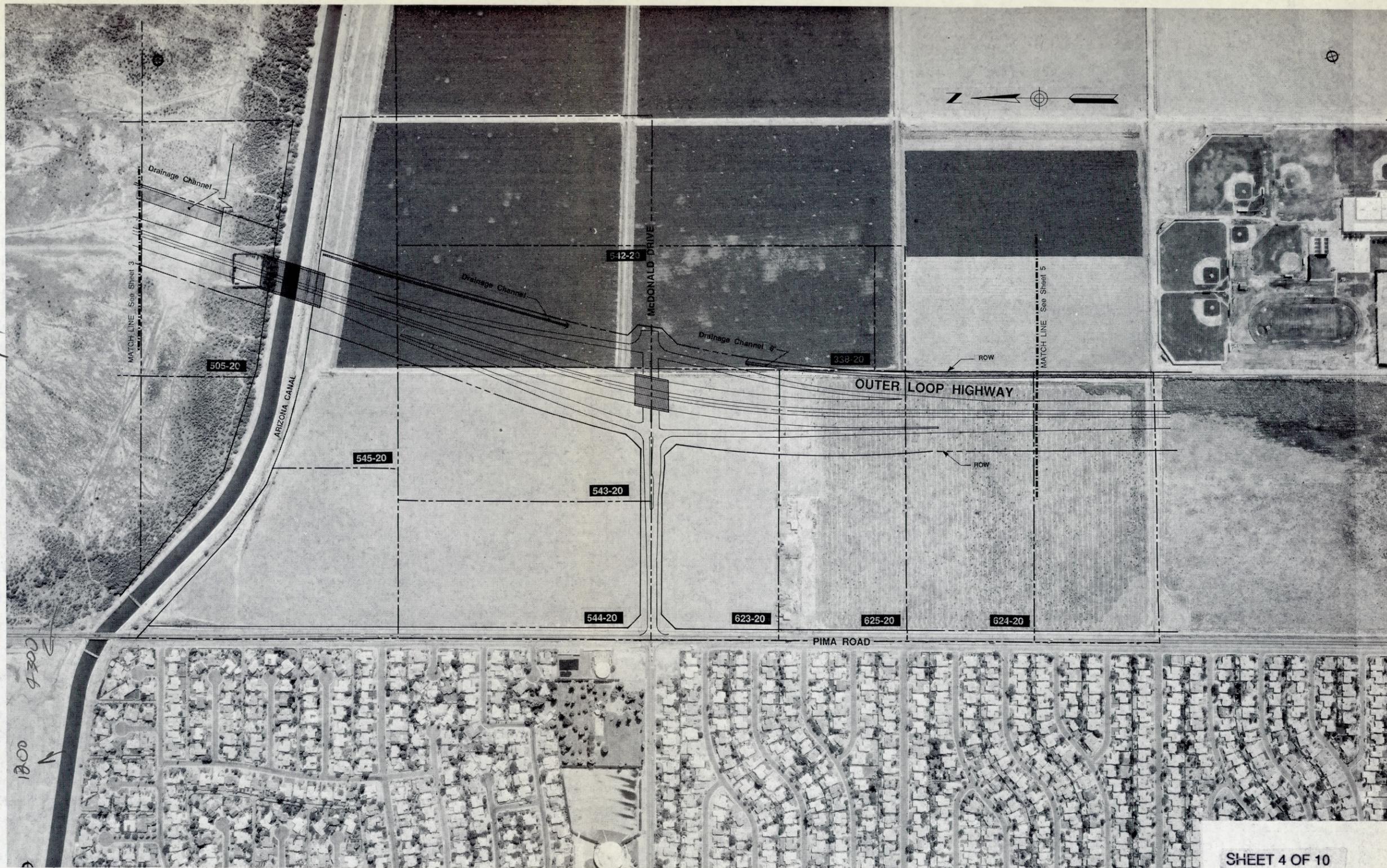
ARIZONA DEPARTMENT OF TRANSPORTATION



PRELIMINARY
SHEET 3 OF 10

Outer Loop Highway

