

Review of Kimley-Horn Sediment Transport Model of the Agua Fria River

Prepared for:
Flood Control District of Maricopa County



Water • **E**nvironmental • **S**edimentation • **T**echnology

DRAFT REPORT

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Prepared for:
Flood Control District of Maricopa County



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1. Executive Summary

WEST Consultants, Inc. reviewed the Sediment Transport Model Developed by Kimley-Horn and Associates, Inc. (Kimley-Horn or KH). A number of technical problems were found in the models which warrant revision of the models and the running of the revised models to insure that the problems identified did not result in inaccurate results. Several of the problems were significant and could lead to erroneous conclusions based on the existing model output. While the results of the corrected models may not differ by large amounts after the analysis, it is important to run the corrected models to insure a correct basis for future decisions and to determine the impact of the proposed model corrections. Based on WEST's modeling of a portion of the Agua Fria River it is likely that the model results will not change drastically due to the corrections proposed herein. However, given the scope of decisions that will be based on these results, it is important to be certain of their impact.

Bed samples obtained by Kimley Horn were compared with those obtained by Arizona State University (ASU) in an earlier study of the Agua Fria River. These sample analysis results differed significantly in some areas and new model results were calculated for the river based on the ASU samples. These results showed local variations as large as ± 4.5 feet while the overall variation was on the order of 0.1 ft. Additional samples collected by WEST for another project were different from both the ASU and the Kimley-Horn samples for the reach where WEST obtained samples. Given the width of the river, the complexity of bedding and the length of the river, variations can be expected in bed samples. Model results from both the Kimley-Horn and ASU sets of data should be used as a sensitivity analysis to view possible variations in calculated results and to determine their impact on planning decisions for the river.

While problems were found in the models, the problems are more an indication of the difficulty of visualizing the input and results than from poor modeling efforts. The current state of the HEC-6 model leaves much to be desired in terms of visualization of input data and output results. The visualization of input data is of critical importance in modeling efforts

2. Introduction

2.1 Scope

The Flood Control District of Maricopa County (FCDMC) has commissioned WEST Consultants, Inc. (WEST) to review the Kimley-Horn and Associates, Inc. (KHA) Final Sediment Trend Analysis Report for the Agua Fria Watercourse Master Plan dated November 2001. The final report followed the Technical Memorandums that KHA has previously prepared and submitted to the FCDMC and incorporated the November 2001 revisions to the draft report.

2.2 KHA Sediment Analysis

The purpose of the sediment trend analysis performed by KHA was to predict and evaluate the long-term and flood response of the streambed profile of the Agua Fria River based on existing, proposed, and alternative development conditions along and within the river. The study was an integral part of a broader investigation of the Agua Fria River for the Agua Fria Watercourse Master Plan.

The KHA analysis developed several sediment trend models to evaluate the streambed profile response to various conditions. All the models were developed using the U.S. Army Corps of Engineers (COE) library version of HEC-6 computer program from October 1993 with some errors corrected in 1995. The models included an existing river conditions model, full sand and gravel mining model, full encroachment model, and a grade control structure model. From the results and interpretation of the above models, KHA recommended river stabilization measures and policy/procedures for the FCDMC to implement regarding sediment management in the Agua Fria River.

The sediment trend analysis began at the confluence of the Agua Fria River with the Gila River. The upstream limit was approximately at the diversion outlet downstream of New Waddell Dam. The total length of the studied reach of the river was approximately 33.8 miles. KHA has identified several tributaries along the study reach of the Agua Fria River, but only the New River (which joins the Agua Fria south of Glendale Airport) was incorporated into the sediment trend analysis.

The KHA analysis attempted to predict anticipated river channel response to long-term channel forming discharge based on the potential release from the New Waddell Dam under the 100-year flood event (30 days of 9,000 cubic feet per second (cfs) into the Agua Fria River). The short-term event selected to evaluate the river response was the 100-year flood (the peak discharge of 54,400 cfs in the lower Agua Fria River) as developed by the COE.

3. HEC-6 Hydraulic Base Model

KHA initiated the study with the development of an HEC-6 fixed bed model (filename: affbrev.dat) based on the 1996 Flood Insurance Study (FIS) HEC-2 hydraulic model. The fixed bed model was the foundation used to develop the subsequent movable bed models.

For the purpose of this review, WEST first imported the KHA fixed bed model into the HEC-RAS interface and examined the model on a cross-sectional basis. The model consists of 113 cross sections at an average channel distance of 1600 feet (max distance of 5300 feet). There are 9 local tributaries, distributaries or flow adjustments along the course of the model. The downstream elevation boundary condition for the 100-year peak flow was set at 916.83 ft for a flow of 52,000 cfs at the most downstream cross section 0.16. The Manning roughness coefficient ranges from 0.03 to 0.05 for the channel, from 0.04 to 0.15 for the left overbank, and from 0.04 to 0.1 for the right overbank.

Channel encroachments and ineffective flow areas are specified in HEC-6 via the X3 or XL records (note that the X3 record in HEC-6 is **different** than the record of the same name in HEC-2). The X3 record permits the user to select from three different methods to define encroachments for a cross section. Methods 1 and 3 are used in the Agua Fria models (note that only one or the other may be chosen for a particular cross section, but not both). Method 1 restricts the water to the channel, as defined by the bank stations, until the water surface elevation exceeds the bank station elevation. Method 3 contains the water between user-specified left and right limits, until the water surface elevation exceeds user-specified left and right elevations. With the XL record, the modeler specifies left and right limits in the cross section beyond which there is no conveyance, regardless of the water surface elevation.

Scour is allowed in a cross section in HEC-6 when the ground points are below the water surface elevation, within the movable bed limits (defined on the HD record), within the effective flow limits (defined on the X3 record) and within the conveyance limits (defined on the XL record). Deposition is allowed for ground points below the water surface elevation and within the movable bed limits. Deposition is not allowed in ineffective areas defined by X3 records, but can occur in areas with no conveyance as defined by XL records.

The fixed bed model features encroachment limits defined on X3 records, but does not have conveyance limits (XL records) needed to describe the ineffective flow areas where sediment will be allowed to deposit. The fixed bed model was developed for the purpose of calibration to the FIS model. Conveyance limits specified in the movable bed model should also be present in the fixed bed (calibrated) model because the limits affect the channel hydraulics.

The subcritical HEC-RAS run revealed that 86 percent of the cross sections experienced an energy loss greater than 1 foot between one cross section and another. The high percentage of energy loss warnings usually indicates the need for additional cross sections. This is particularly true for cross sections 25.860, 17.638 and 4.094 where the energy equation could not be balanced due to an abrupt velocity variation. Two of these cross sections are very likely supercritical since 4.094 appears to be an existing drop structure and 17.638 is a pit haul road that is modeled as a drop structure. This road will very likely erode away during a high flow event but the retaining of the road is a conservative approach. It is recommended that additional

cross sections be introduced at other locations where an abrupt change in velocity occurs (e.g. cross section 32.998, 30.73, 30.07, 17.638, 15.374, 11.428, etc.).

Significant expansion and contraction of the water surface width was noted at several cross sections (e.g. 1.33, 3.43, 15.374, 25.666, 32.92 as well as others). The expansion ratio should generally be limited to 3:1 (3 streamwise units to 1 lateral unit) and the contraction ratio should generally be limited to 1:1 by setting ineffective flow or conveyance limits (X3 or XL records) where appropriate. The cross sections listed above appear to be in violation of these recommendations.

Discontinuous features in the channel geometry (e.g. structures, high ground) should also be defined by ineffective flow or conveyance limits (X3 or XL records) both upstream and downstream when the influence is seen at the upstream and downstream cross sections. An example of this is a feature at cross section 6.91 which is not seen in either the upstream or downstream cross section (Figure 1). This feature is a protected electrical transmission tower but should appear in the downstream cross section which is only 10 feet below the tower according to the input file. The influence would probably not be noticed upstream (cross section 6.93) where the distance is larger than the diameter of the tower base (e.g. a 1:1 contraction) but should be modeled in cross section 6.89. The influence of this tower may not be extremely important in cross section 6.89 but it appears the cross sections were designed to model the influence of the towers in this reach of the river. Currently however, the influence is limited to only cross section 6.91.

Several cross sections should have encroached areas specified (by X3 record) behind what appear to be levees. For example: cross section 8.105 for both berms/levees (Figure 2); 15.0, 16.506, and 21.157 (Figure 3) for the right levee. The "levees" on the left side of cross section 8.105 appears to be a road embankment and flow in that area should have been set to be ineffective. There is no X3 or XL card for this cross section however. It should also be noted that the cross section geometry appears to be different when a comparison is made between Figure 2 and the same cross section in the Appendix which shows the existing conditions base movable bed model. Similar problems need to be resolved at the other cross sections listed above.

Bank stations need to be revised at some locations to better reflect the channel hydraulics that will later be used for the computation of sediment transport capacity in the movable bed model. They should be identified from planform analysis and field visits and should consistently identify the (continuous) main channel. Generally, channel bank stations define erosion limits in HEC-6. This is because river erosion occurs mainly within the channel, while floodplains are mainly depositional areas. Therefore, the effort should be made to describe the channel by the appropriate specification of the bank stations. This is particularly important for braided systems where usually one channel predominates. An example of this is given in Figure 4 for cross section 2.6 where the right bank should have been specified at station 10580 rather than 10850. Other examples of the same kind can be seen at cross sections 3.27 (Figure 5), 8.992 (Figure 6), and others. Note that this is a conservative approach if scour depths are the critical design considerations (rather than deposition depths).

The bank location problem should be remedied in the model geometry because it will greatly affect sediment transport calculations later in the movable bed models. For the movable bed models (described subsequently), the hydraulic parameters calculated within the defined channel will be used to compute the sediment transport capacity that is then applied over the entire cross section. It is also recommended to increase the resolution of the cross section by adding ground points between the bank stations where necessary, so that the bed can be properly adjusted during the vertical movement computations. When the model can adjust only one or two points within the movable bed limits to satisfy the sediment computations, unrealistic channel shapes may result. The same type of problem may arise when there are wide overbank areas defined by only a few ground points, and additional interpolated points should be added in this situation as well.

At certain cross sections, the bank stations were specified at the channel bottom: e.g. 28.12 (Figure 7), 28.67 (Figure 8), etc. X3 records (with 10 in the first field), used to confine the flow to the channel until the water surface exceeds the bank elevations, are not functional for such cross sections.

The Manning roughness coefficient in the channel ranges from 0.03 to 0.05. At several cross sections (e.g. 21.157, 21.431, 23.974, etc.) there is a sudden change in the channel roughness (from 0.035 to 0.05) but these values appear reasonable in light of areas with much denser vegetation in some areas of the channel and overbanks.

4. HEC-6 Existing Conditions Movable Bed Model

The KHA existing conditions movable bed model (ecafrev.dat) was constructed from the HEC-6 fixed bed model (affbrev.dat) as a basis for subsequent development of other movable bed models (full sand and gravel, encroachment, and grade control). Therefore, this review of the KHA sediment transport analysis concentrates on the features of the existing conditions HEC-6 model.

4.1 Geometry

The existing conditions model was based on the geometry of the fixed bed model with additional cross sections introduced at the locations of existing sand and gravel mines. The horizontal and vertical movable bed limits, additional X3 (encroachment) records, and XL (conveyance limits) records were also specified. Sediment data (inflow loads and bed gradation) and hydrology data (100-year hydrograph) were incorporated as well.

In order to visualize the existing model geometric features, its cross sections were imported into HEC-RAS graphical interface. These cross sections are shown in Appendix A. For convenience, movable bed limits (from HD records) are plotted as **Levee**. Conveyance limits (from XL records and X3.10 records) are shown as **Ineffective flow limits**, and **Encroached regions** (from X3 records) are shown as black areas.

The HEC-6 library version used in the KHA study does not have the capability to specify erosion limits independently of conveyance limits (XL records). Therefore, the erosion limits

need to be set up by conveyance limits that restrict erosion within the channel (banks), allowing deposition in the overbanks. An example of this is cross section 12.359 (Figure 9) which seems to be setting an ineffective flow area downstream of the Northern Avenue Bridge.

It was noted that some of the encroachment limits specified on X3 records were different from those used in the fixed bed model (e.g. cross sections 0.63, 21.061, etc.). These changes cause a difference in the hydraulic results between the two models. The modeler should verify that the changes were within acceptable limits. Also, for a few encroachment limits the decimal point was missing (compared to the limits used in the fixed bed model), thus increasing the stationing by an order of magnitude (cross sections 8.198, 14.430 in this model but not in the fixed bed model). These inconsistencies need to be checked and eliminated.

Movable bed limits should be revised at several locations. In areas experiencing mostly erosion, the movable bed should be limited to the “main channel.” In areas experiencing mostly deposition, the active flow area should be limited by using XL records, allowing deposition (but not erosion) in the overbanks. The reason for this strategy is that overbank areas are mostly depositional and are not usually lowered by erosion. In the HEC-6T model, simulating this situation is made somewhat easier by the availability of the HE record to define erosion limits independent of the conveyance limits specified on the XL records. However, this option is not available in the HEC-6 model. An abrupt change in movable bed widths was also noted at several cross sections: 1.33, 4.3, 8.433, 8.992, 10.343, 11.428, etc. The effort should be made to smoothly transition the changes in width by adding more cross sections at these locations.

Special attention should be paid to braided channels. Although in reality braided systems will avulse, for sediment modeling it is necessary to pick out the “main channel” (Figure 10) whose lateral dimensions will not change radically. Because HEC-6 only computes vertical bed movement and the concern is average bed elevation change, lateral shifting of the main channel is not important for the one-dimensional model (however, model results showing scour in the main channel should be considered to occur anywhere within the braided area for design purposes). An approximate main channel width can be identified using a channel forming discharge and regime theory. A 5- or 10-year event is commonly used in the Southwest for the channel forming discharge. Regime widths corresponding to the discharge can be computed using several equations (e.g., Neill’s approach as presented in U.S. Army Corps of Engineers, 1994, or the Julien and Wargadalam (1995) Equation). It is recommended to match this computed width to features in the cross section to identify the movable bed limits for braided channels.

4.2 Sediment Gradation

There is an obvious difference between the sediment sample gradations obtained from the Arizona State University (ASU) study (most collected in 1992) and those collected for the KHA study (collected in 2001). It is not known whether the discrepancies arise from variations in sampling methods or testing, or from remixing of the bed by flows in the river. It is also not known if the sampling sites were at bars or cross-overs or in active or inactive parts of the braided system. Apparently most of the samples from the two studies were collected from the surface (less than 3 feet depth) in the main channel of the Agua Fria River.

Log interpolation of the gradation data allowed Figure 11 through Figure 14 to be prepared, showing the variation of the 85th, 50th, and 15th percentile finer (by weight) grain size with distance along the river. The ASU gradations are coarser than the KHA ones from River Mile (RM) 0 to 16 and 26 to 34 (approximately). The KHA samples are coarser in the range RM 18-25, with the gradations in the remaining areas being roughly equal.

The sensitivity of the existing conditions KHA model (ecafrev.dat) to the sediment gradations was examined. The bed sediment records (PF records) in the KHA model were replaced by records corresponding to the gradations used in the ASU model and this new model was saved with the name ecafasu.dat. Average bed elevations at a discharge of 9,000 cfs after 30 days at that same flow were then compared for the two models (Figure 15 through Figure 21). The average bed elevations at the beginning of the simulations are identical for both models (curve labeled TS1 for Time Step 1). However, the results after 30 days at 9,000 cfs differ.

As might be expected, general results for the new model showed the average bed elevation higher than the KHA existing condition results where gradations were coarser (less erosion) and lower where the gradations were finer (more erosion). Average bed elevations from the new model when compared with those from the KHA model ranged from 4.5 feet higher (cross section 14.43) to 4.3 feet lower (cross section 24.54) with the average difference over the entire river reach being 0.1 feet. Overall, the net effect of the gradation change is very small as shown by the average difference of 0.1 feet. However, for certain reaches, a sustained difference over a long distance yields a considerable difference in sediment volume eroded or deposited. For example, in the reach between RM 21.893 and 23.409 the modified model results show the average bed elevation approximately 2 feet lower than the KHA results over a distance of roughly 1.5 miles. When viewed on a 1-2 mile reach scale, the volume differences can be considerable. This factor should be addressed in terms of model sensitivity since few, if any, significant flows have occurred between the sample periods.

4.3 Hydrology

The hydrology for the movable bed models consists of 30 days of 9,000 cfs (unchanged along the length of the river) followed by a hydrograph representing the 100-year flood event (with one flow change location at the New River confluence). According to the KHA report, the 9,000 cfs discharge is the anticipated 100-year release from New Waddell Dam. Details of development of the 100-year inflow hydrograph are also provided. The KHA report states that “the purpose of running the 9,000-cfs through the river for a long duration is to average out the bed of the river before hitting the river with the 100-year hydrograph and to simulate a “long-term” flow prior to the 100-year flood” (sic).

Although a “warm-up” period at the start of a simulation is normal, it usually consists of only a few time steps at the beginning discharge in order to allow the bed material gradations and bed elevations to become computationally compatible with the flow hydraulics. A common practice is to use an approximate bank-full discharge for several days before beginning the event hydrograph. As a “warm-up” period, 30 days appears excessive. Also, it is not clear why it is desirable to “average out” the bed of the river for a 100-year dam release before simulating a 100-year flood event. A 100-year release does not seem representative of the long-term behavior of the river unless this is also considered to be the channel forming discharge. However, no

indications were given in the report that this was the modeler's intent. In addition, it is expected that simulation of the 100-year hydrograph at the beginning of the time series of flows will give different results than the current model at the end of the series. For example, in a depositional zone, placing the 100-year hydrograph at the beginning of the time series will usually yield "worst-case" scour results when compared to those obtained when the hydrograph is at the end of the time series of flows. A better case needs to be made as to why the current model hydrology is representative of both long-term and single event (100-year) behavior.

4.4 Inflowing Sediment Loads

The KHA report states:

The development of the inflow curve for the Agua Fria River follows the methodology provided in the materials for the HEC-6 short course given by the Flood Control District in November 1999 and the guidelines published in "Applications of Hec-6 to Ephemeral Rivers of Arizona" (Report Number: FHWA/AZ86/214 prepared by Arizona State University for the Arizona Department of Transportation, January 1986) (sic).

The report does not elaborate on what these methodologies are or how they were implemented. Also a description was not found of how the sediment inflow rating curve was developed for the New River at the confluence with the Agua Fria River.

Although the report states in several places that New Waddell Dam releases are essentially clear water, the inflowing sediment load rating table for the upstream end of the model does not reflect this. The table in the existing conditions model (ecafrev.dat) shows 12,913 tons of sediment per day entering the model for a flow rate of 5,000 cfs with increasing amounts for larger flows. Sediment from all size classes from very fine sands to very coarse gravels are allowed to enter the river model. A sensitivity to inflowing sediment load is carried out with other models (ec10yang.dat and ec50yang.dat), but it is not clear why there should be any inflowing load at all as the most upstream cross section is located immediately below the dam.

5. Other Movable Bed Models

Movable bed models were also developed by KHA for Manning's 'n' value sensitivity (ecval.dat), reduced sediment inflows (ec10yang.dat and ec50yang.dat), full sand and gravel mining activities (sgbase50.dat and sgm1-50.dat), full encroachment to the floodway limits (ench.dat), and proposed grade control (enchgcs.dat). However, these models are all derived from the existing conditions movable bed model (ecafrev.dat) reviewed in the previous section of this report. Because of this, the focus of this review is on the existing conditions model. The problems outlined in the preceding section affect all the movable bed models.

6. Summary of Model Review Findings

The KHA series of sediment transport models prepared for the Agua Fria River watercourse master plan have several deficiencies serious enough to warrant correction of the models. These include:

- The hydraulic base model (fixed bed model) used to develop the subsequent movable bed models should be revised. Additional cross sections are necessary at locations with sudden velocity changes, specifically where the energy equation cannot be balanced and no change in flow regime occurs. Abrupt expansion/contraction of the water surface width should be limited to recommended lateral ratios by conveyance limits. Discontinuous features in the channel geometry need to be defined by conveyance limits both upstream and downstream of the flow obstruction. The effective flow area in some cross sections should be restricted to the main channel only. Encroachments added later in the movable bed model should be input to the fixed bed model as well to verify that the river hydraulics are still within acceptable limits of the FIS study results.
- Special attention should be paid to the specification of bank stations in the fixed bed model to better reflect the channel hydraulics that will be subsequently used for the computation of sediment transport capacity in the movable bed models. The banks should be identified and should consistently identify the (continuous) main channel to the extent possible. Another recommended is to increase the resolution of the cross section by adding ground points (both in the channel and the overbanks) where necessary, so that the bed can be properly adjusted during the vertical movement computations.
- The existing conditions (movable bed) model, used to derive other movable bed models, should be revised as well. Because the HEC-6 library version does not have the capability to specify erosion limits independently of conveyance limits, the former needs to be established by conveyance limits that restrict erosion within the channel (banks), allowing deposition in the overbanks. There are some inconsistencies in the specification of encroachment limits (compared to the fixed bed model) that should be verified and corrected where necessary.
- Movable bed limits should be revised at many locations. In areas experiencing mostly erosion, movable bed should be limited to the “main channel.” In areas experiencing mostly deposition, the active flow area should be limited by using XL records, allowing deposition (but not erosion) in the overbanks. The effort should be made to smoothly transition the changes in movable bed width by adding more cross sections at particular locations.
- It is not clear why the hydrology used should be considered representative of the long-term behavior of the river.
- The sediment entering the upstream end of the model is at odds with the statement that essentially clear water is released from the dam.

7. References

Julien, P.Y., and Wargadalam, J. (1995). "Alluvial Channel Geometry: Theory and Applications," *Journal of Hydraulic Engineering*, September, pp. 312-325.

U.S. Army Corps of Engineers (1994). "Channel Stability Assessment for Flood Control Projects," *Engineer Manual 1110-2-1418*, October.

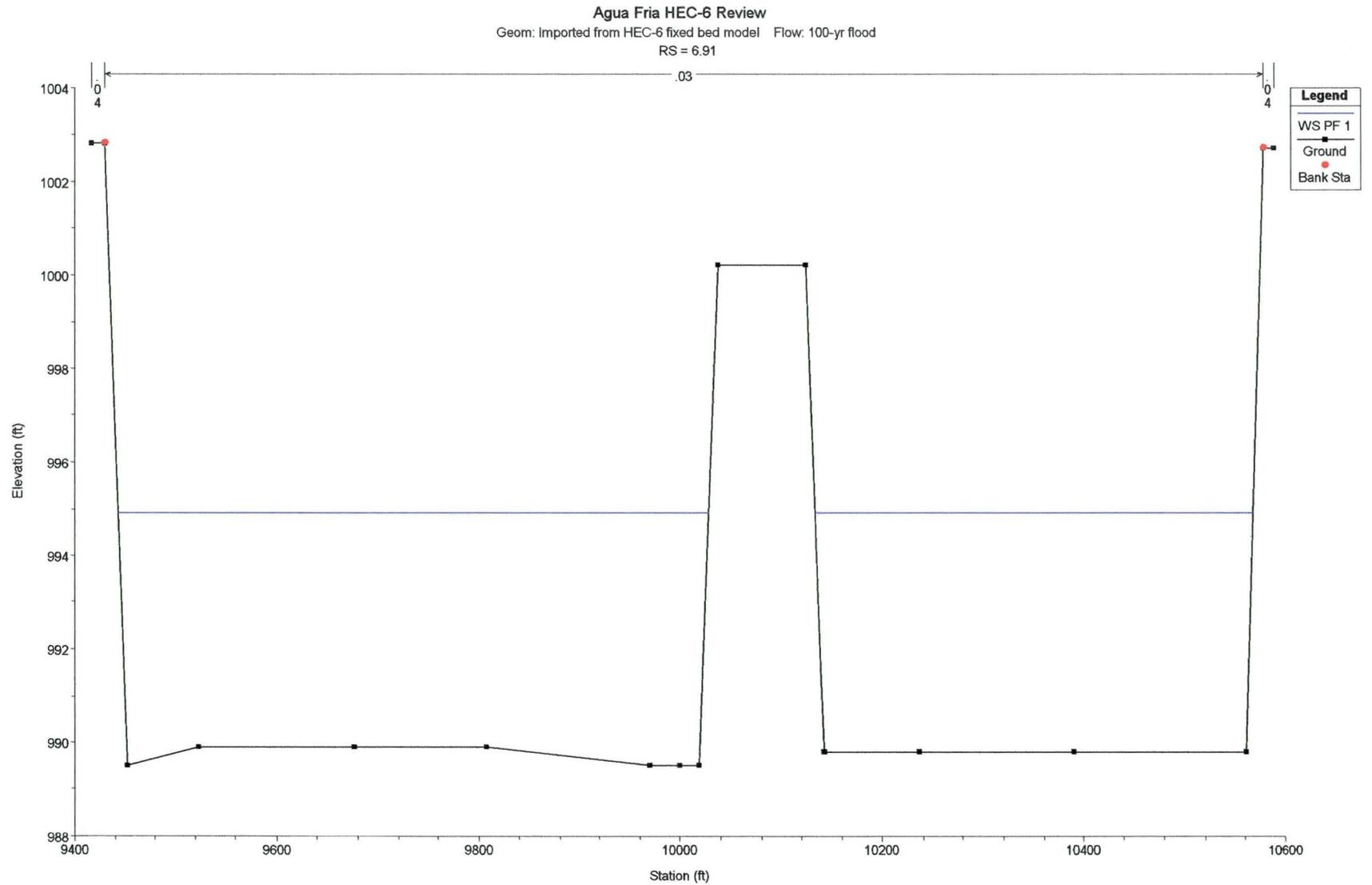


Figure 1. Feature at XSEC 6.91 requiring upstream/downstream ineffective flow limits.

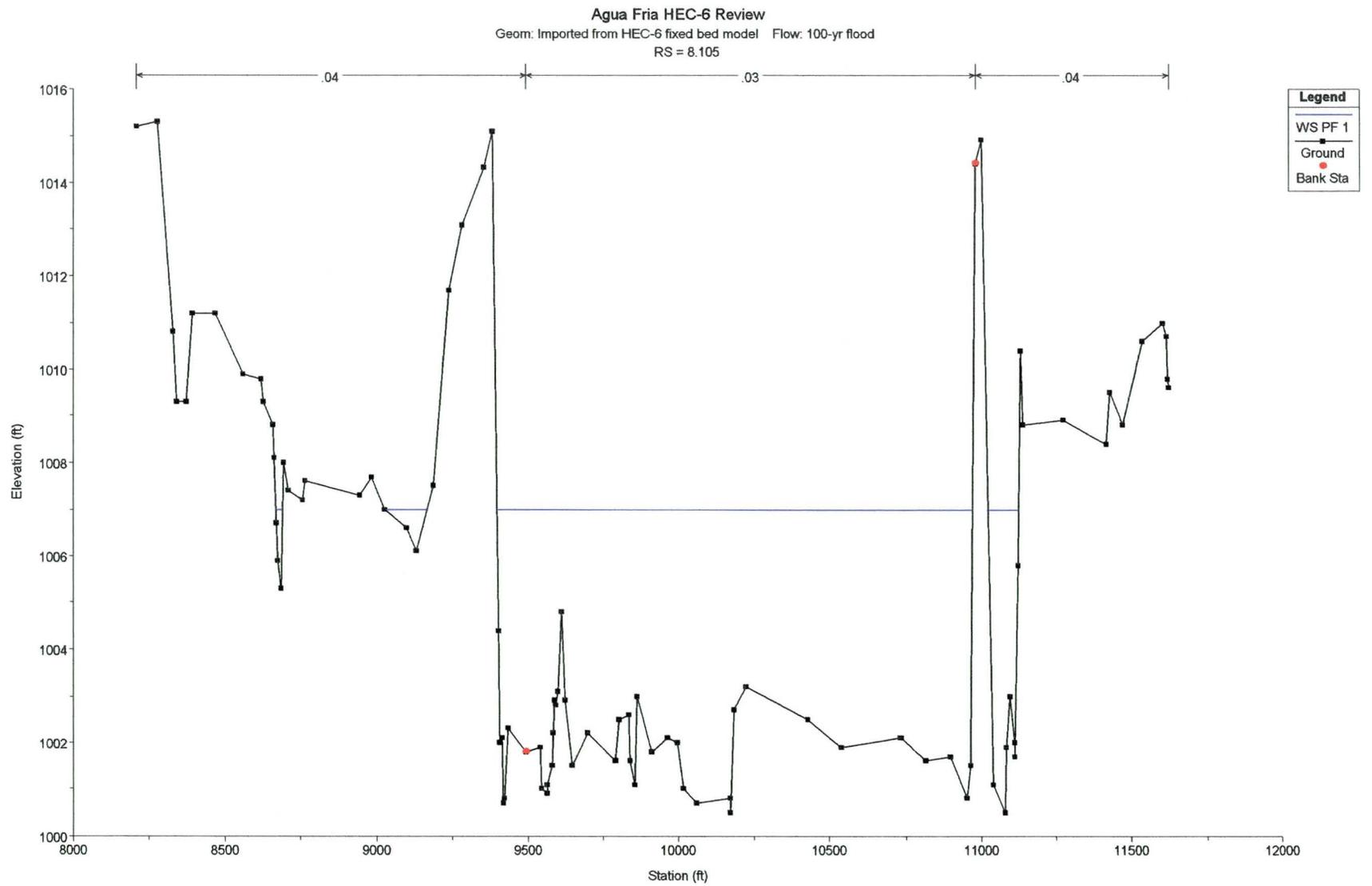


Figure 2. Blocked flow necessary at XSEC 8.105 behind both levees.

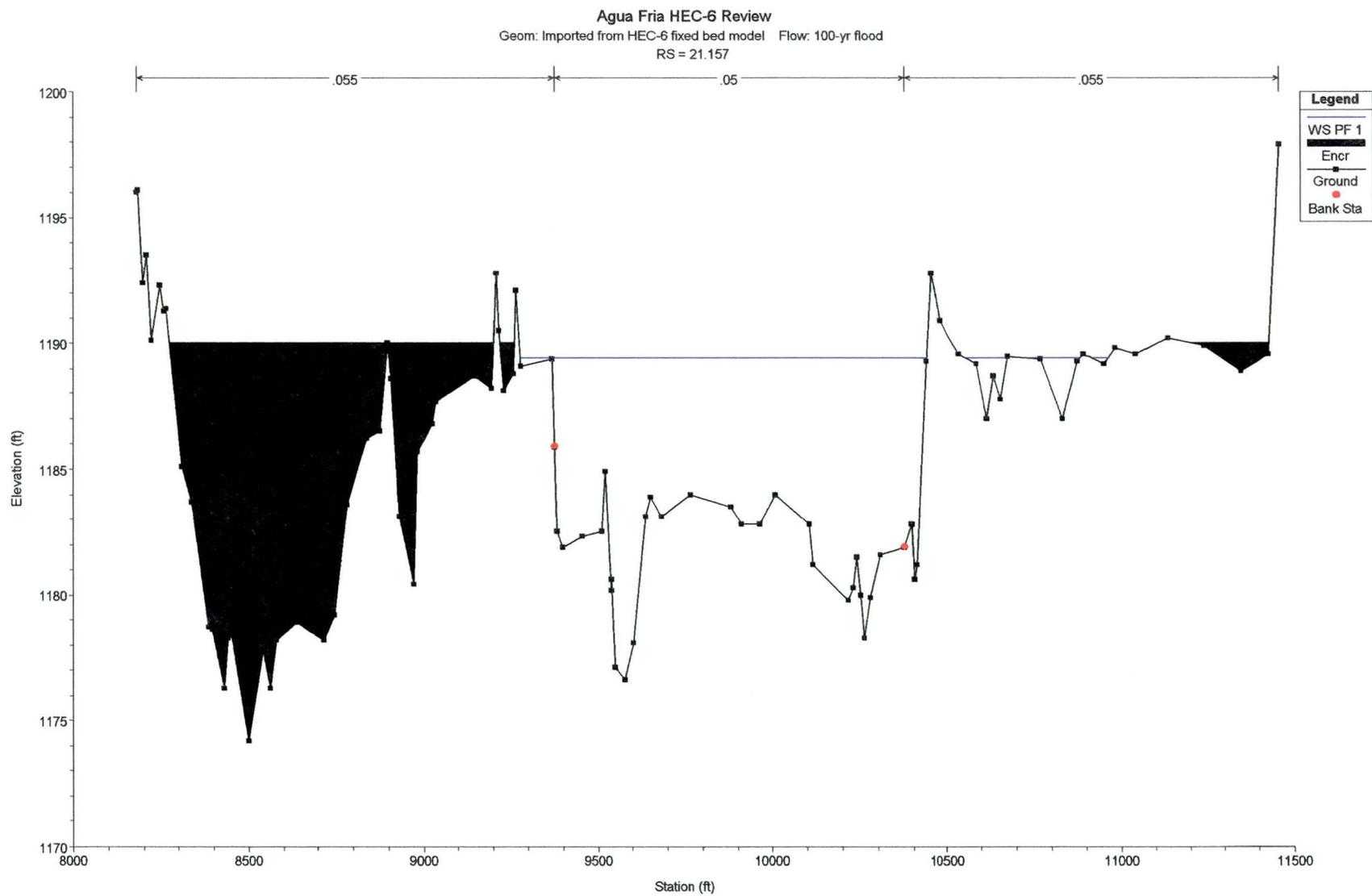


Figure 3. Blocked flow necessary at XSEC 21.157 behind the right levee.

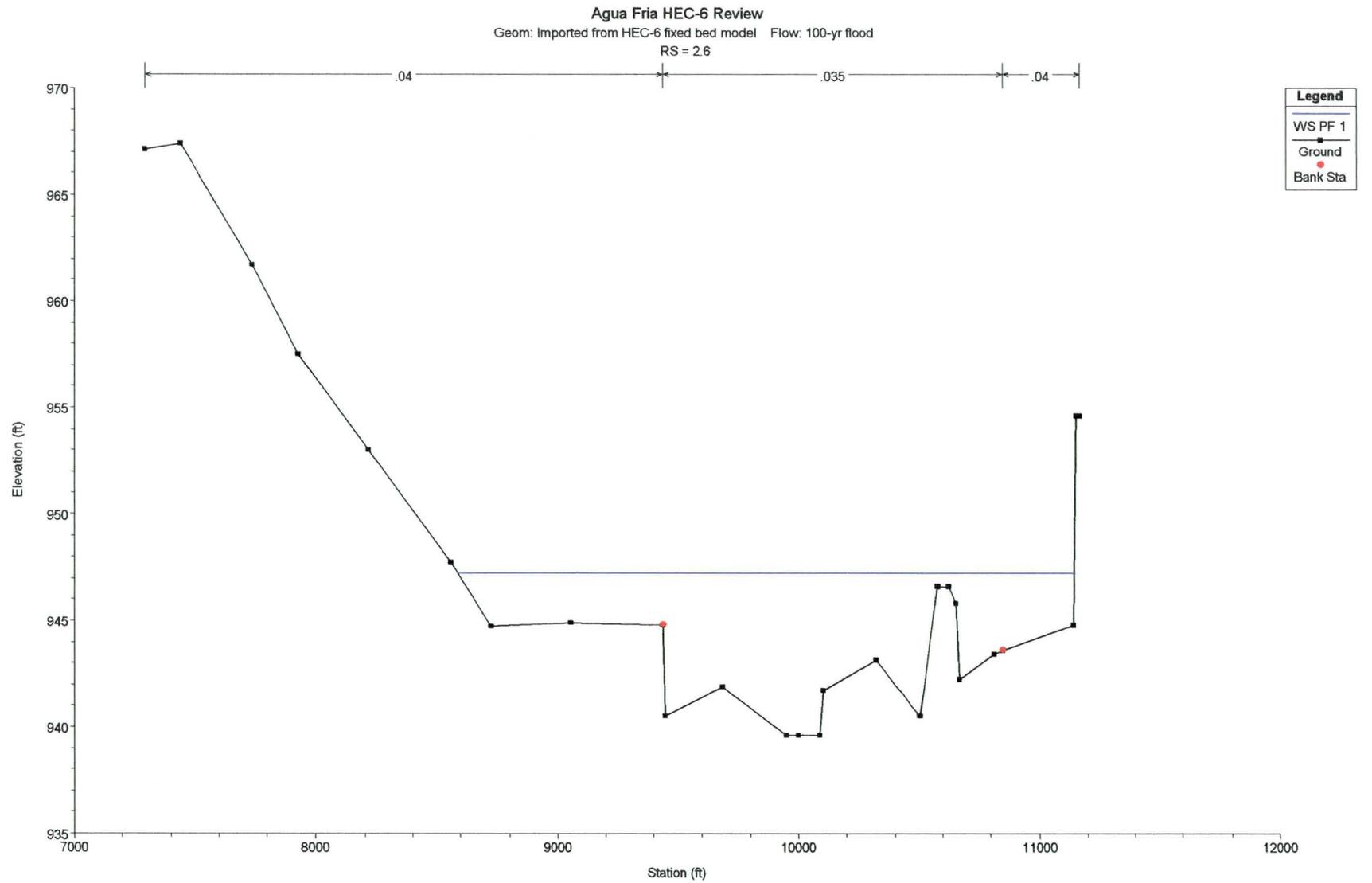


Figure 4. Inappropriate right bank station at XSEC 2.6.

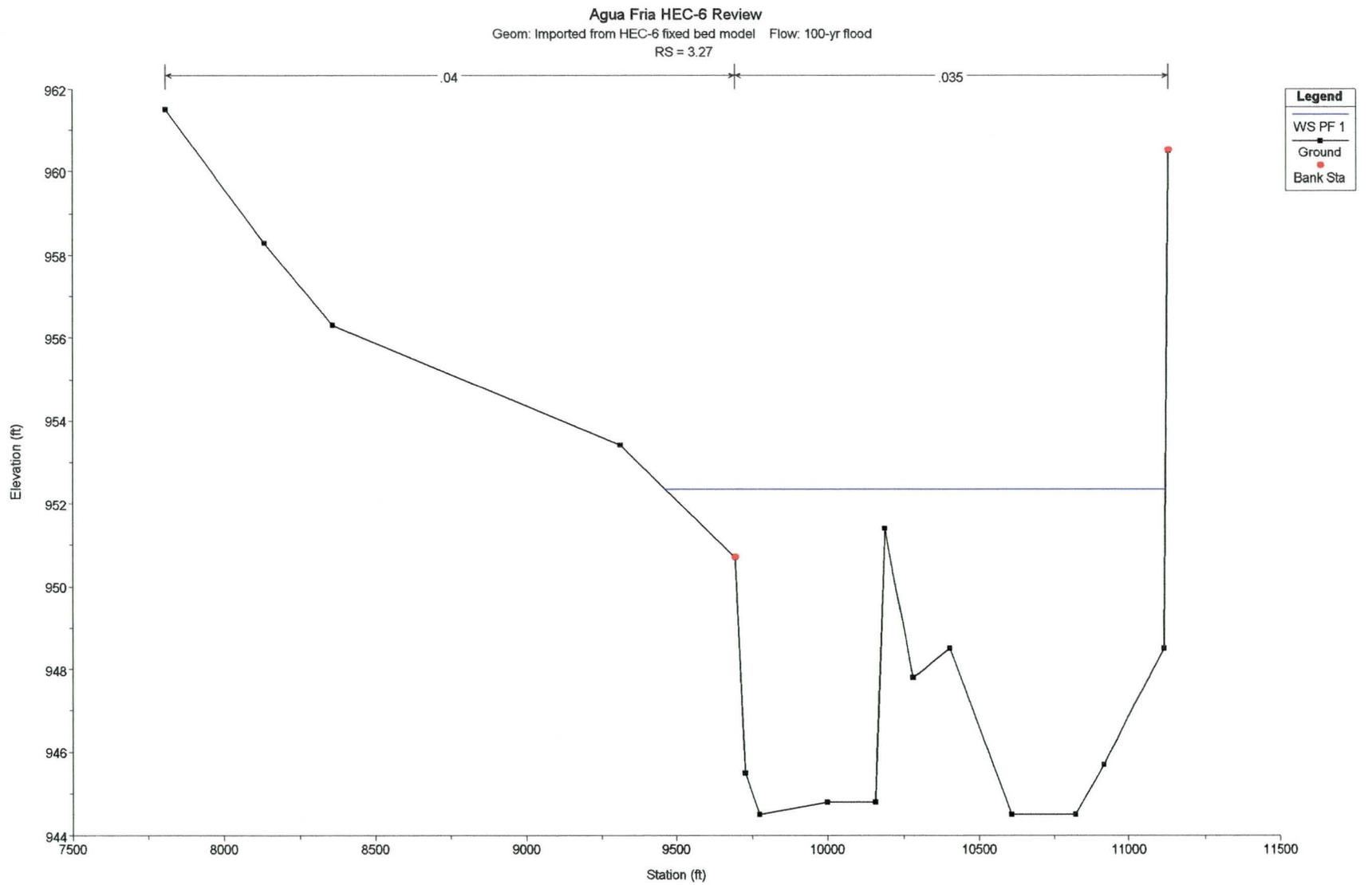


Figure 5. Inappropriate left bank station at XSEC 3.27.