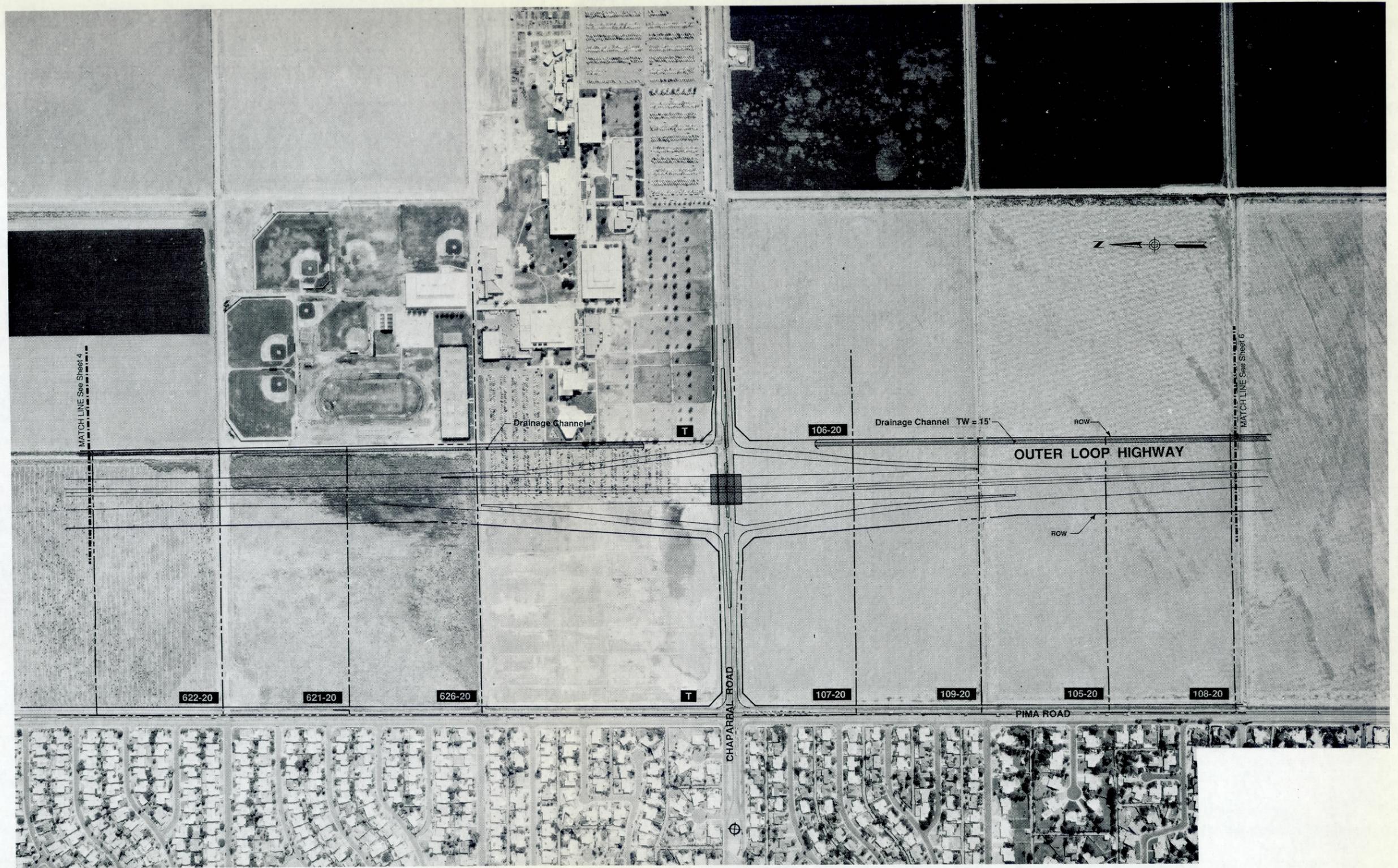
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SHEET 4 OF 10

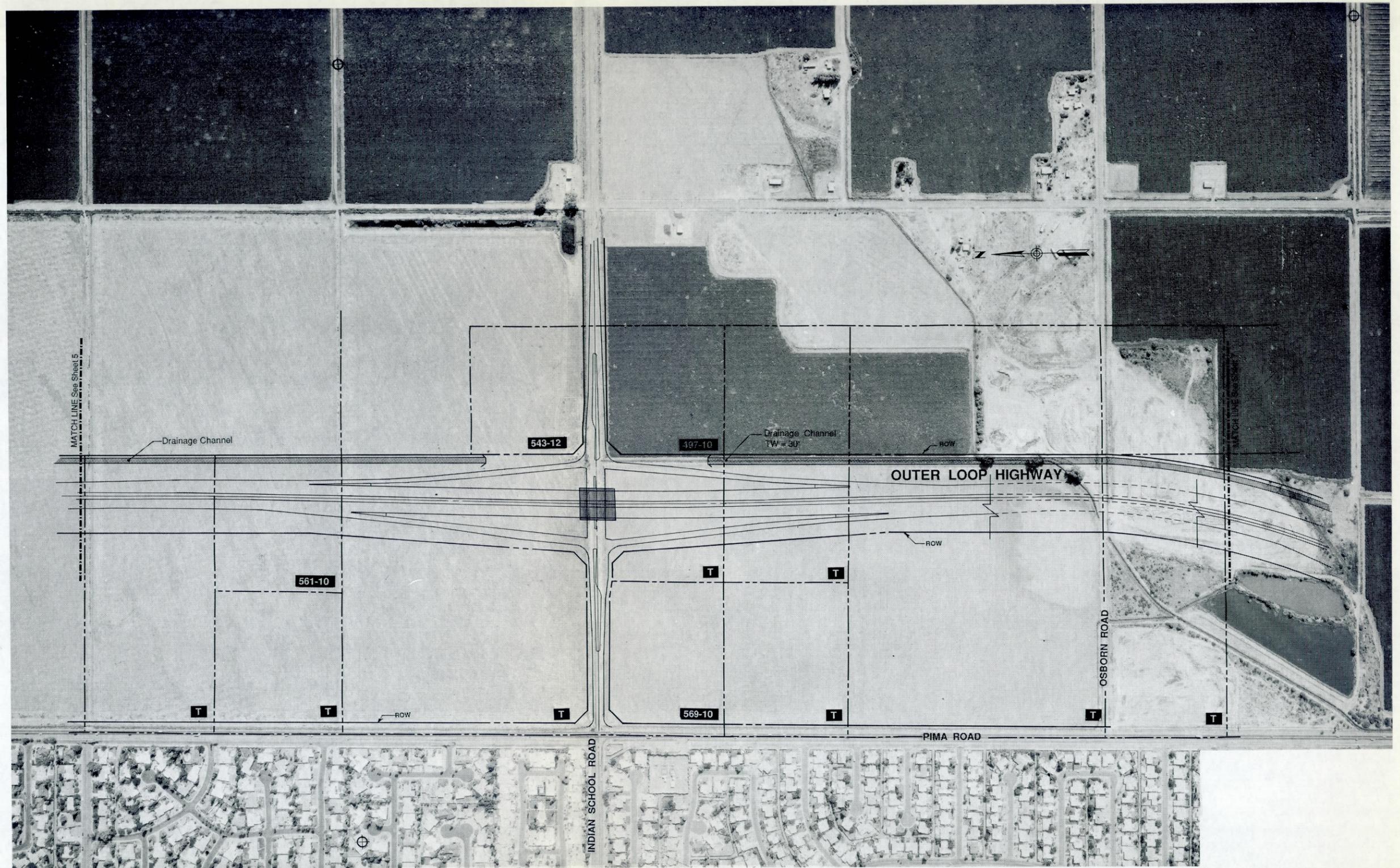
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