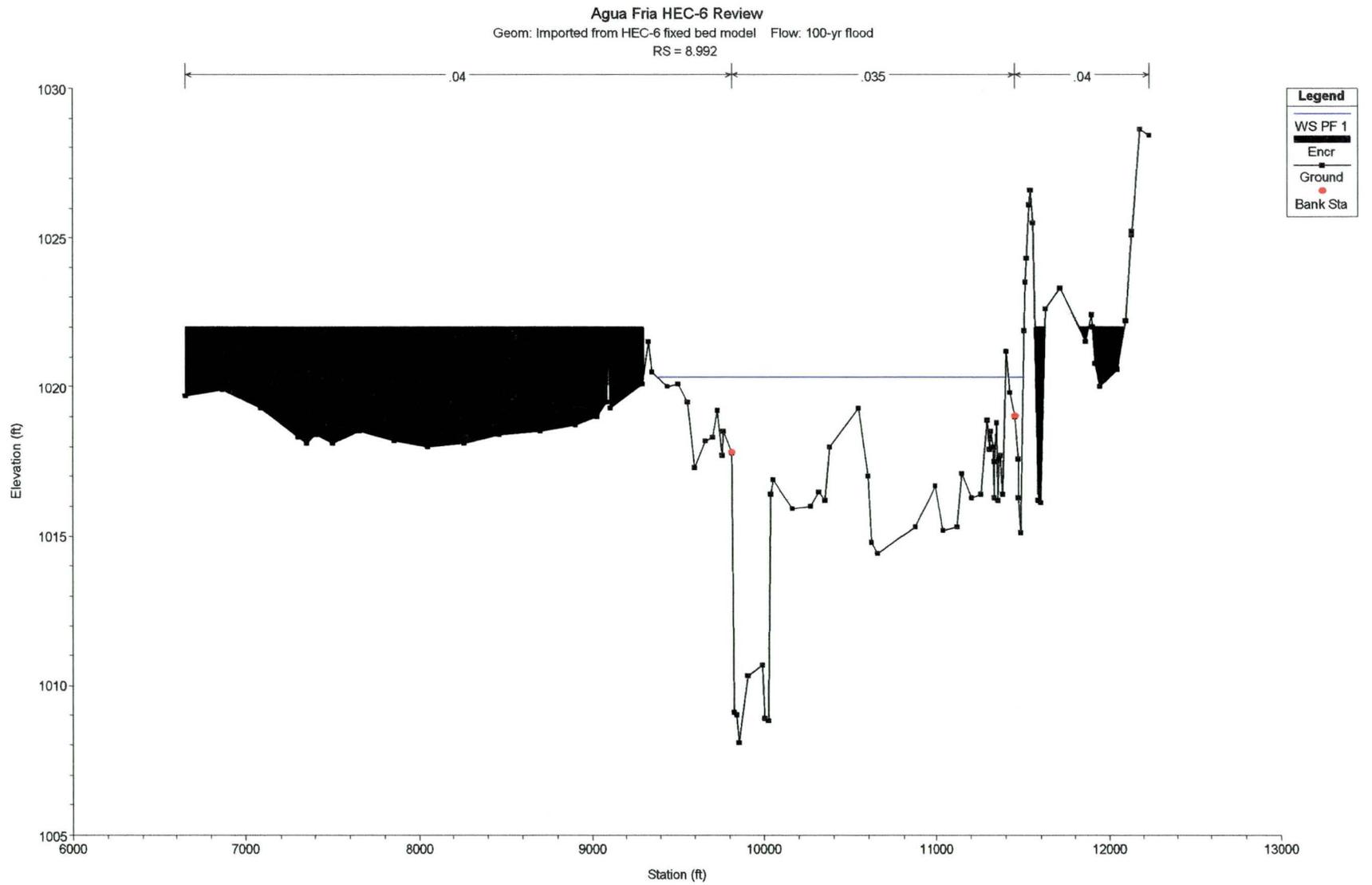


Figure 6. Inappropriate right bank station at XSEC 8.992.

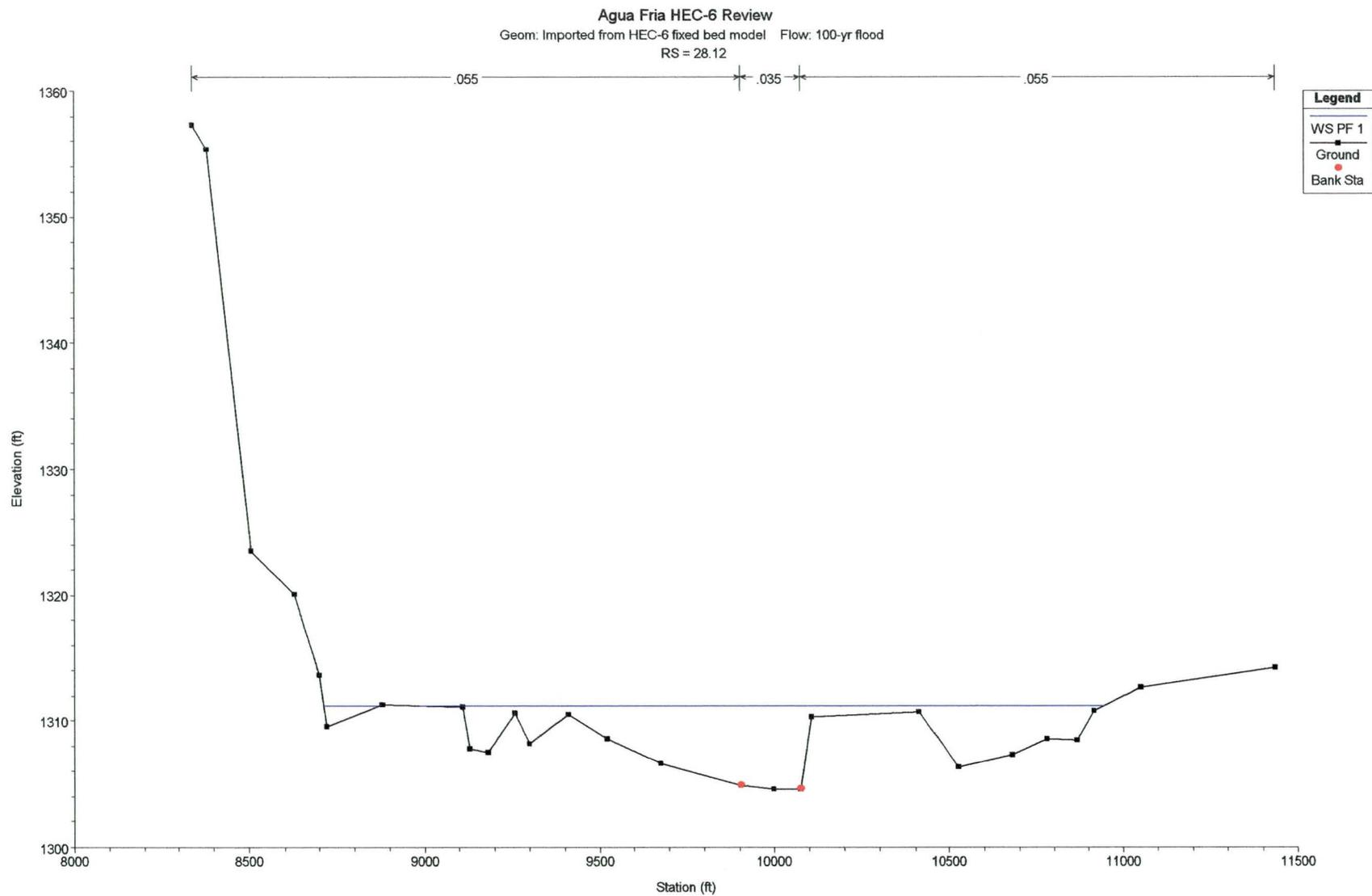


Figure 7. Bank stations on the channel bottom at XSEC 28.12.

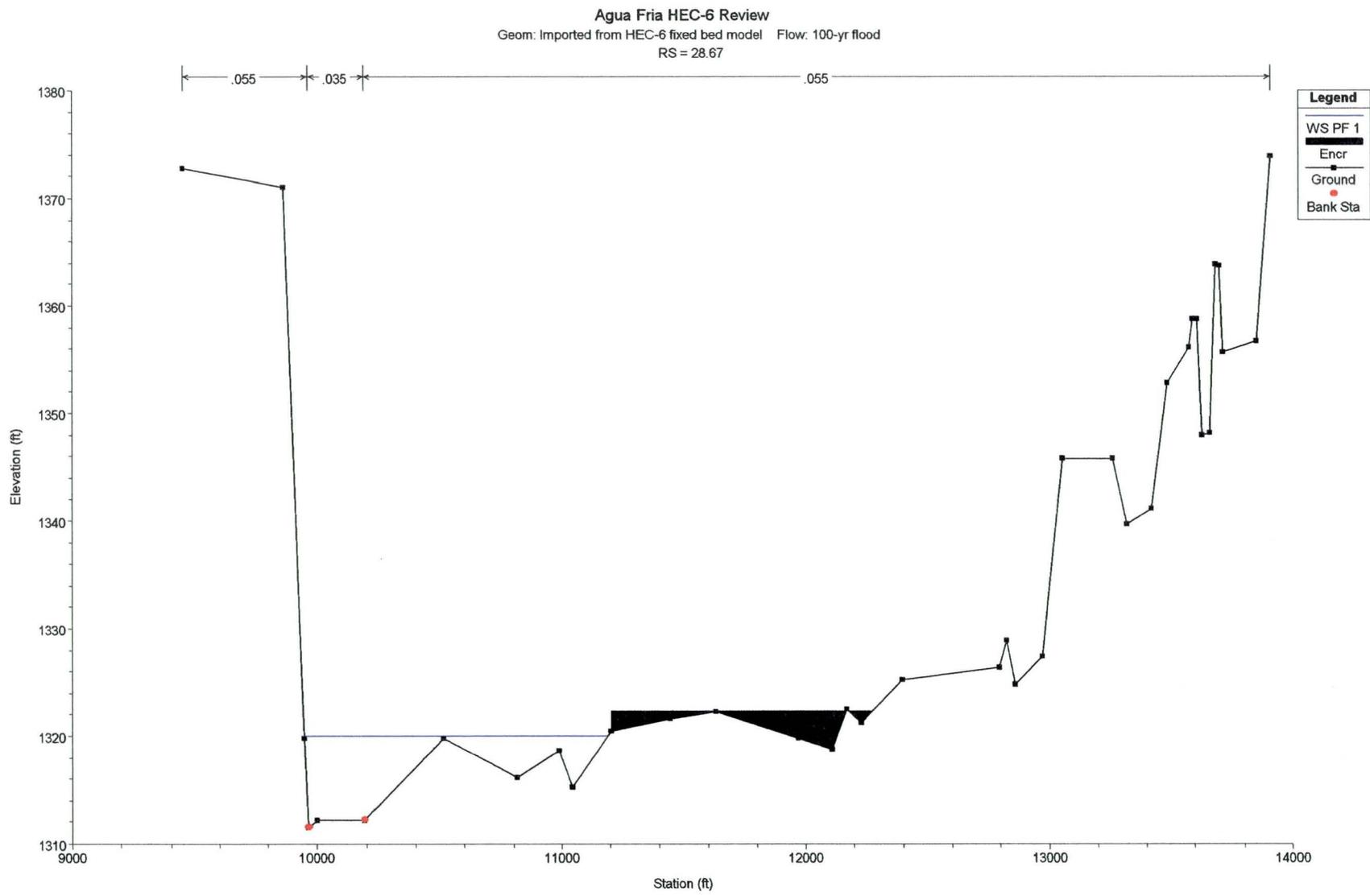


Figure 8. Bank stations on the channel bottom at XSEC 28.67.

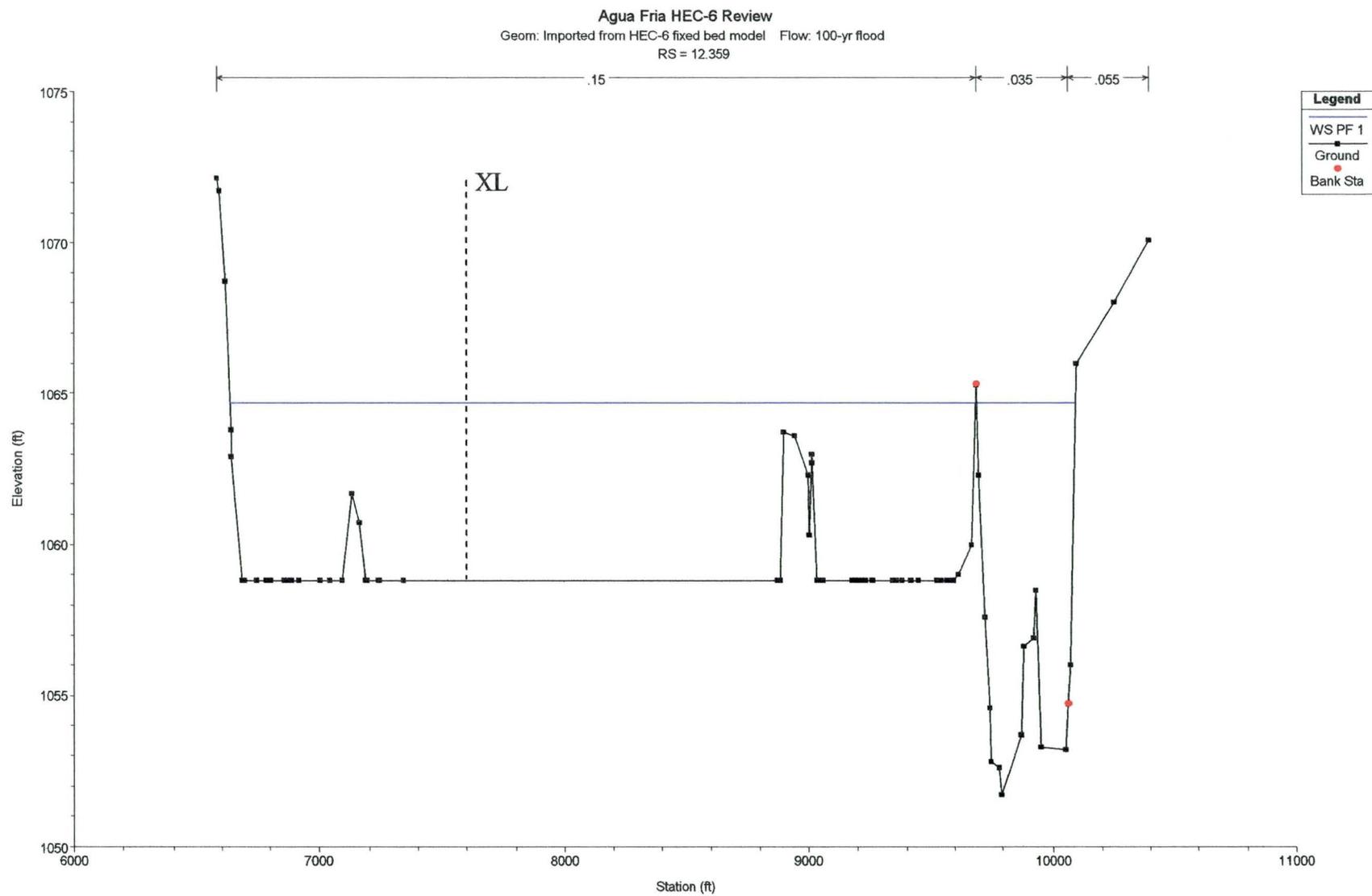


Figure 9. Inappropriate left conveyance limit (XL) at XSEC 12.359.

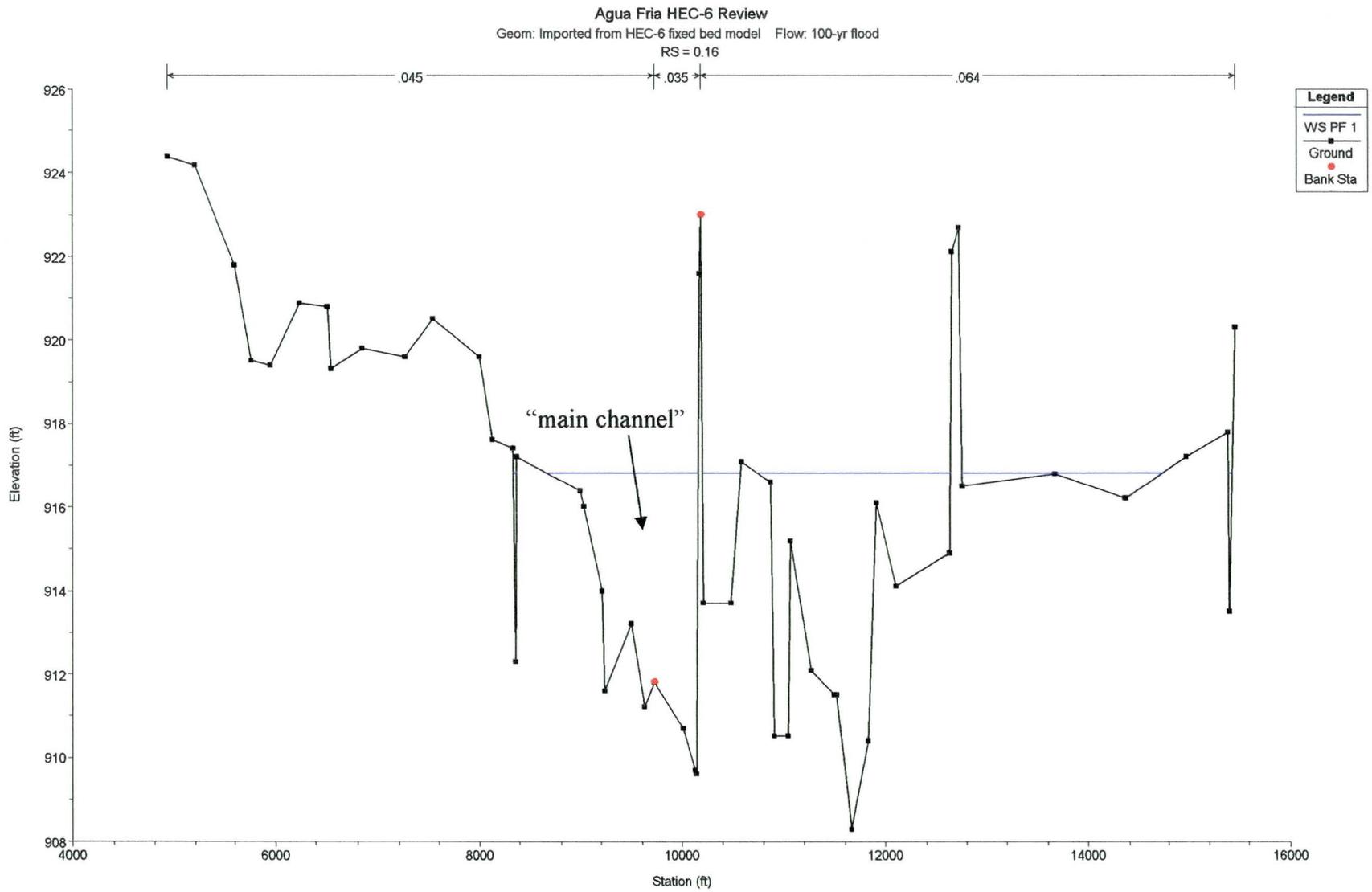


Figure 10. Braided channel at XSEC 0.16.

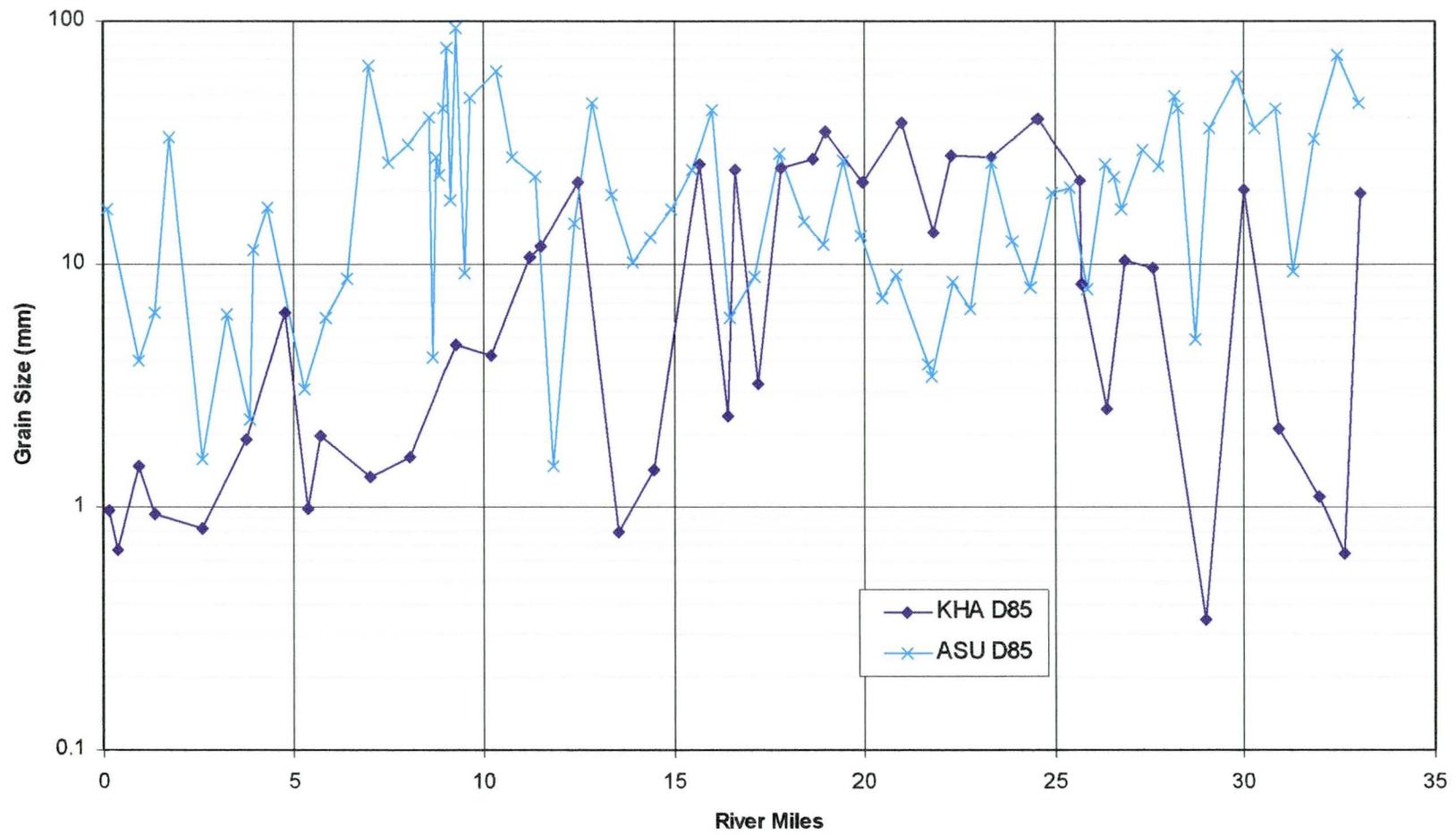


Figure 11. Sediment Gradation Comparison for D₈₅.