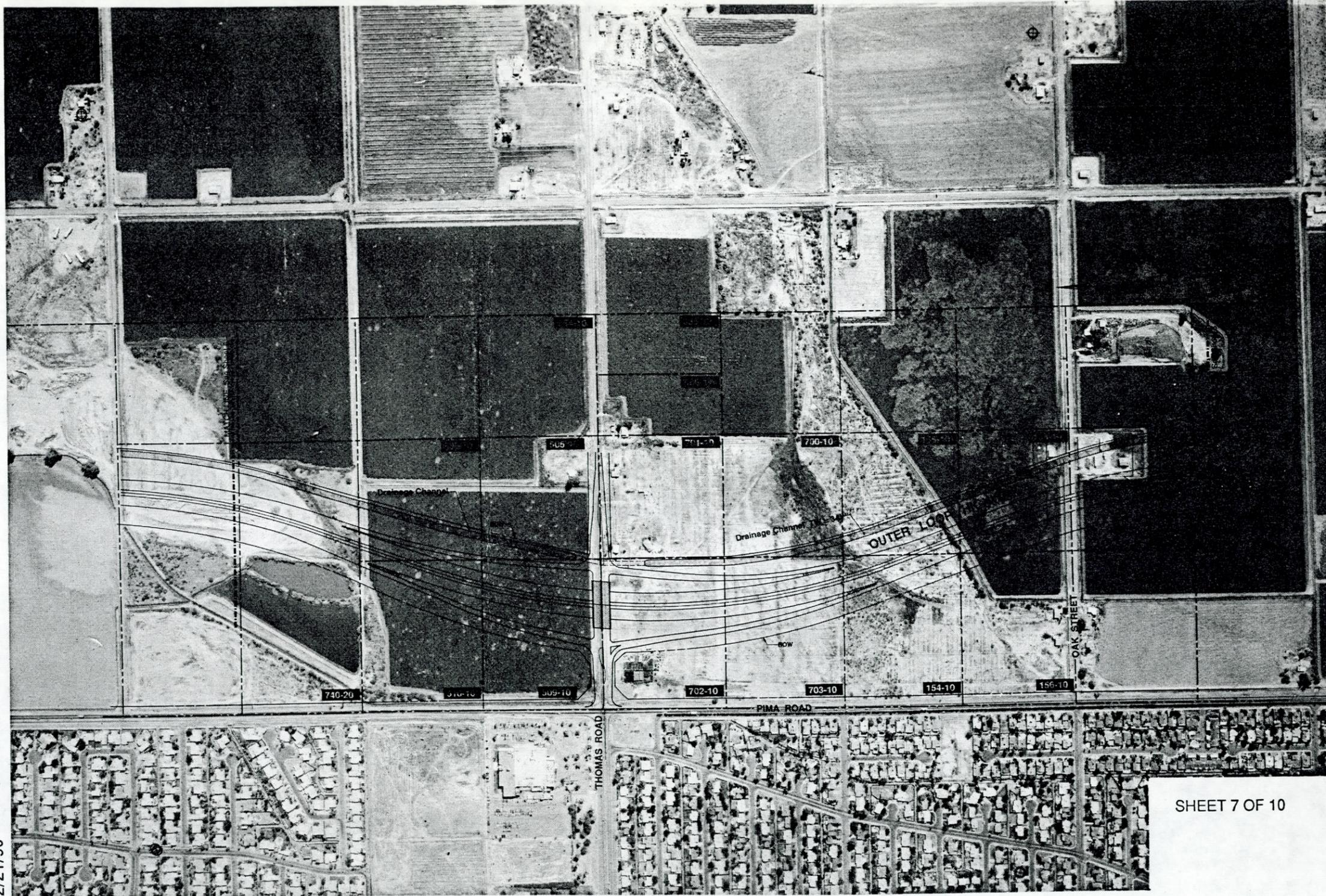
Outer Loop Highway