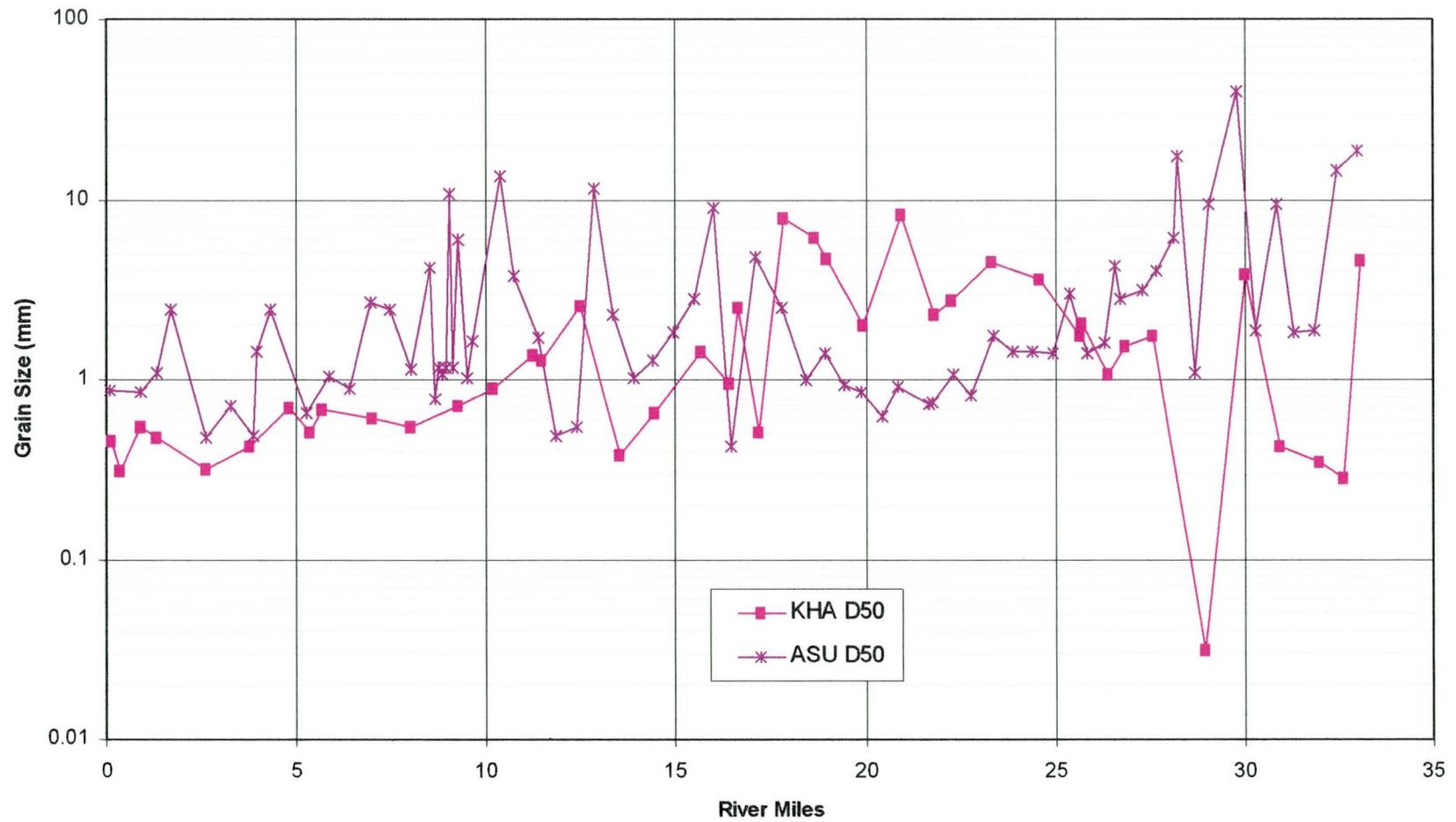


Figure 12. Sediment Gradation Comparison for D₅₀.

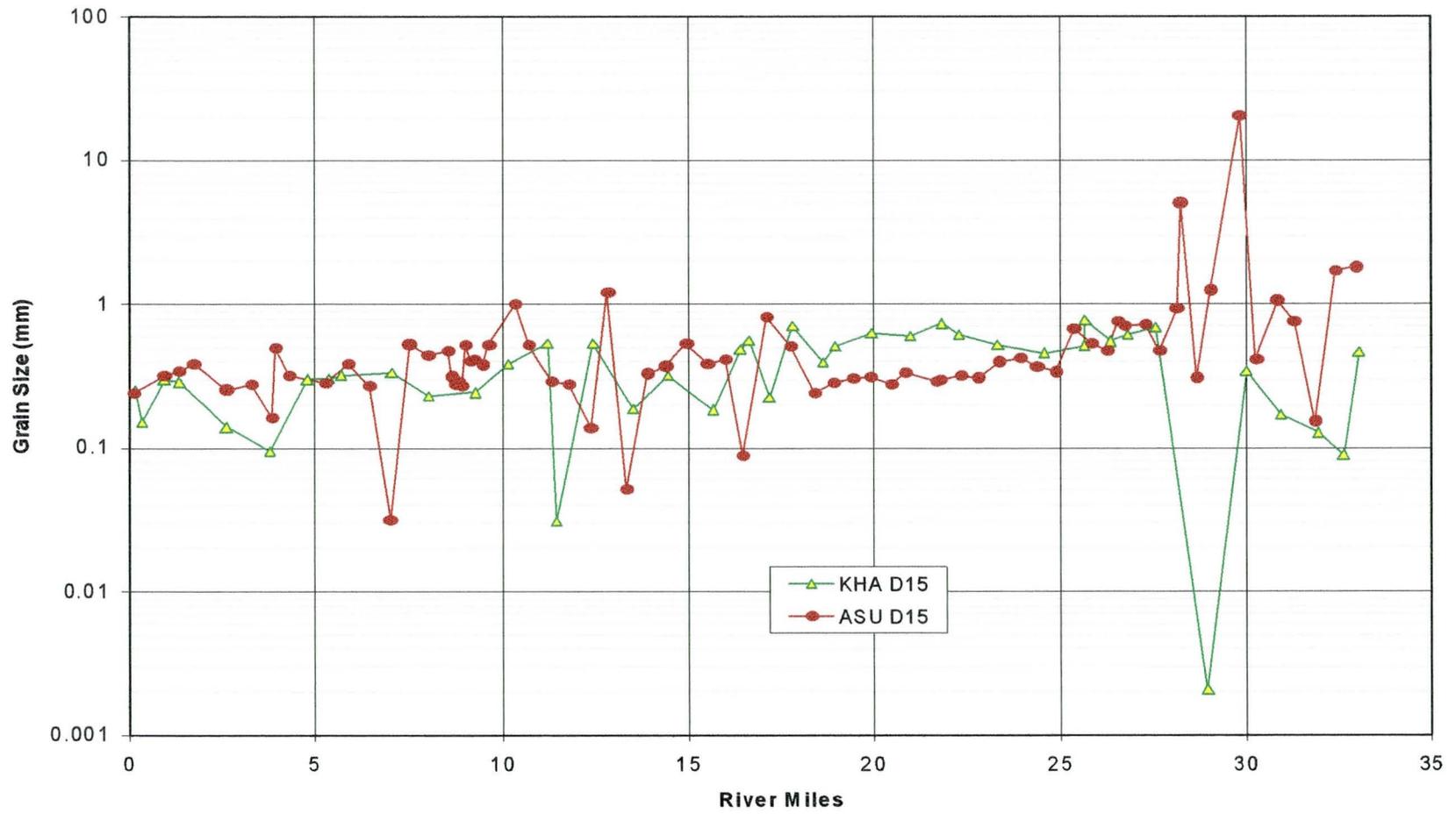


Figure 13. Sediment Gradation Comparison for D₁₅.

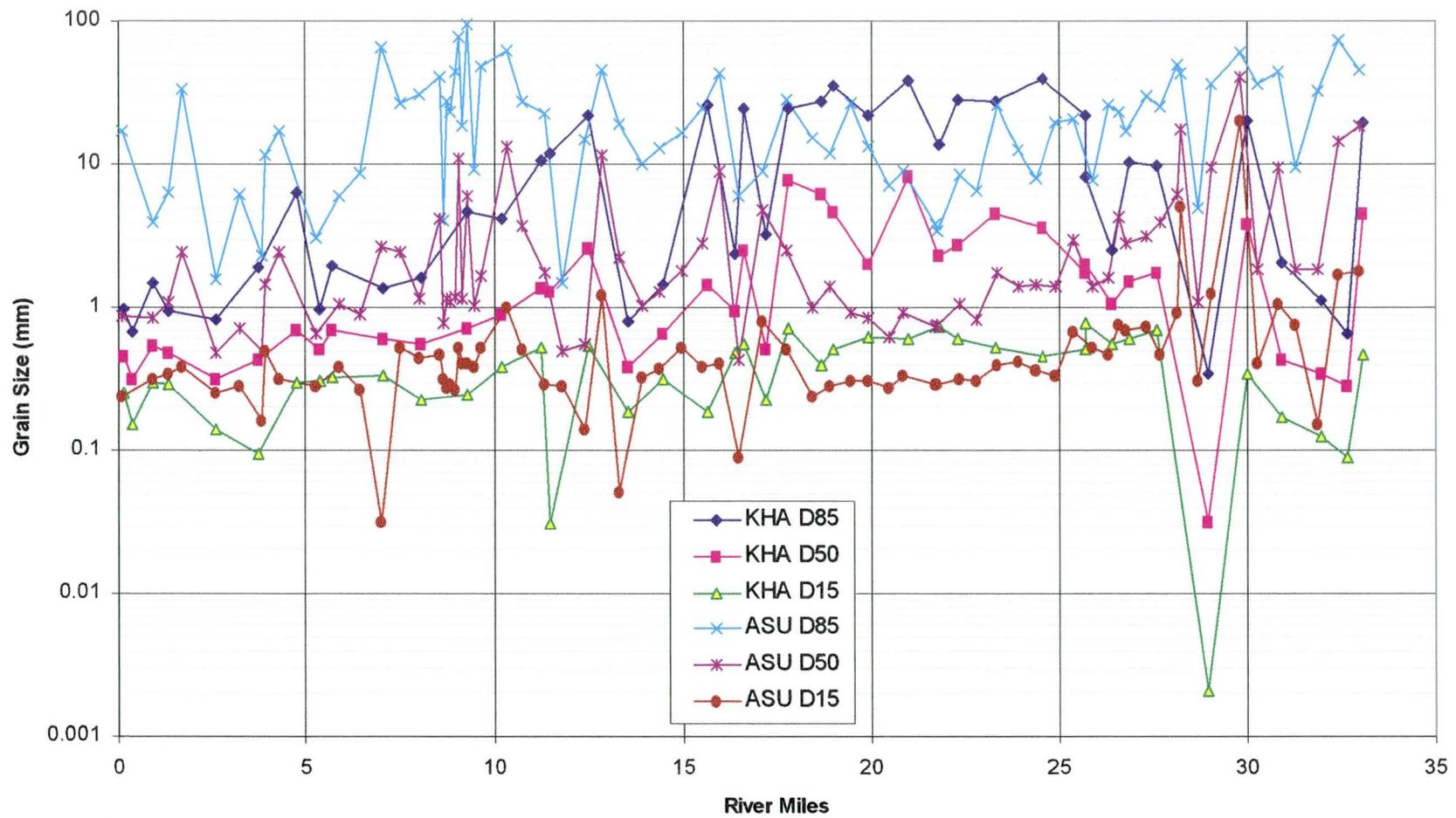


Figure 14. Overall Sediment Gradation Comparison.

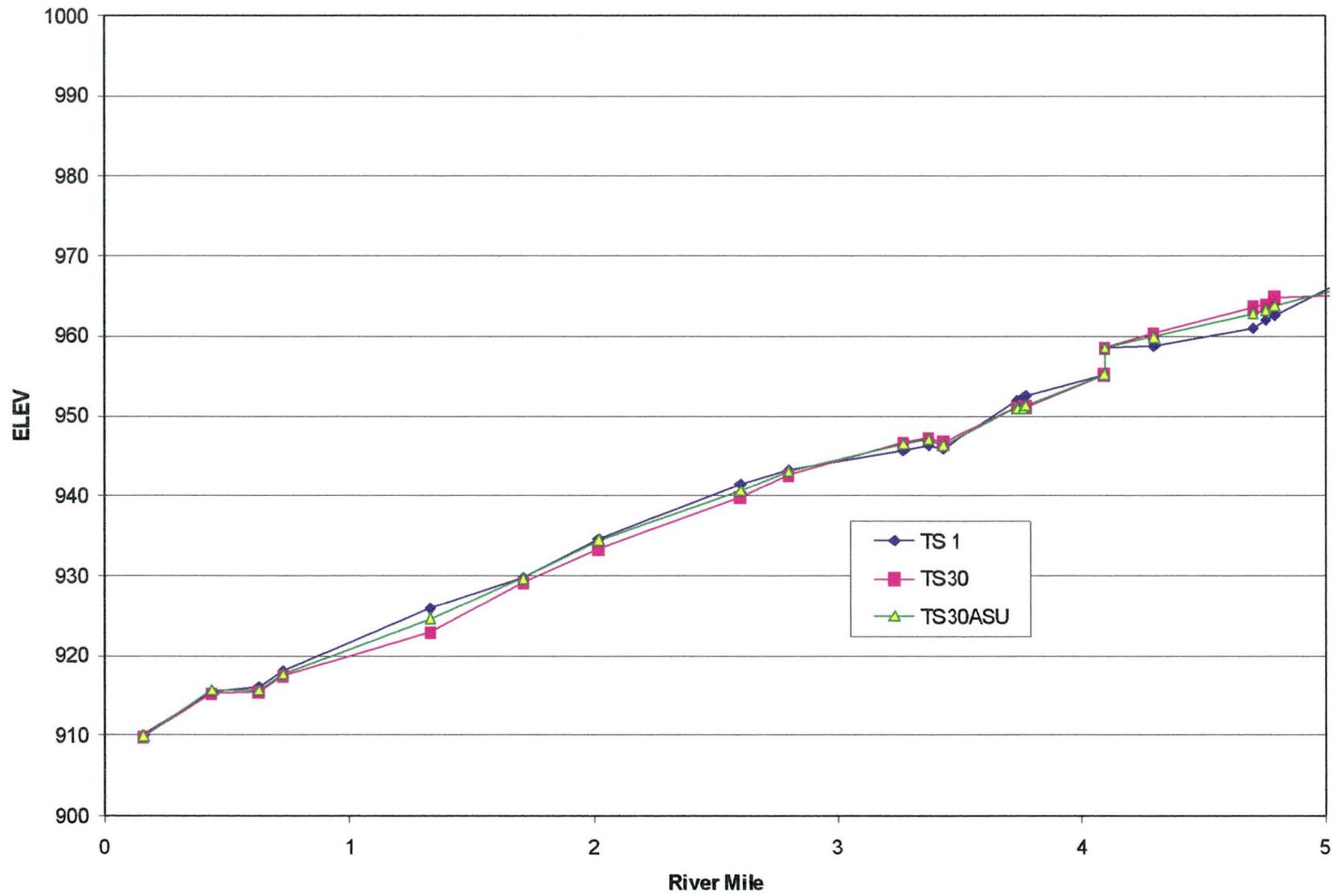


Figure 15. Average Bed Elevation RM 0.0 to RM 5.0.

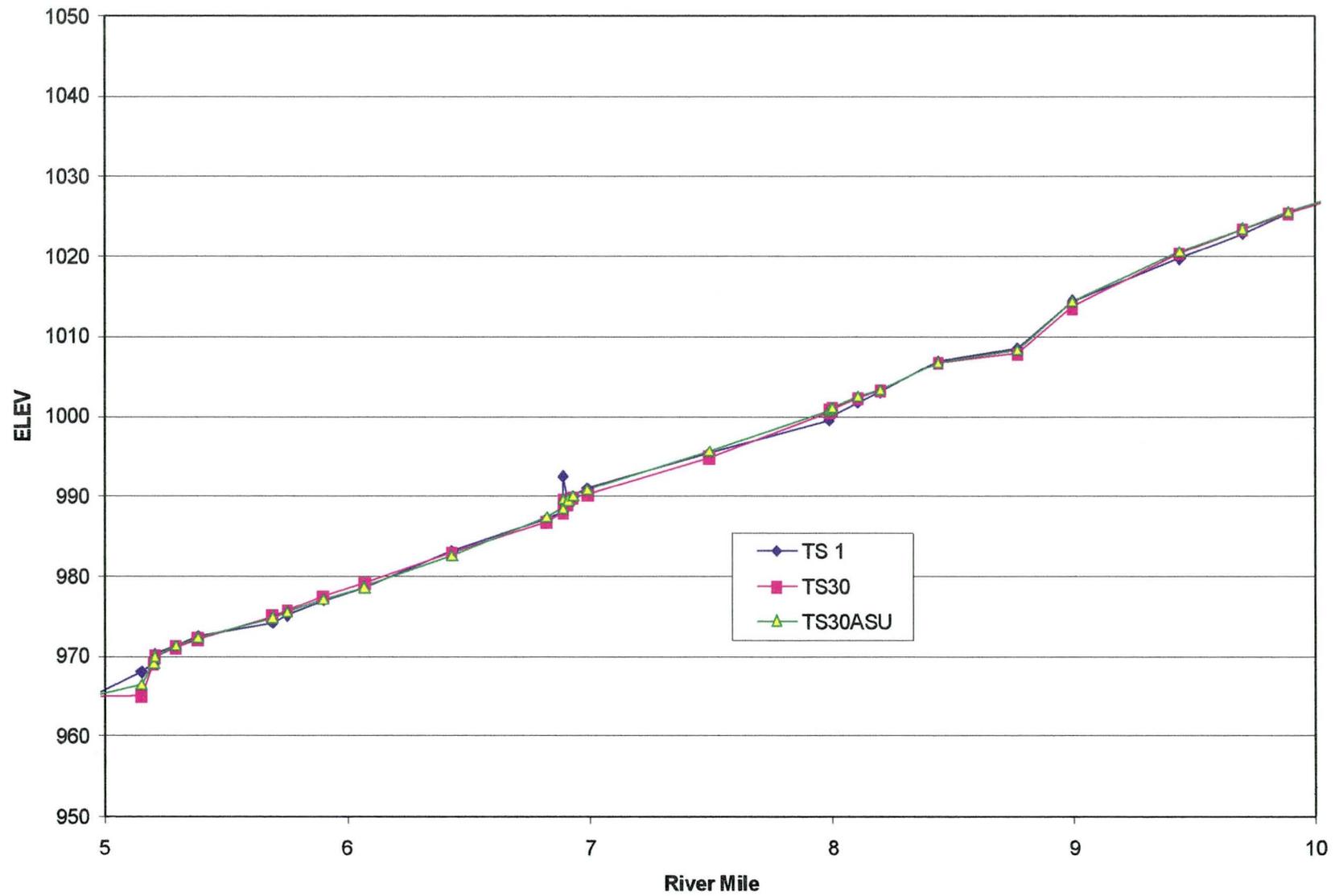


Figure 16. Average Bed Elevation RM 5.0 to RM 10.0.

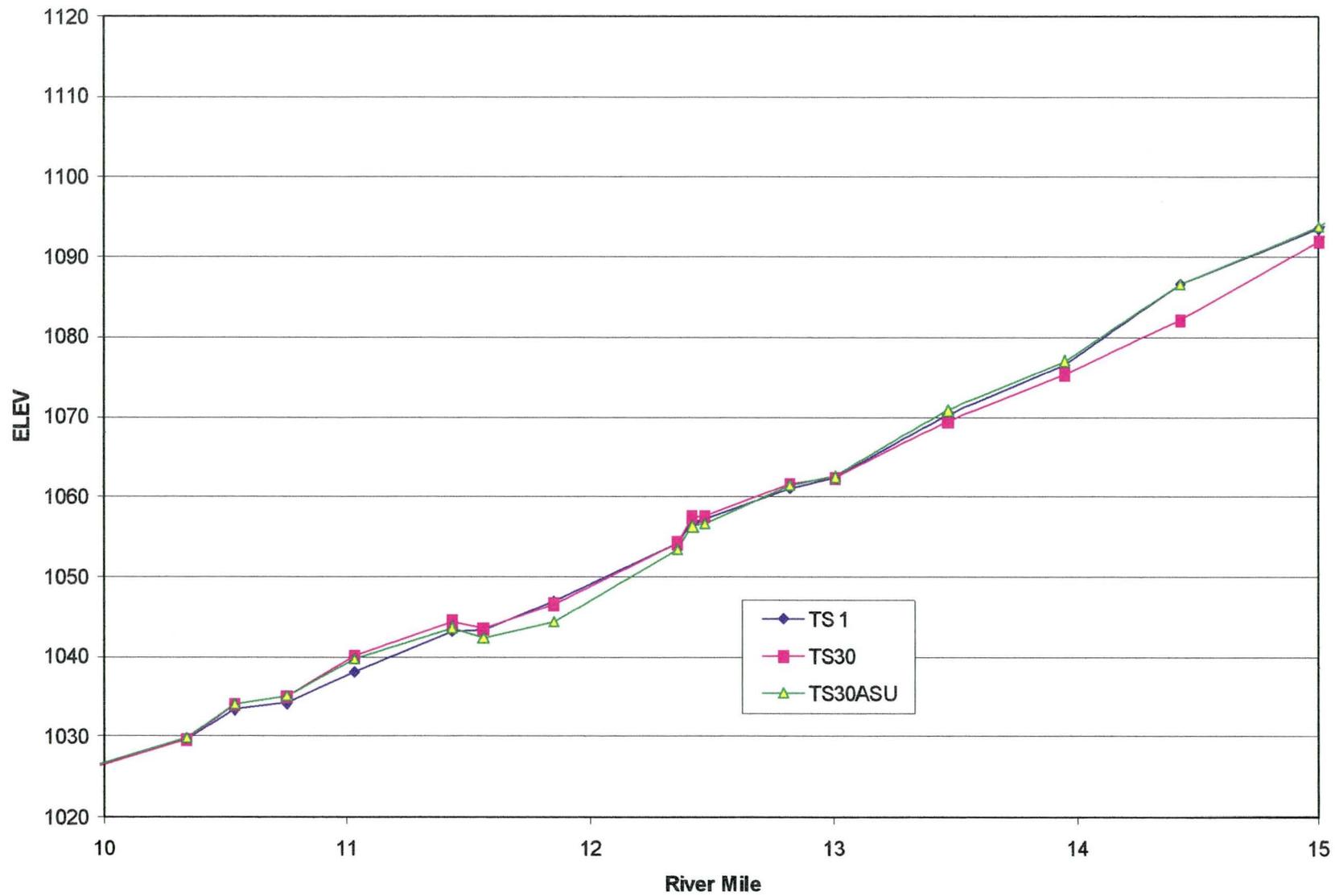


Figure 17. Average Bed Elevation RM 10.0 to RM 15.0.

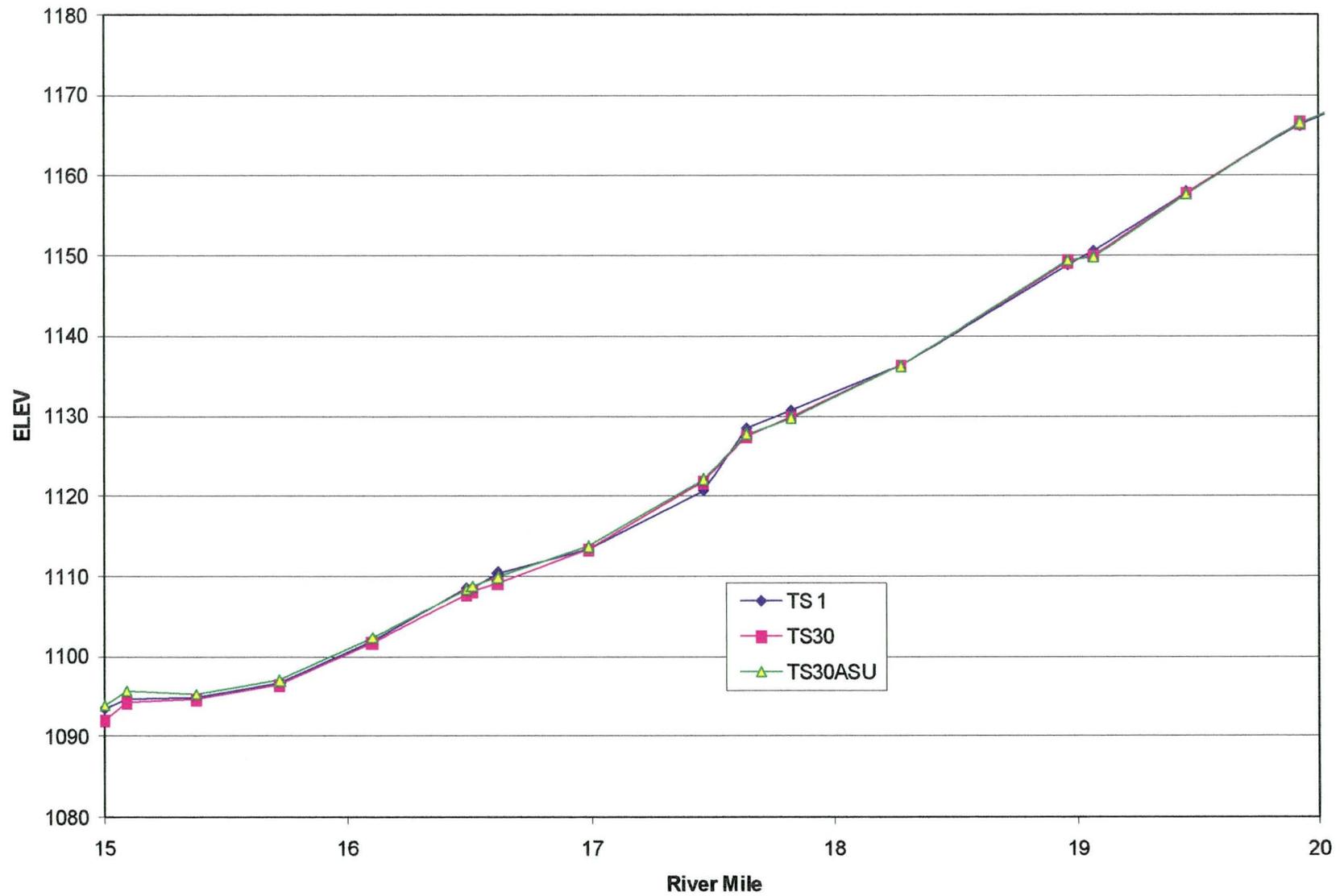


Figure 18. Average Bed Elevation RM 15.0 to RM 20.0.

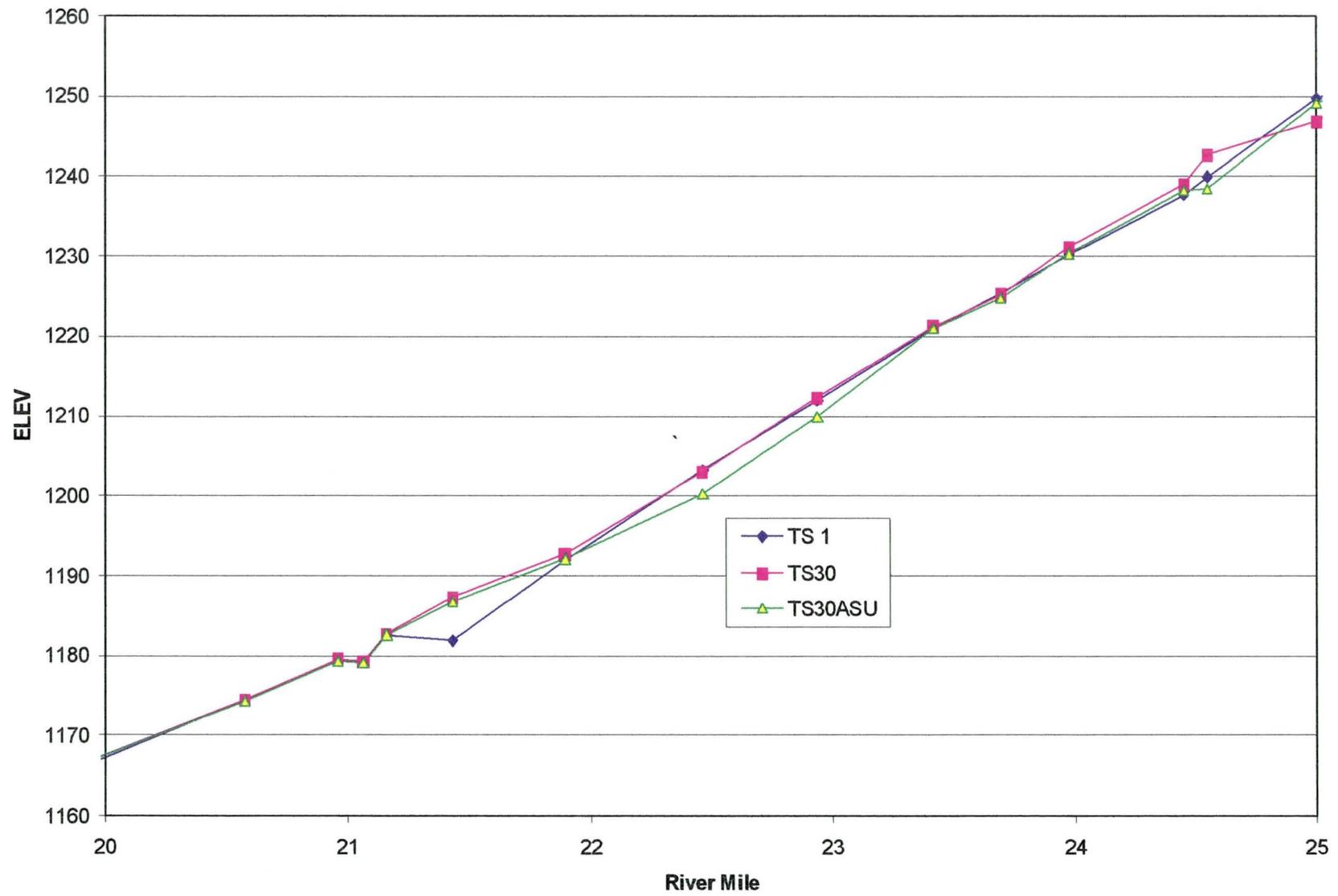


Figure 19. Average Bed Elevation RM 20.0 to RM 25.0.

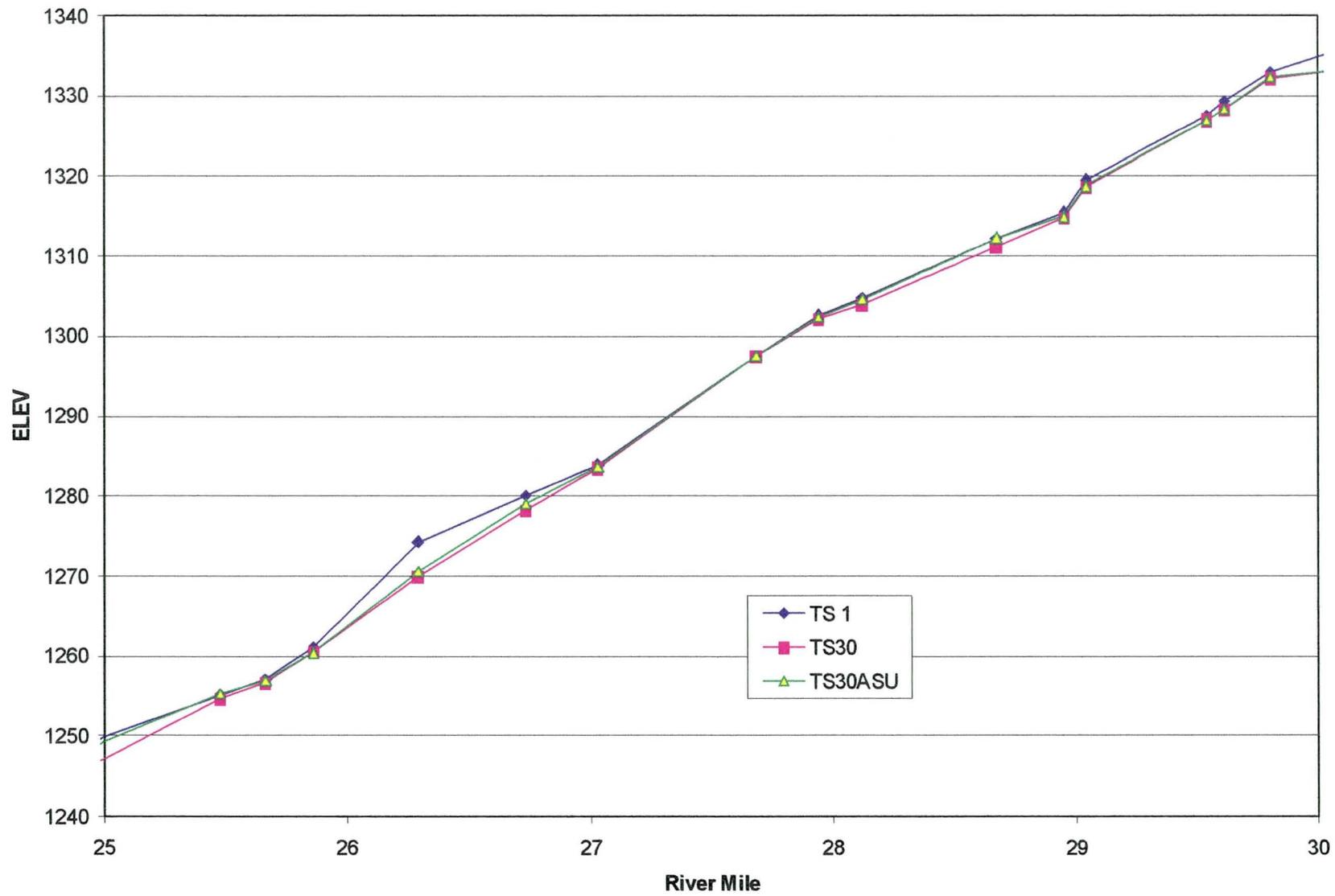


Figure 20. Average Bed Elevation RM 25.0 to RM 30.0.

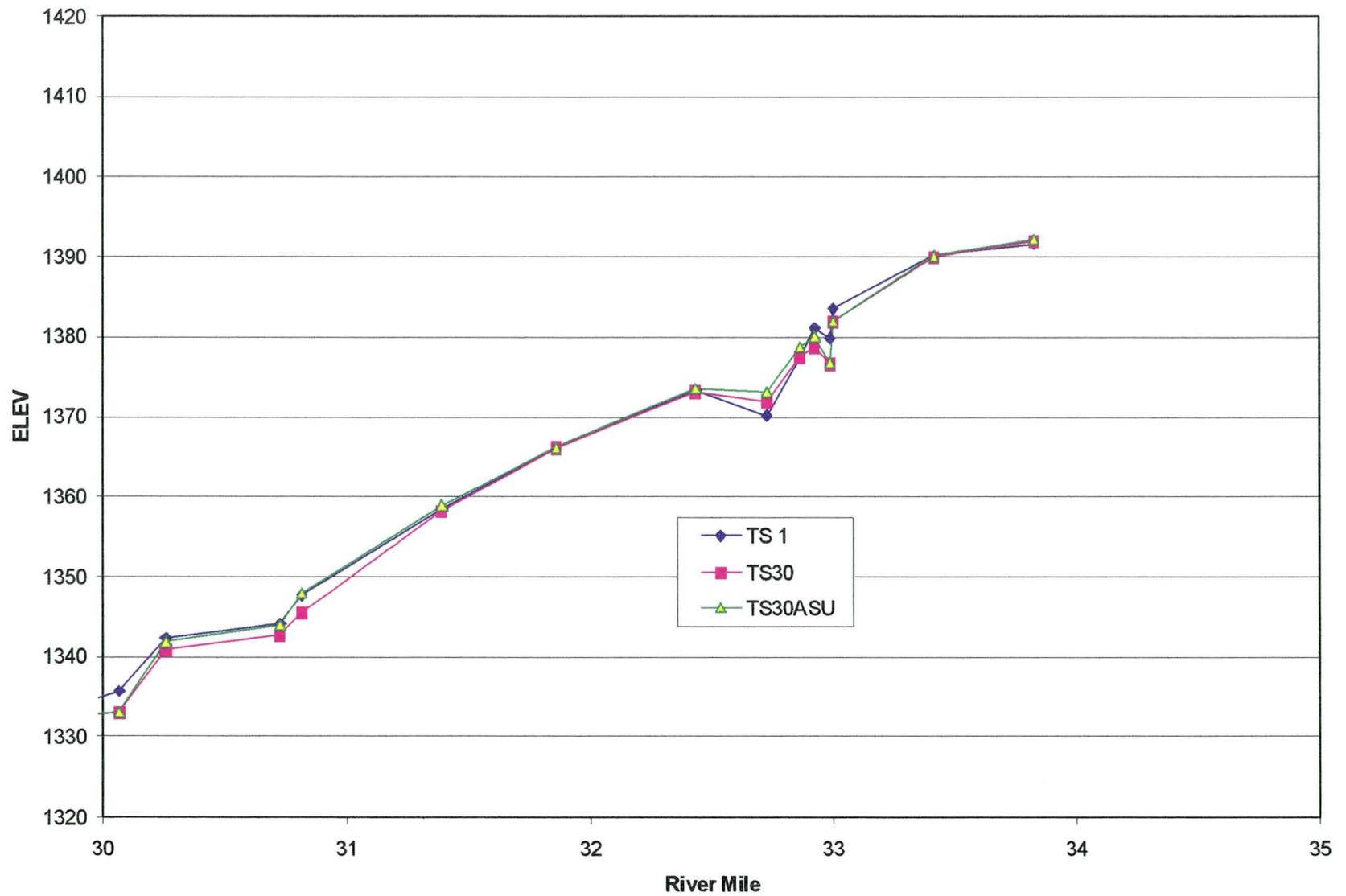


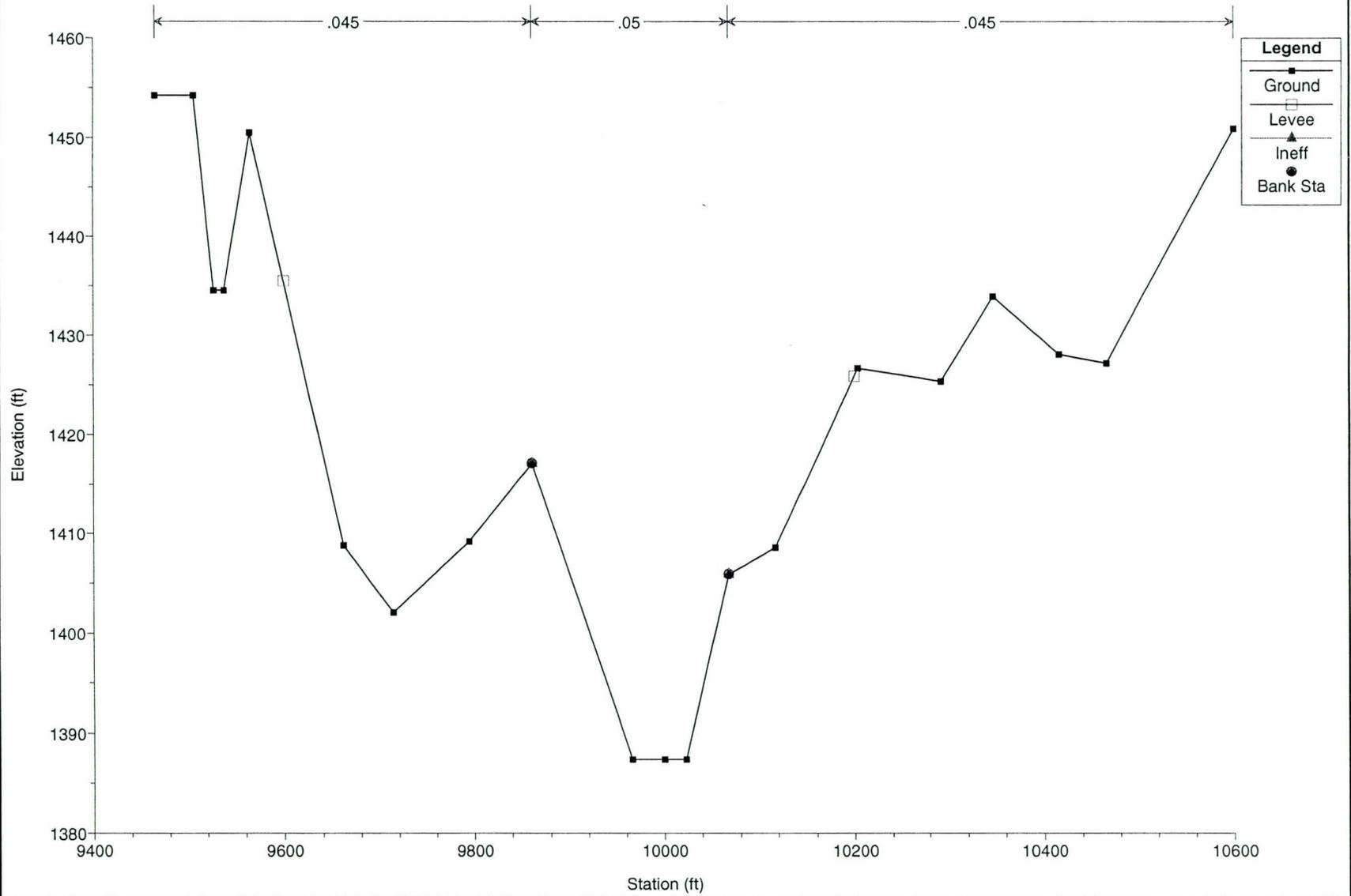
Figure 21. Average Bed Elevation RM 30.0 to RM 33.0.