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ARIZONA DEPARTMENT OF TRANSPORTATION



Outer Loop Highway

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ARIZONA DEPARTMENT OF TRANSPORTATION

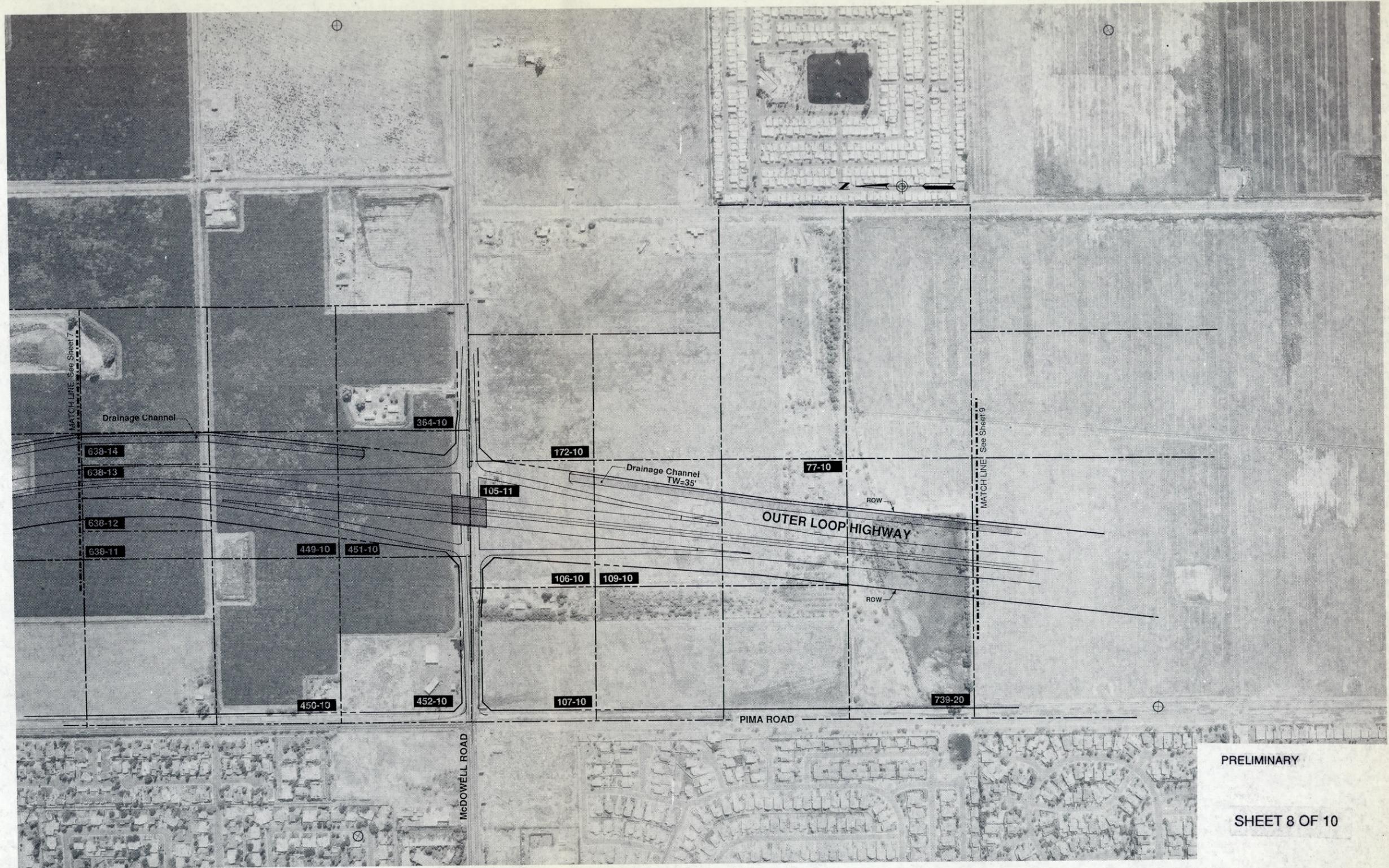


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SHEET 7 OF 10

Outer Loop Highway

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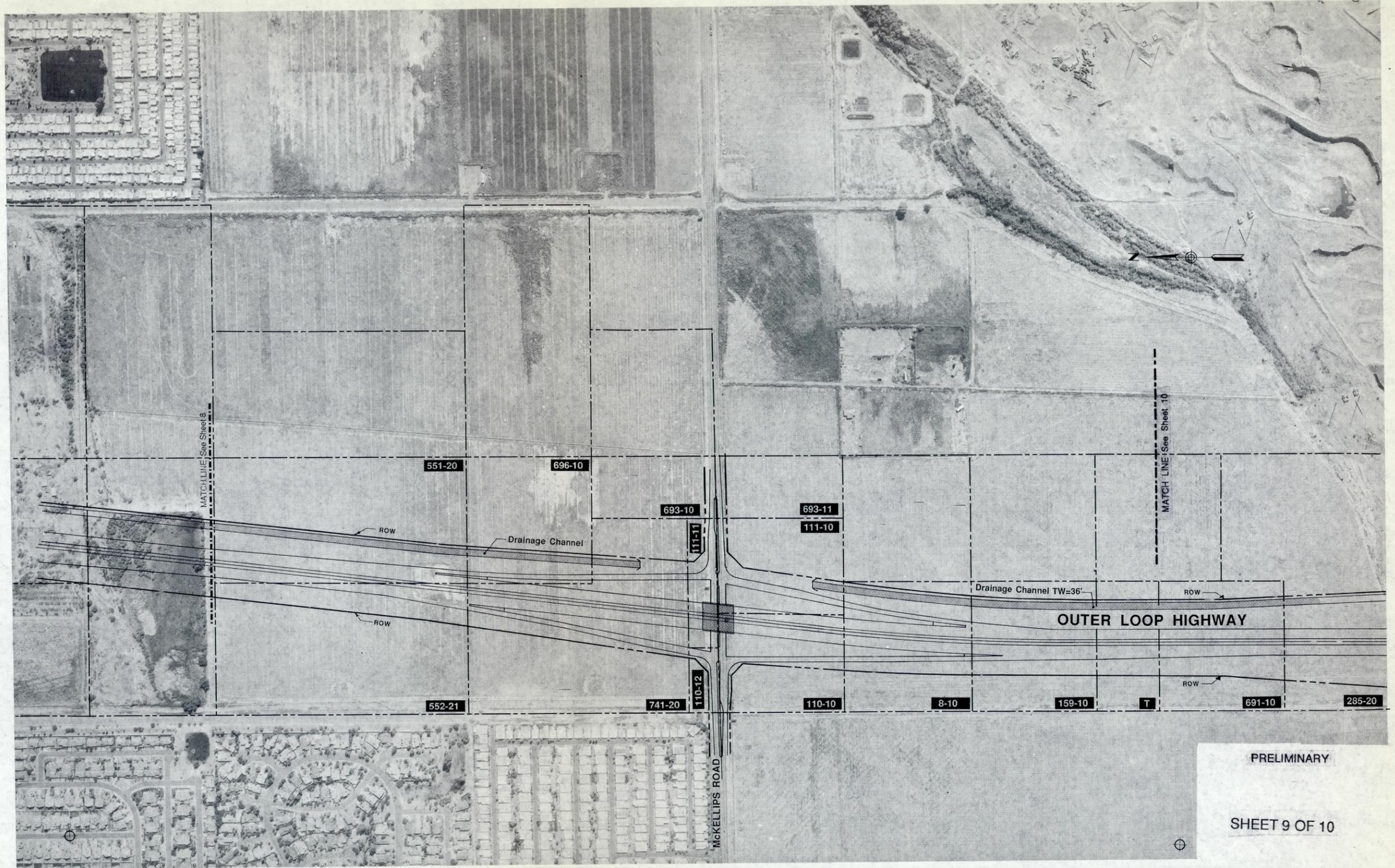


PRELIMINARY

SHEET 8 OF 10

Outer Loop Highway

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PRELIMINARY

SHEET 9 OF 10

Outer Loop Highway

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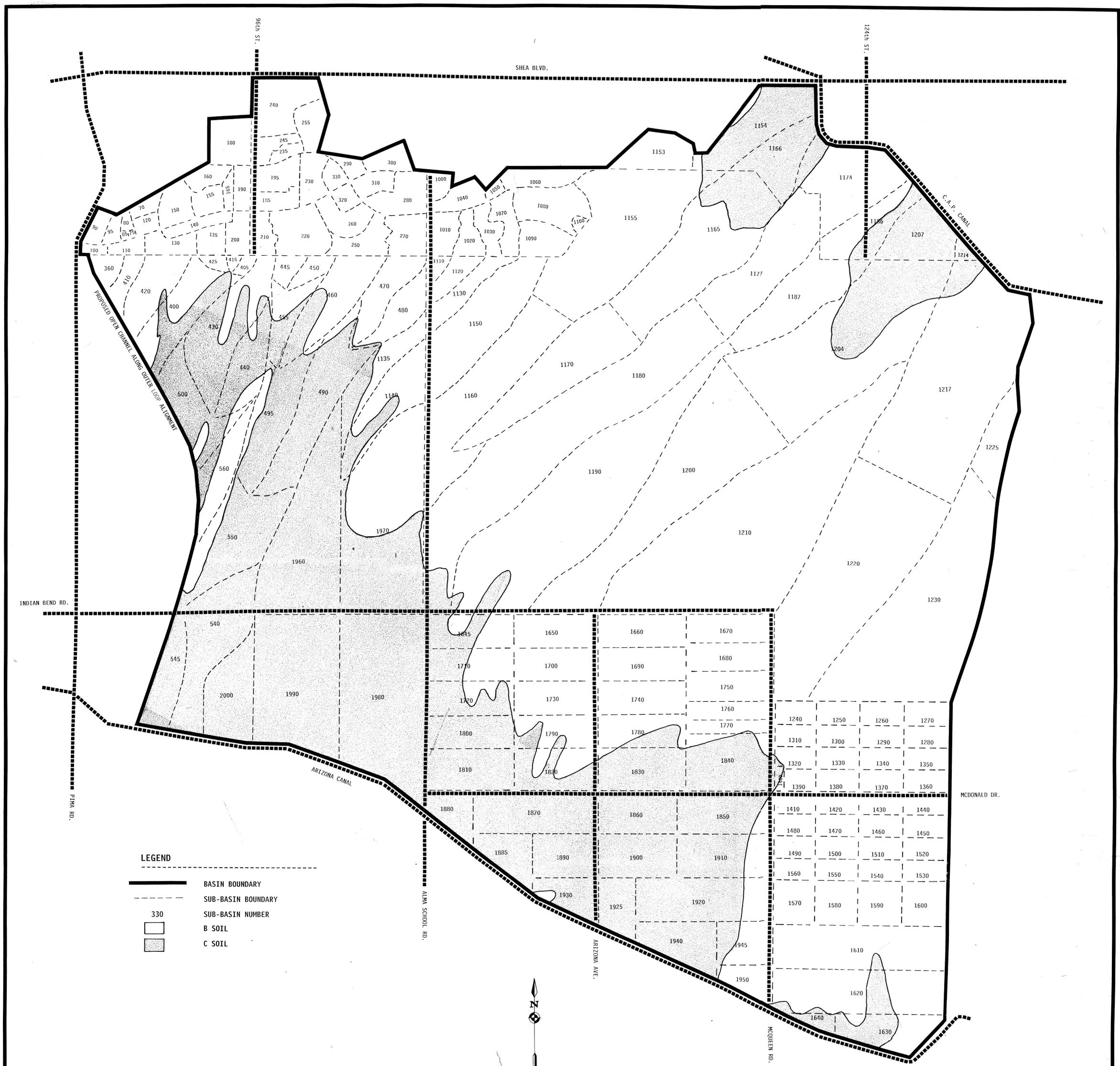