APPENDIX A

Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

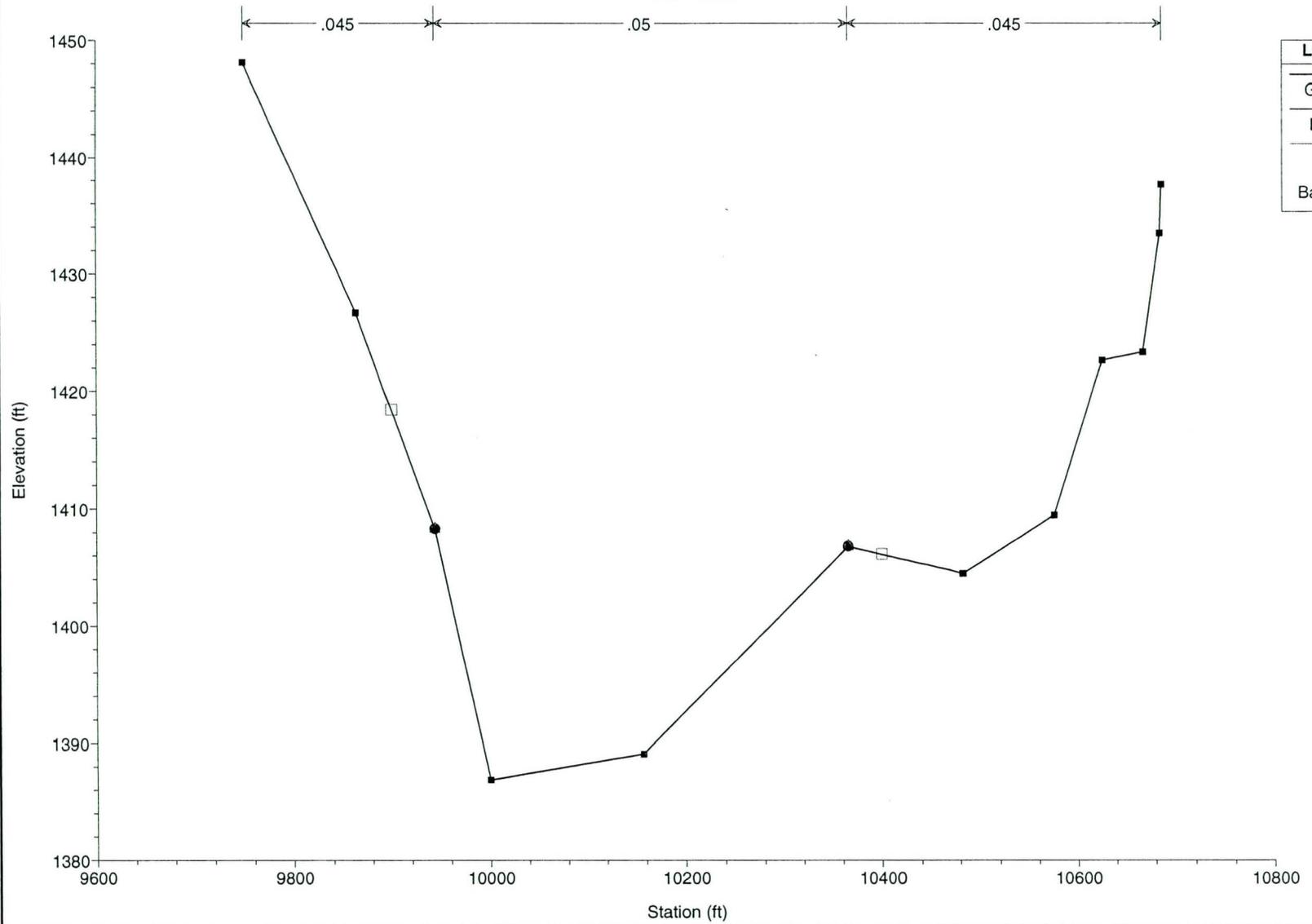
RS = 33.82



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

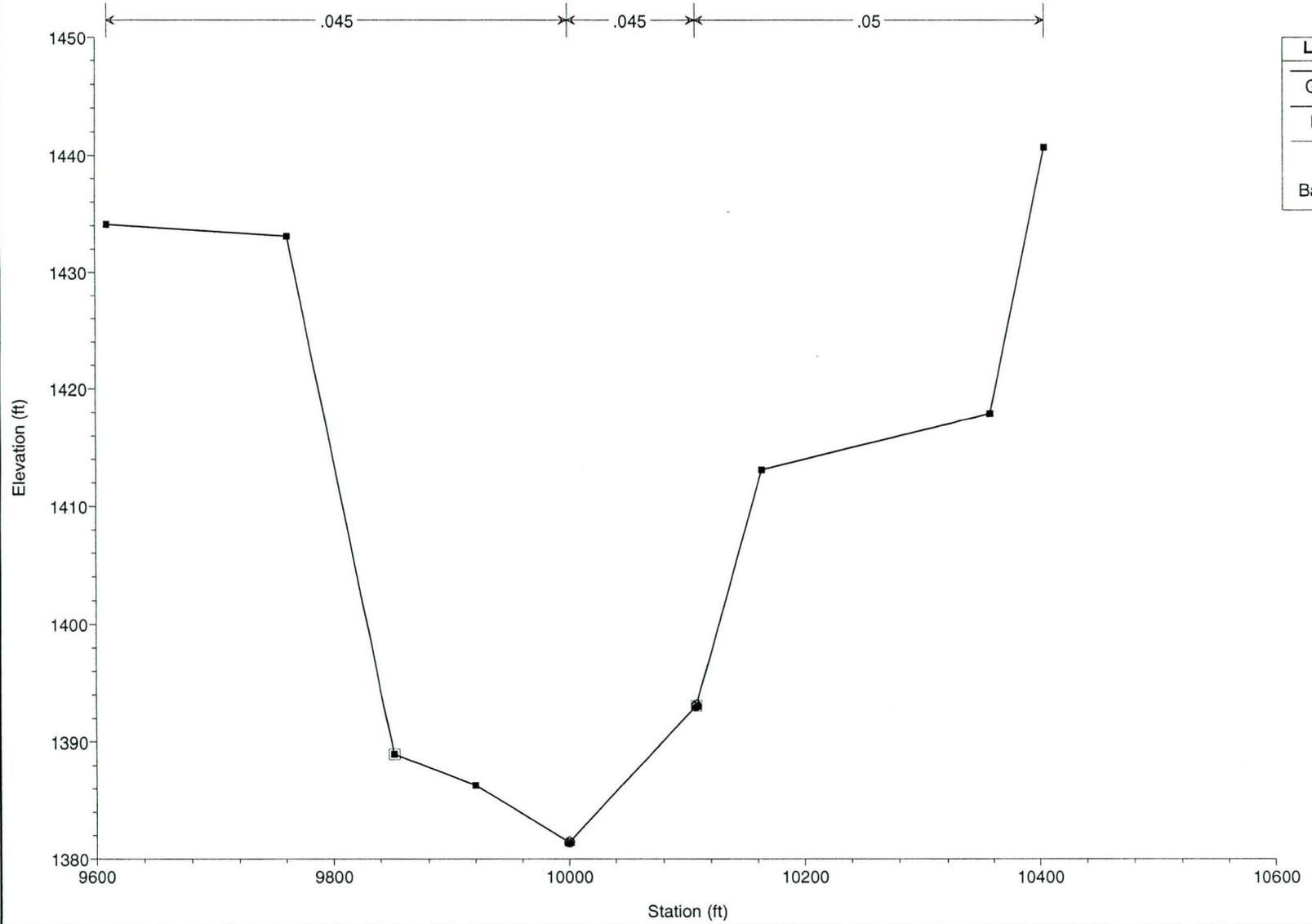
RS = 33.41



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

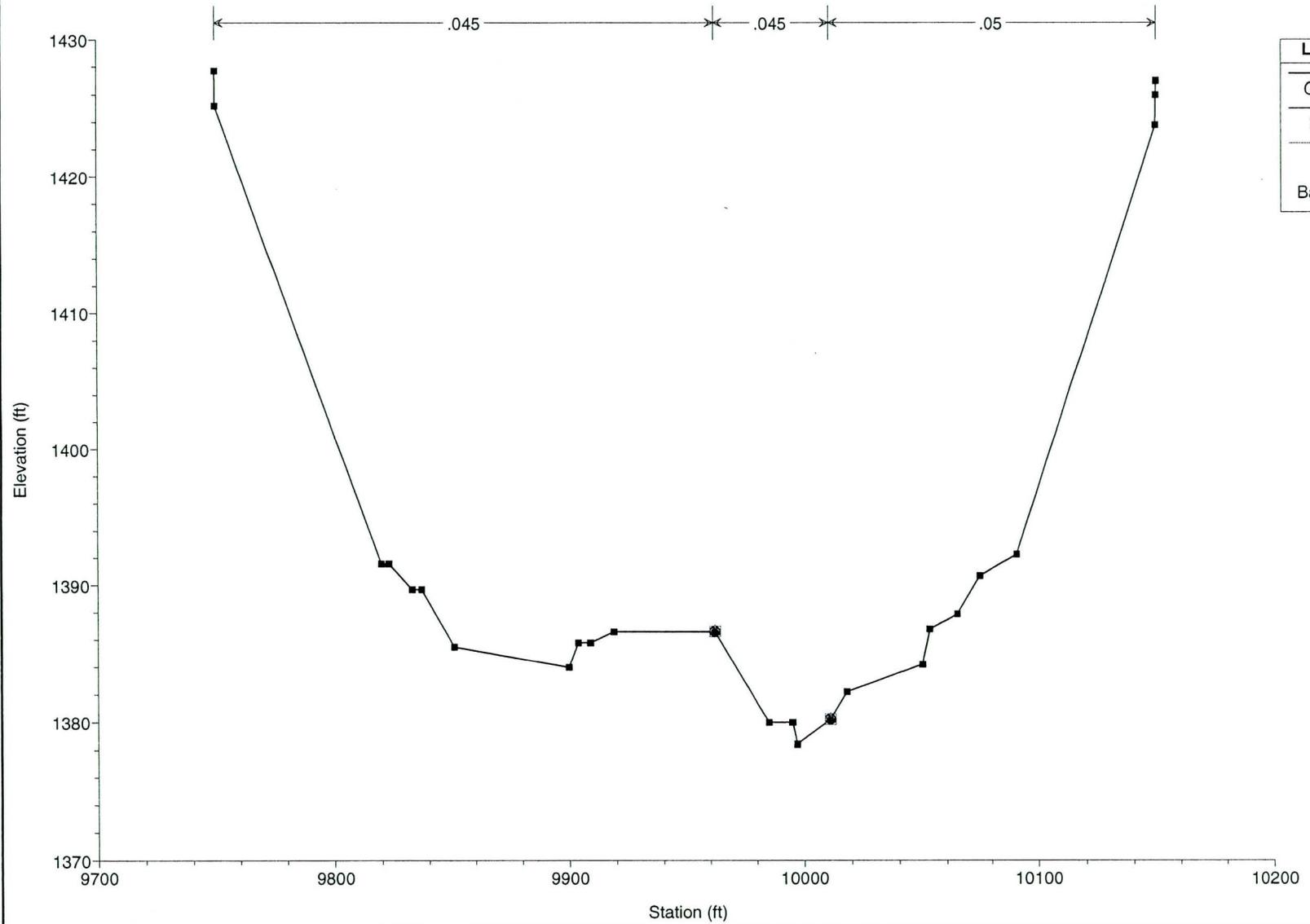
RS = 32.998



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

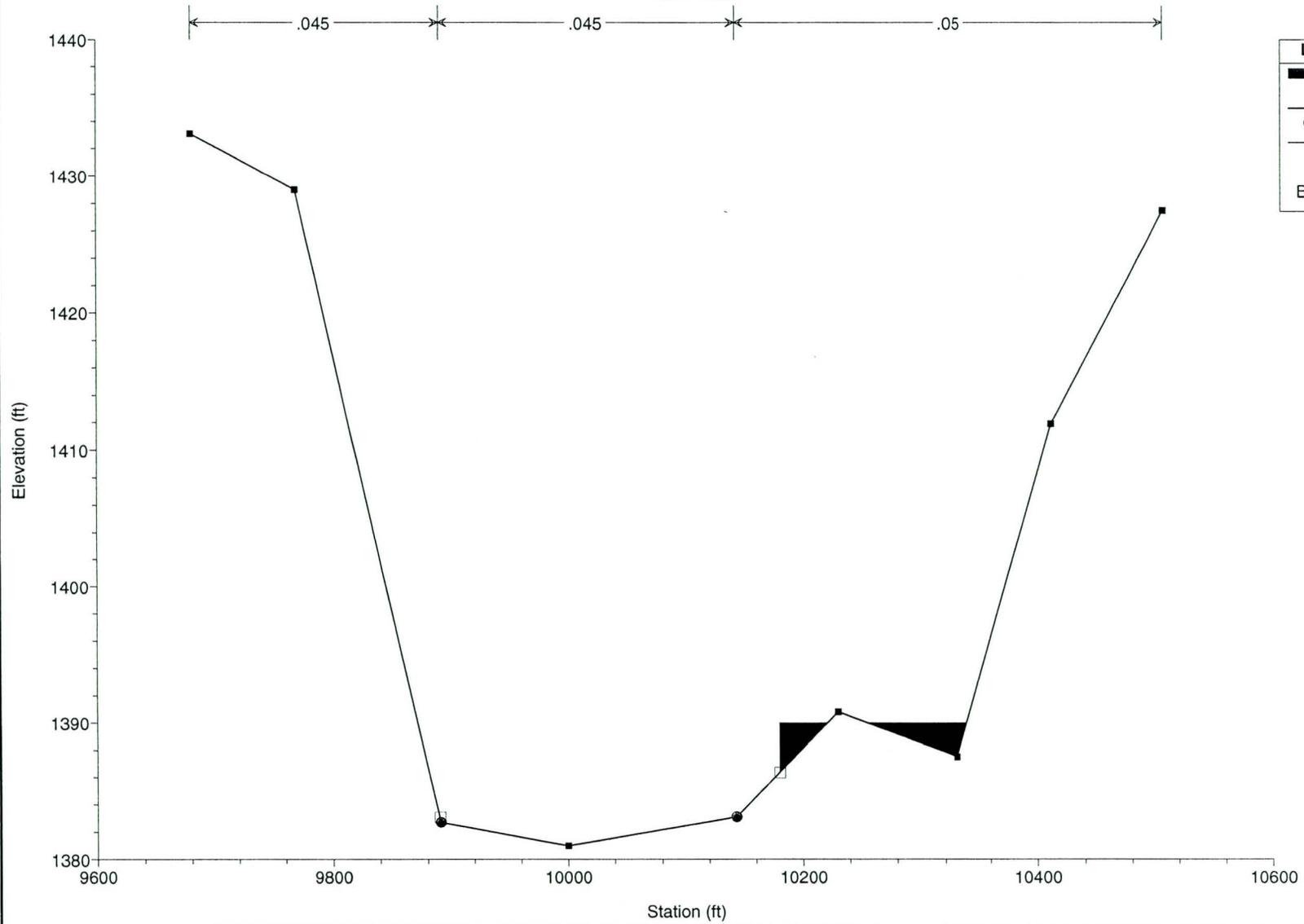
RS = 32.984



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 32.92



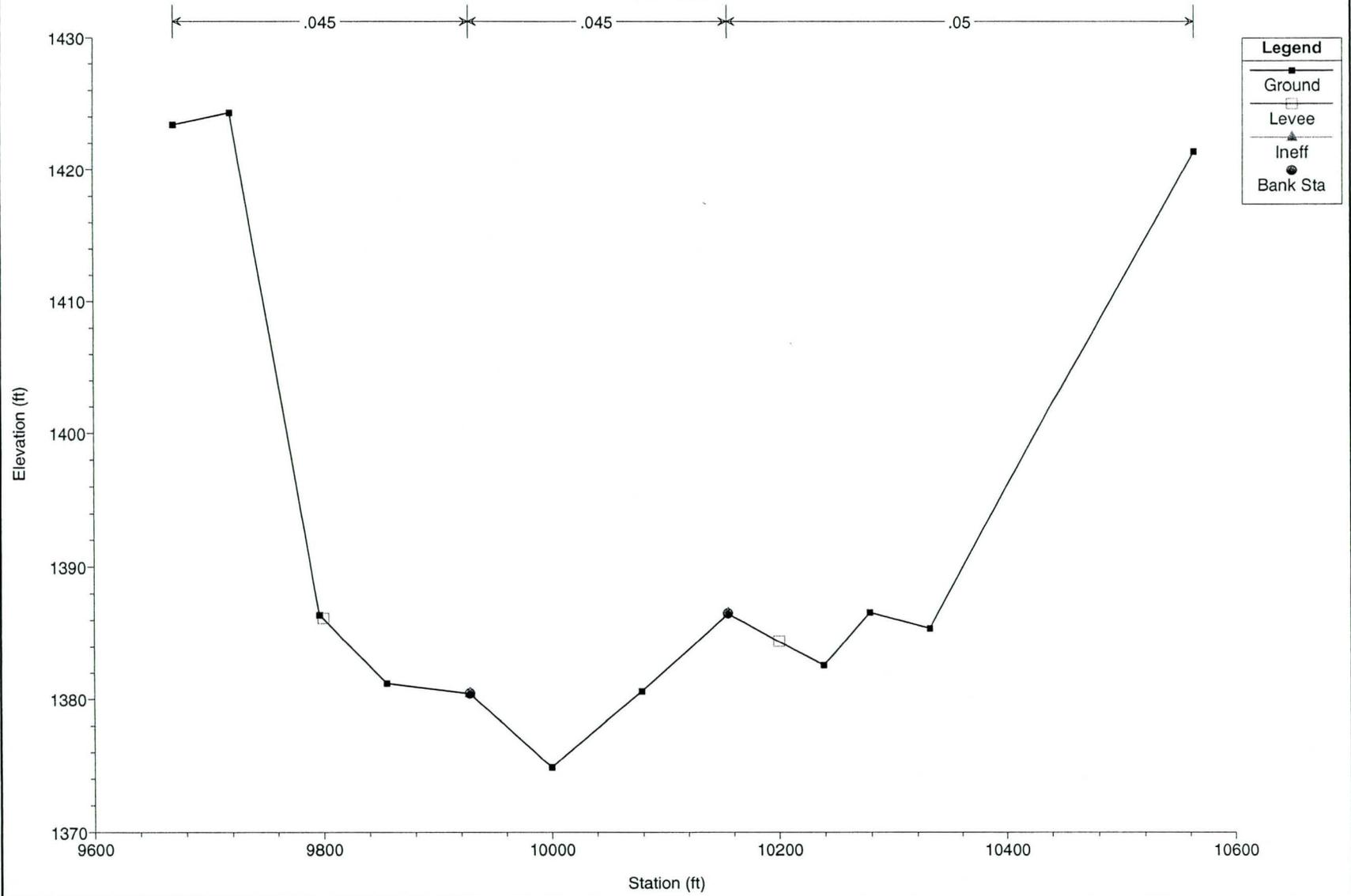
Legend

- Encr (shaded area)
- Ground (solid line with square)
- Levee (dashed line with square)
- Bank Sta (solid circle)

Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

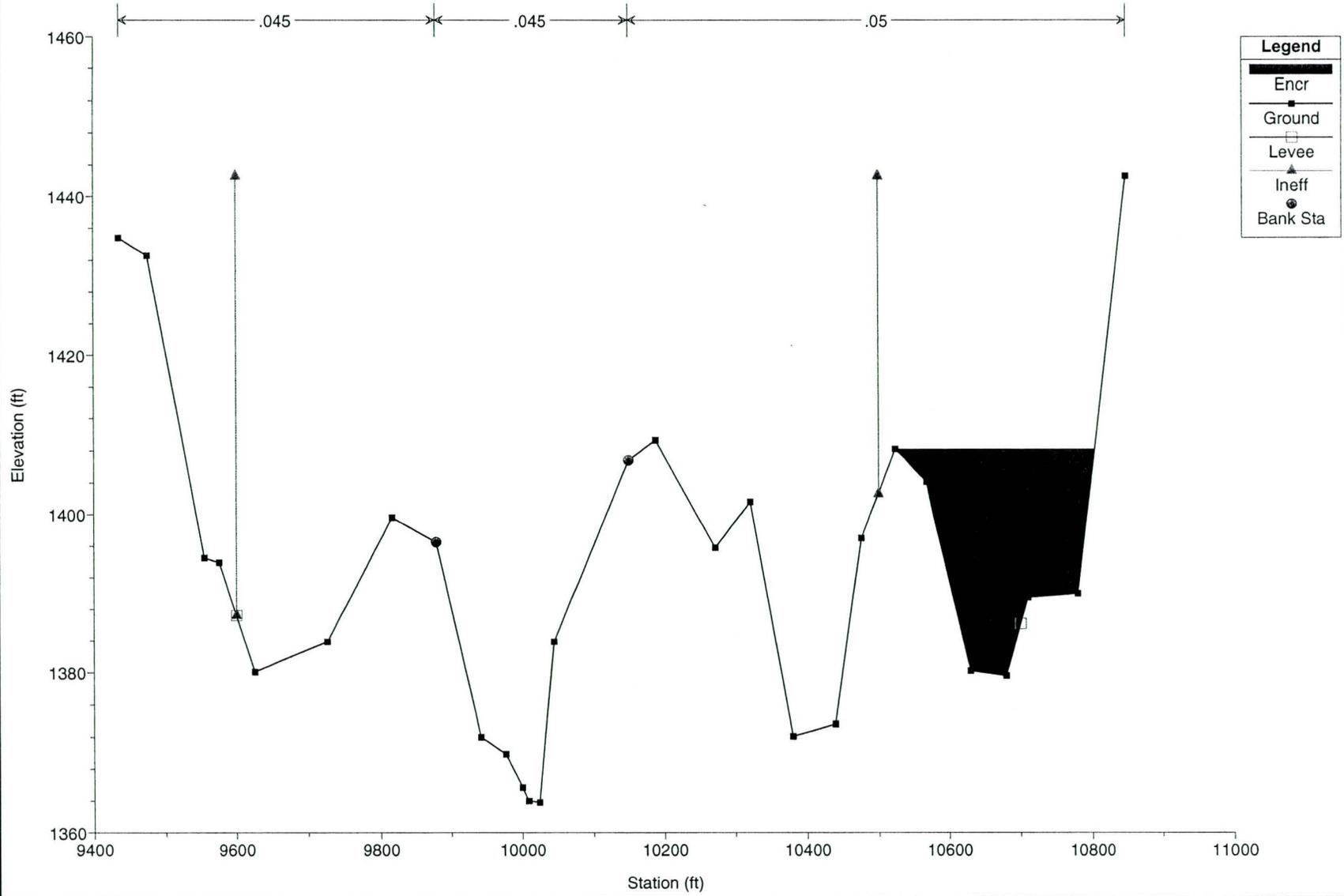
RS = 32.86



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

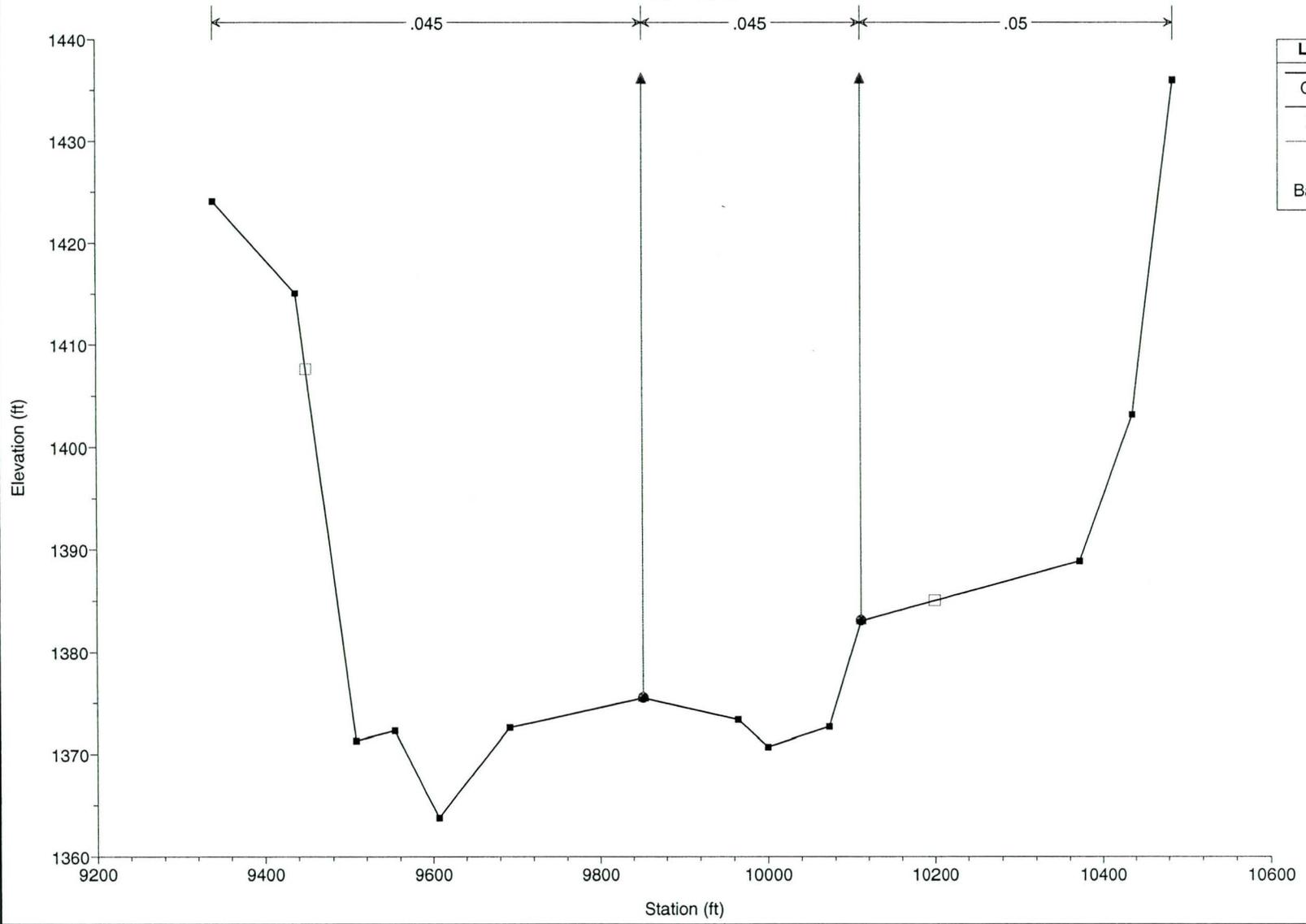
RS = 32.72



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 32.43



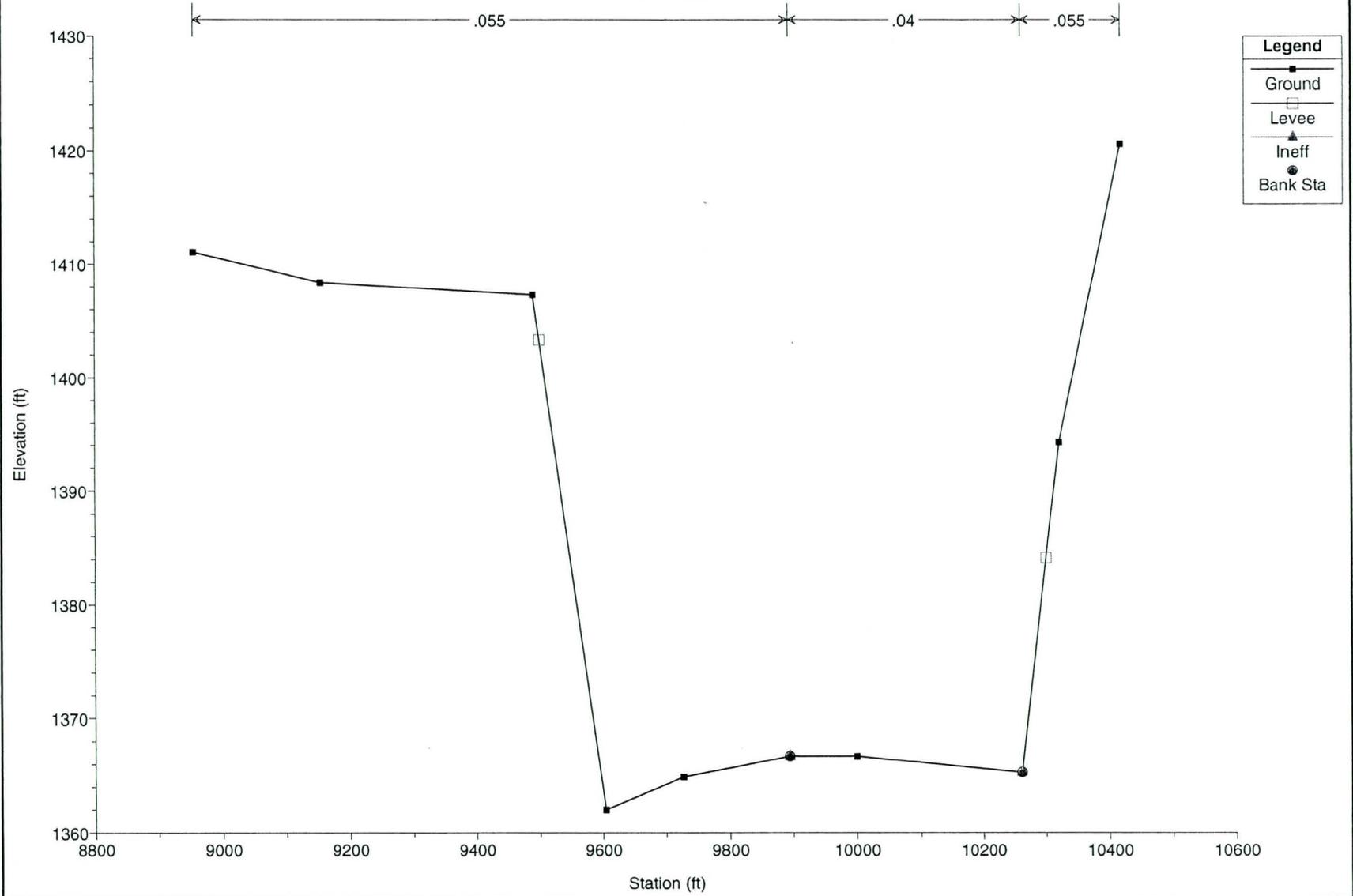
Legend

- Ground
- Levee
- Ineff
- Bank Sta

Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

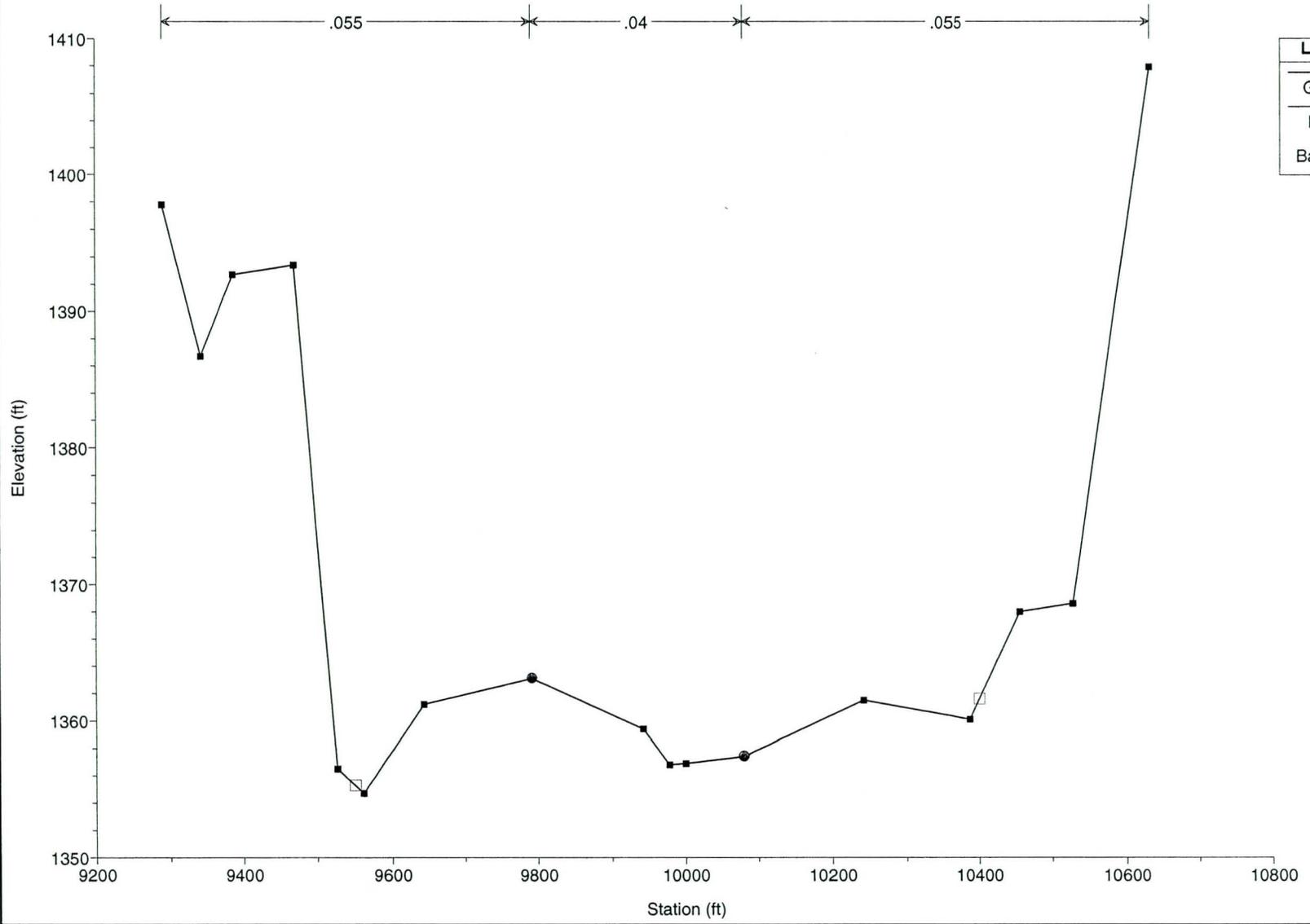
RS = 31.86



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

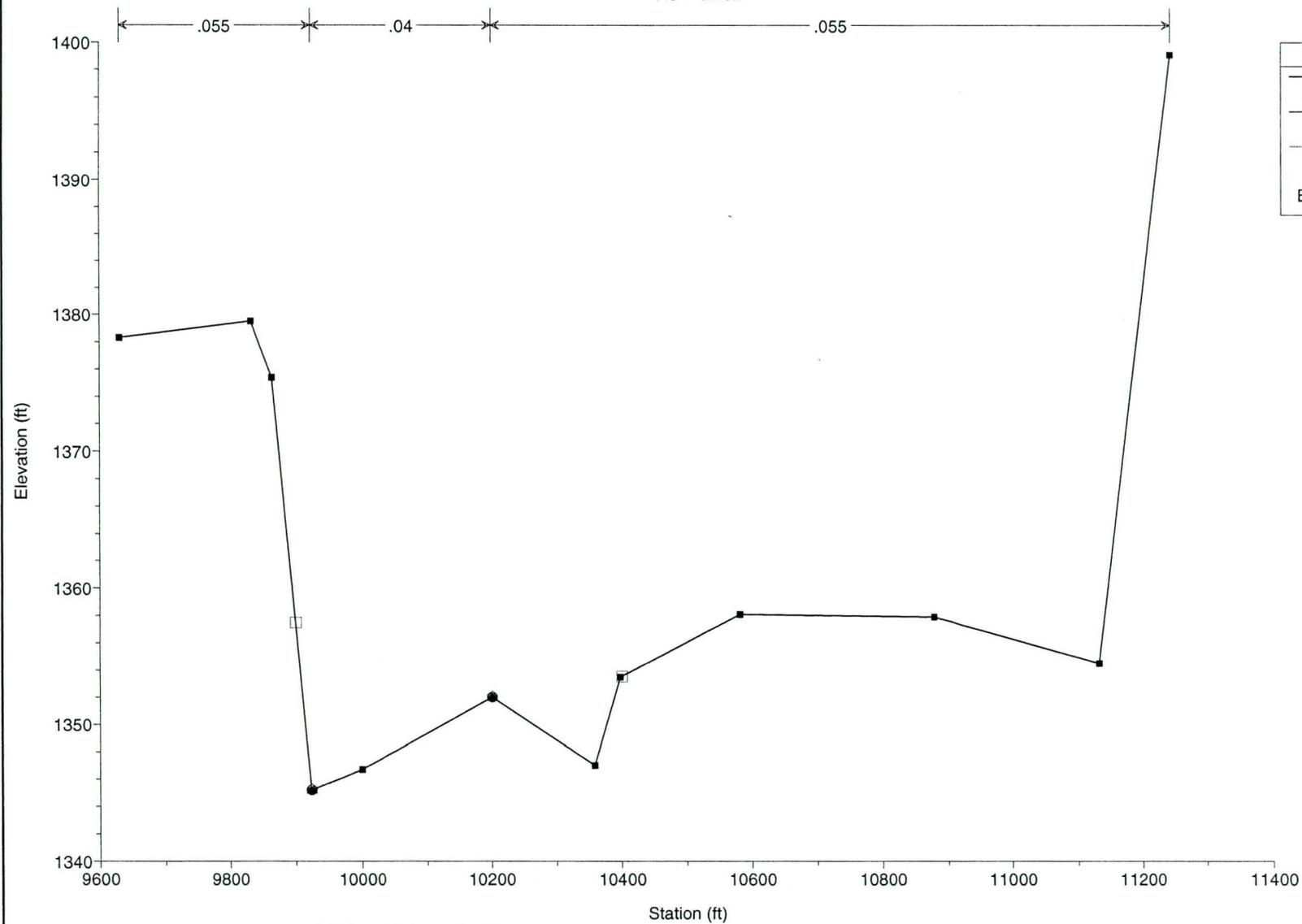
RS = 31.39



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

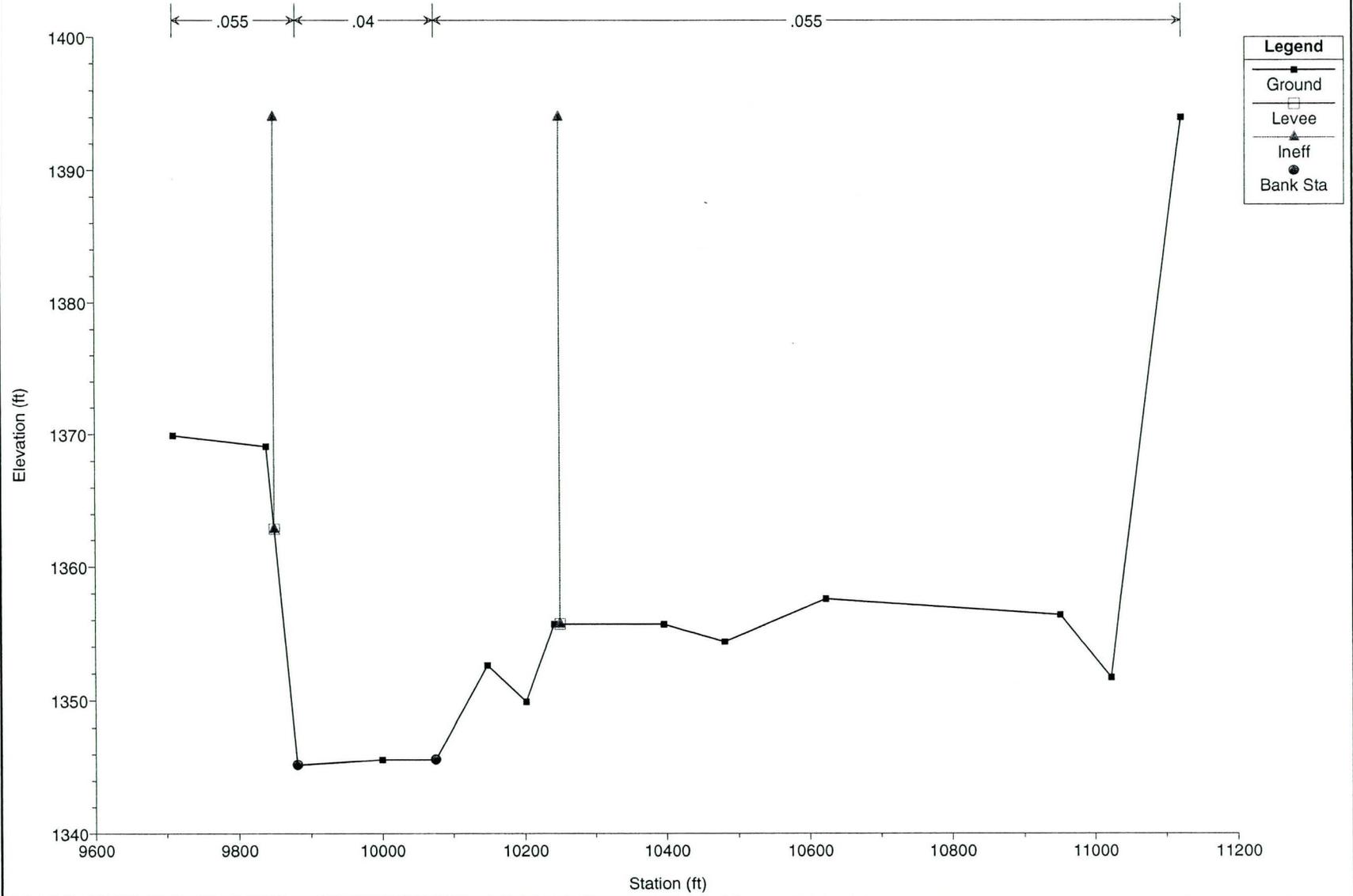