PRELIMINARY

SHEET 10 OF 10

Outer Loop Highway

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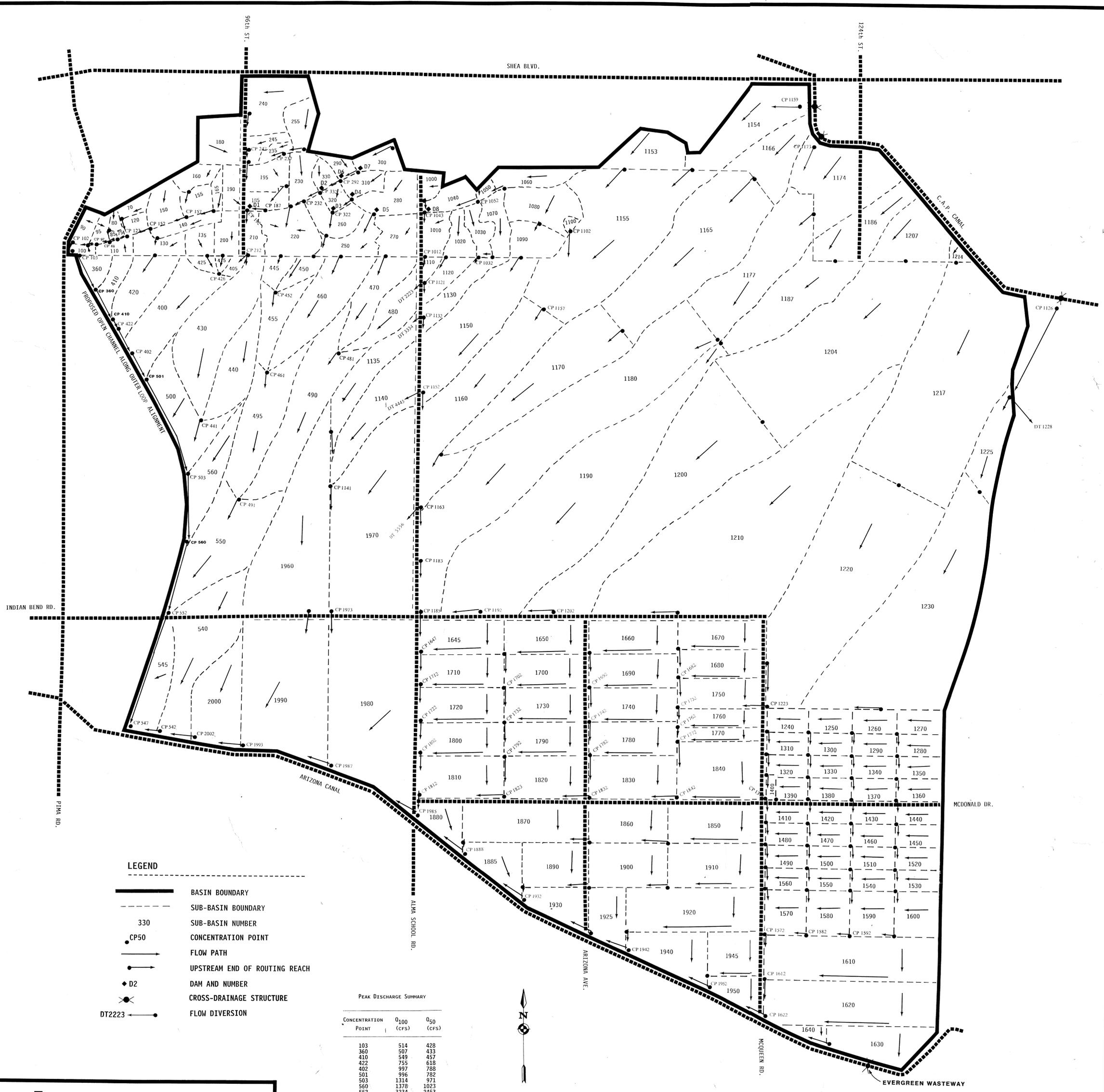
LEGEND

— BASIN BOUNDARY
 - - - SUB-BASIN BOUNDARY
 330 SUB-BASIN NUMBER
 □ B SOIL
 ■ C SOIL

500 2000
 0 1000
 SCALE
 1 INCH : 1000 FEET

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 TUCSON, AZ • PHOENIX, AZ.

OUTER LOOP HIGHWAY
 MARCH 1989 ALIGNMENT
 HYDROLOGIC SOIL GROUP MAP

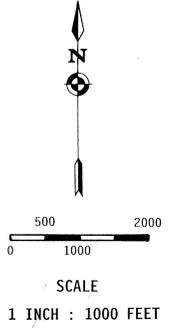


LEGEND

- BASIN BOUNDARY
- SUB-BASIN BOUNDARY
- SUB-BASIN NUMBER
- CONCENTRATION POINT
- FLOW PATH
- UPSTREAM END OF ROUTING REACH
- DAM AND NUMBER
- CROSS-DRAINAGE STRUCTURE
- FLOW DIVERSION

PEAK DISCHARGE SUMMARY

CONCENTRATION POINT	0 ₁₀₀ (CFS)	0 ₅₀ (CFS)
183	514	428
360	507	433
410	549	457
422	755	618
402	997	788
501	996	782
503	1314	971
560	1378	1023
552	3234	2453
1985	7241	5515
1987	8384	6622
1993	8248	6533
2002	8160	6455
542	8154	6447
547	9326	7449



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 Tucson, AZ • Phoenix, AZ

OUTER LOOP HIGHWAY
 MARCH 1989 ALIGNMENT
 HYDROLOGIC MODEL SCHEMATIC