RS = 30.82



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

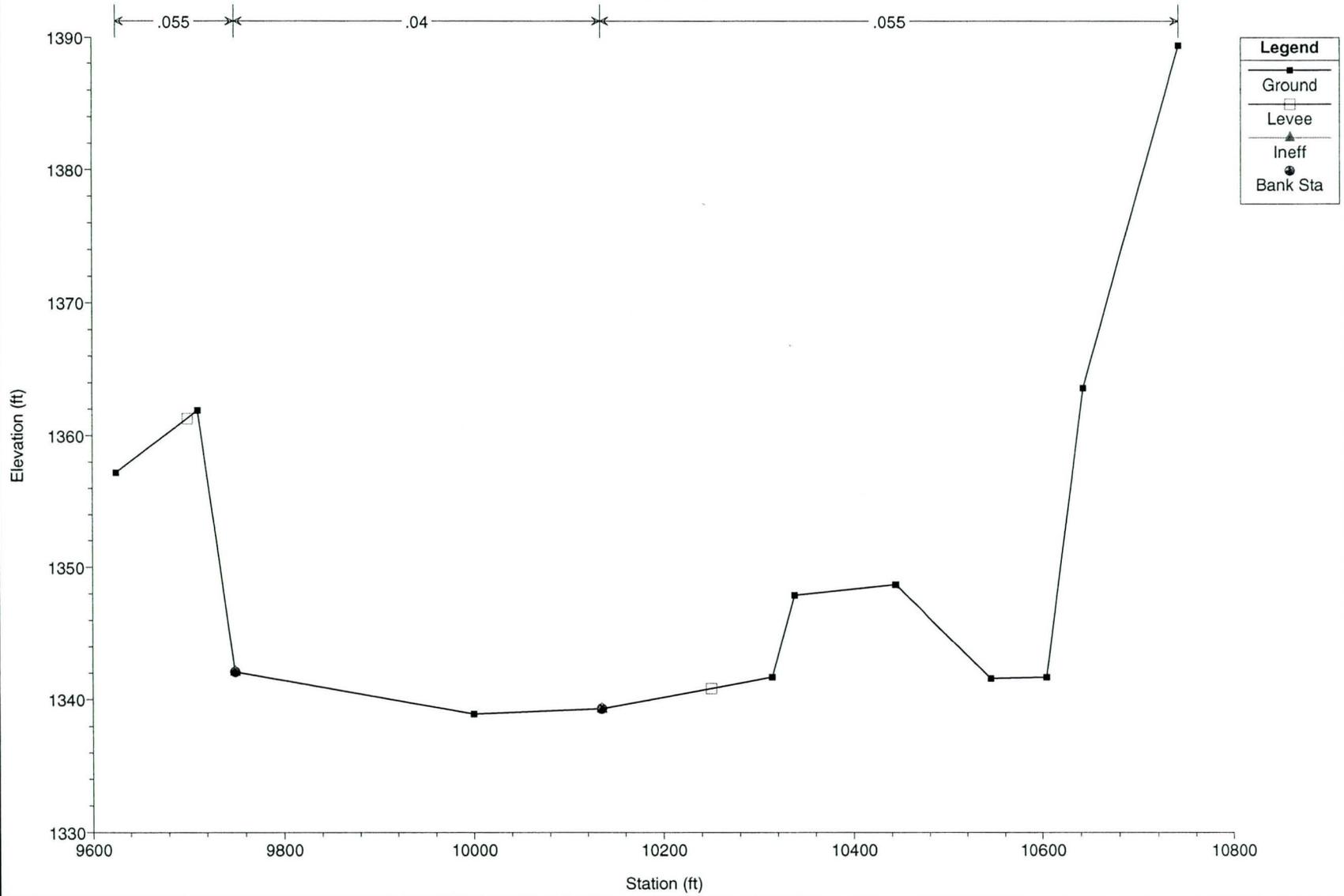
RS = 30.73



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

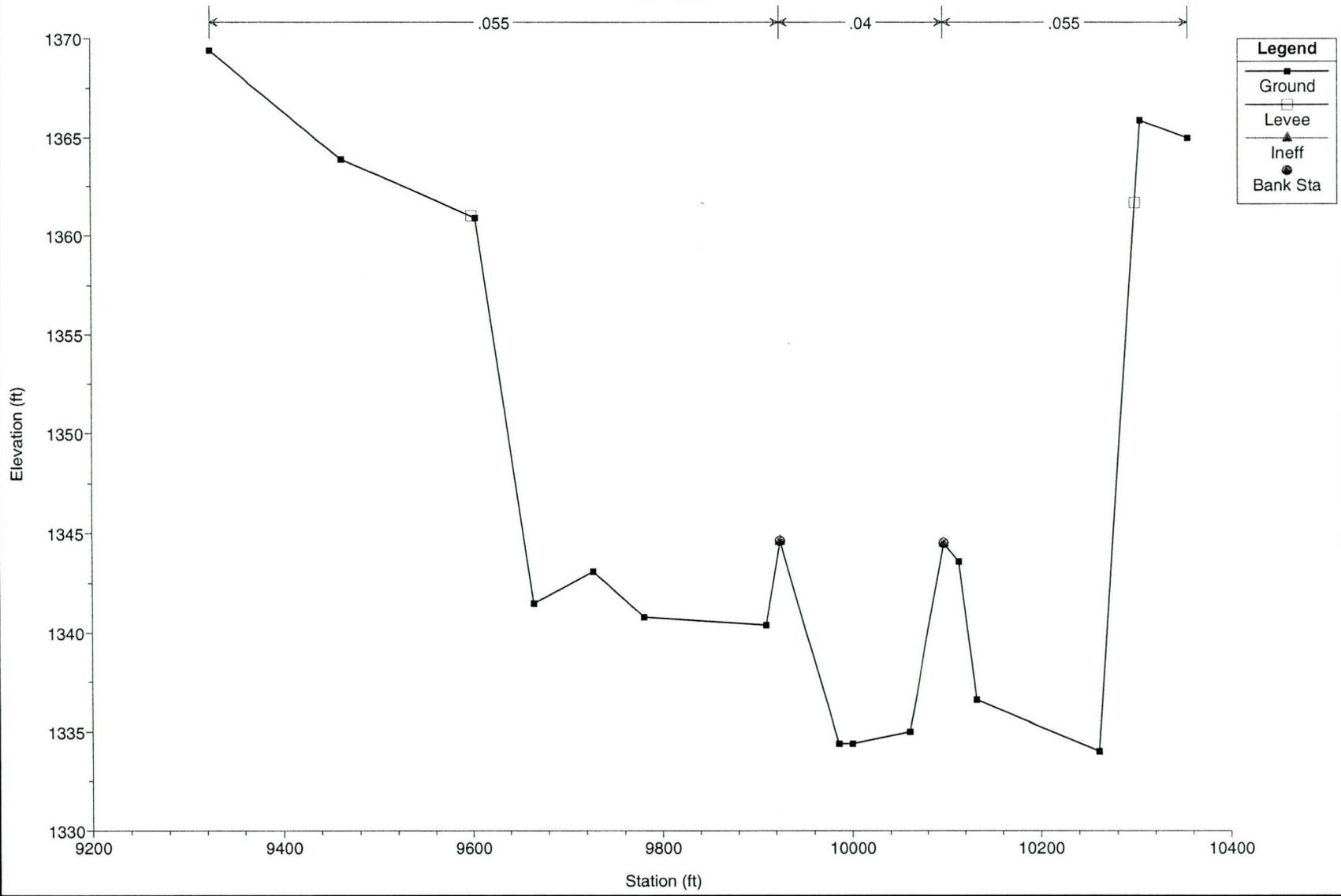
RS = 30.26



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

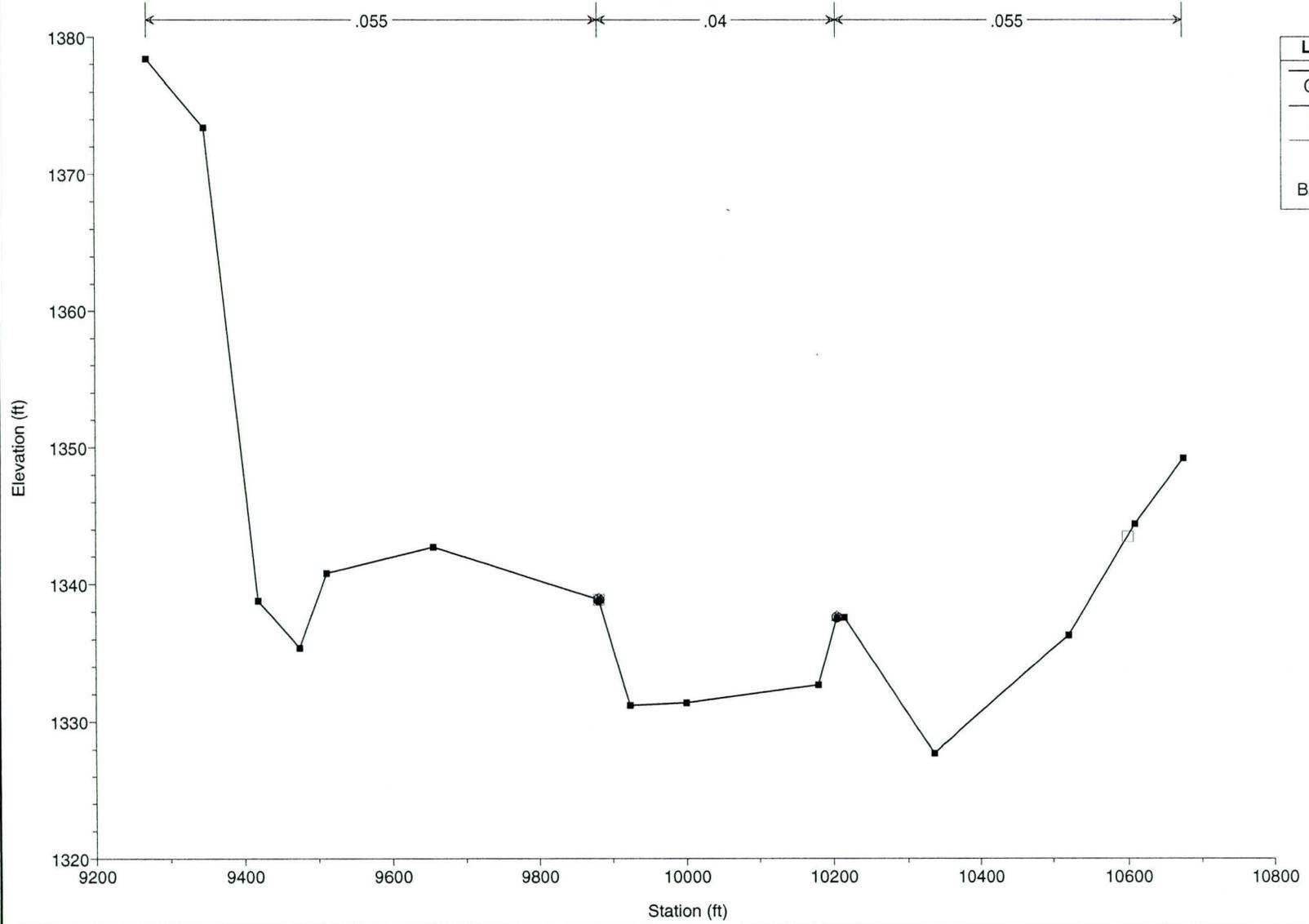
RS = 30.07



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

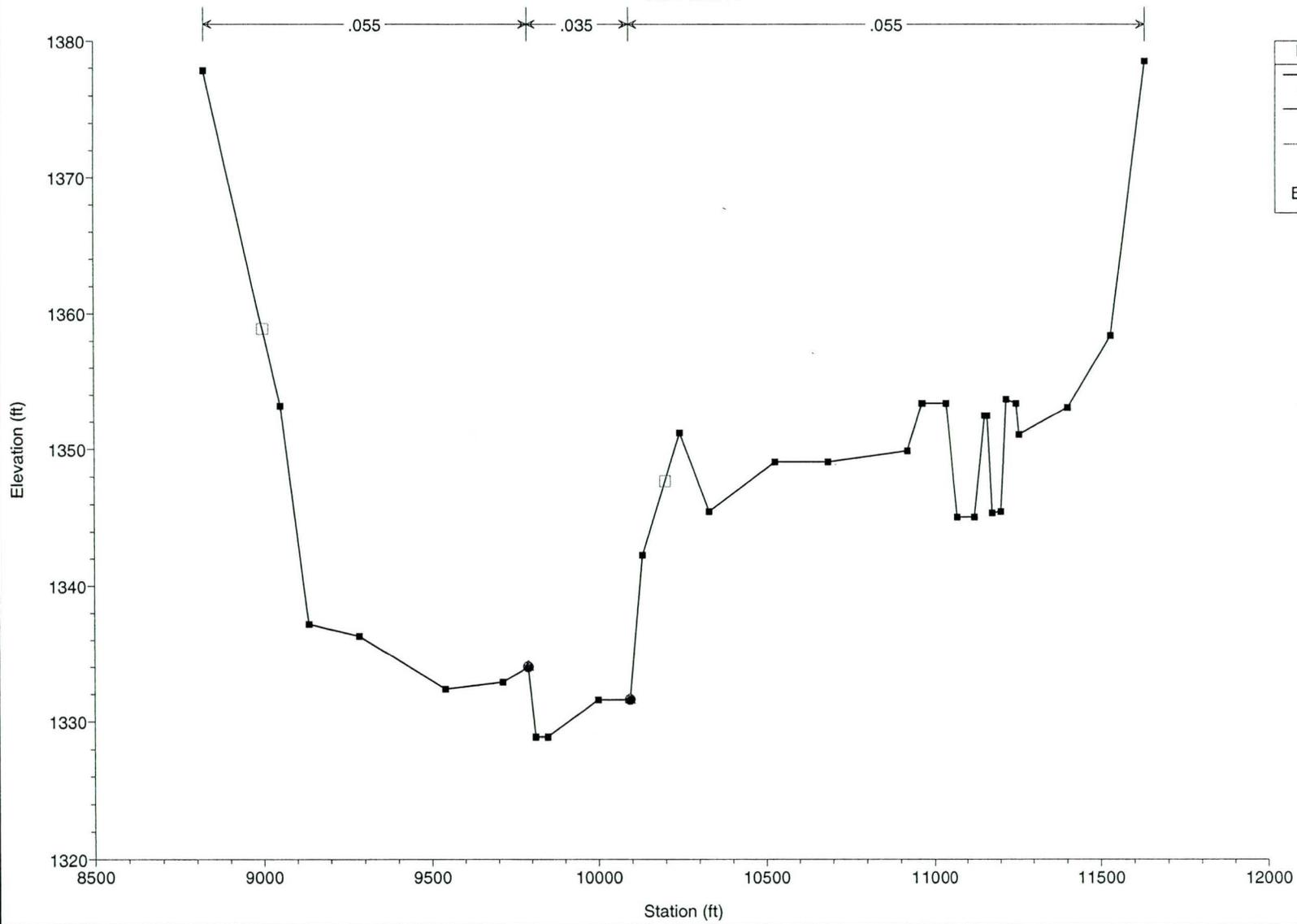
RS = 29.8



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 29.611

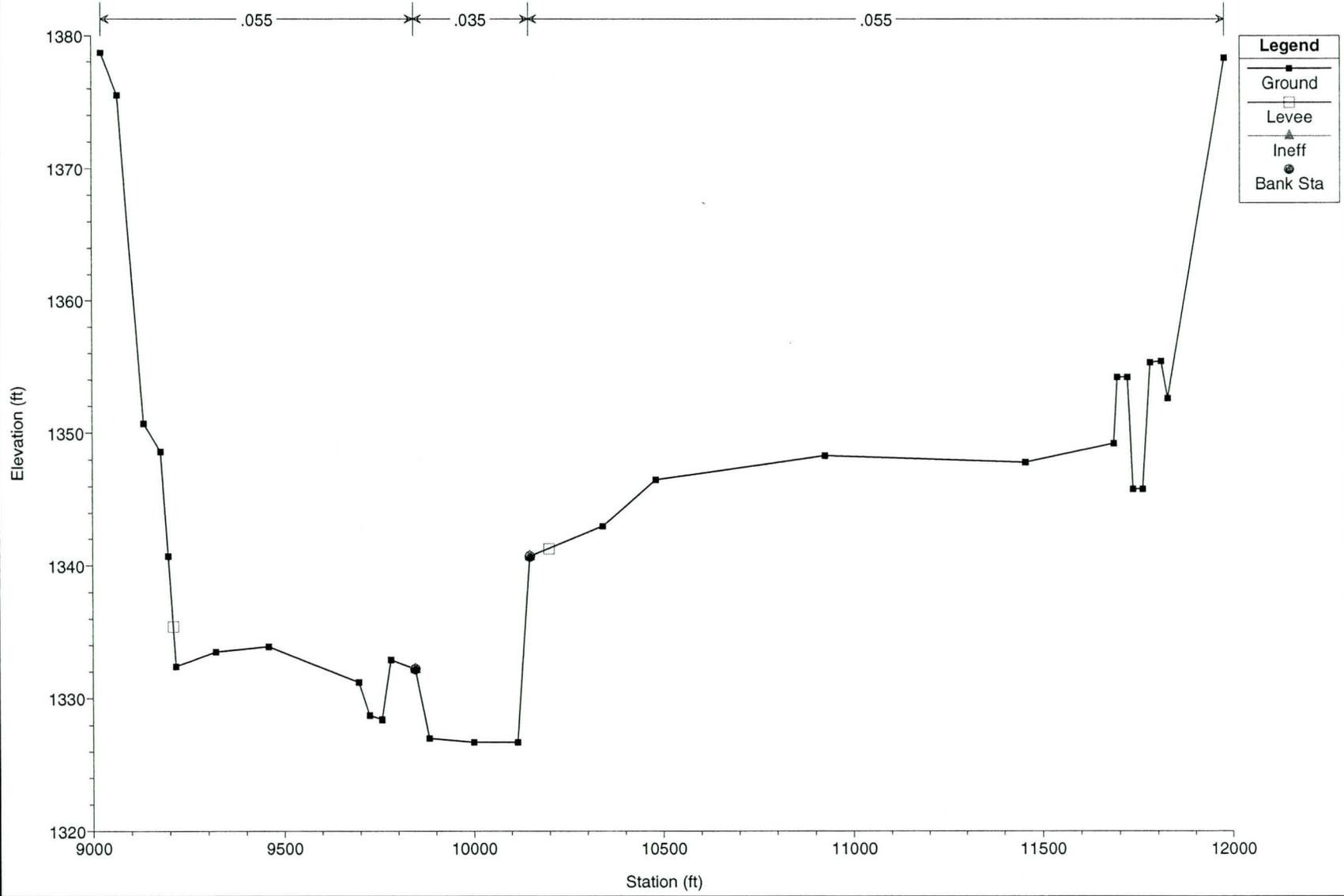


Legend	
—■—	Ground
—□—	Levee
—▲—	Ineff
—●—	Bank Sta

Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

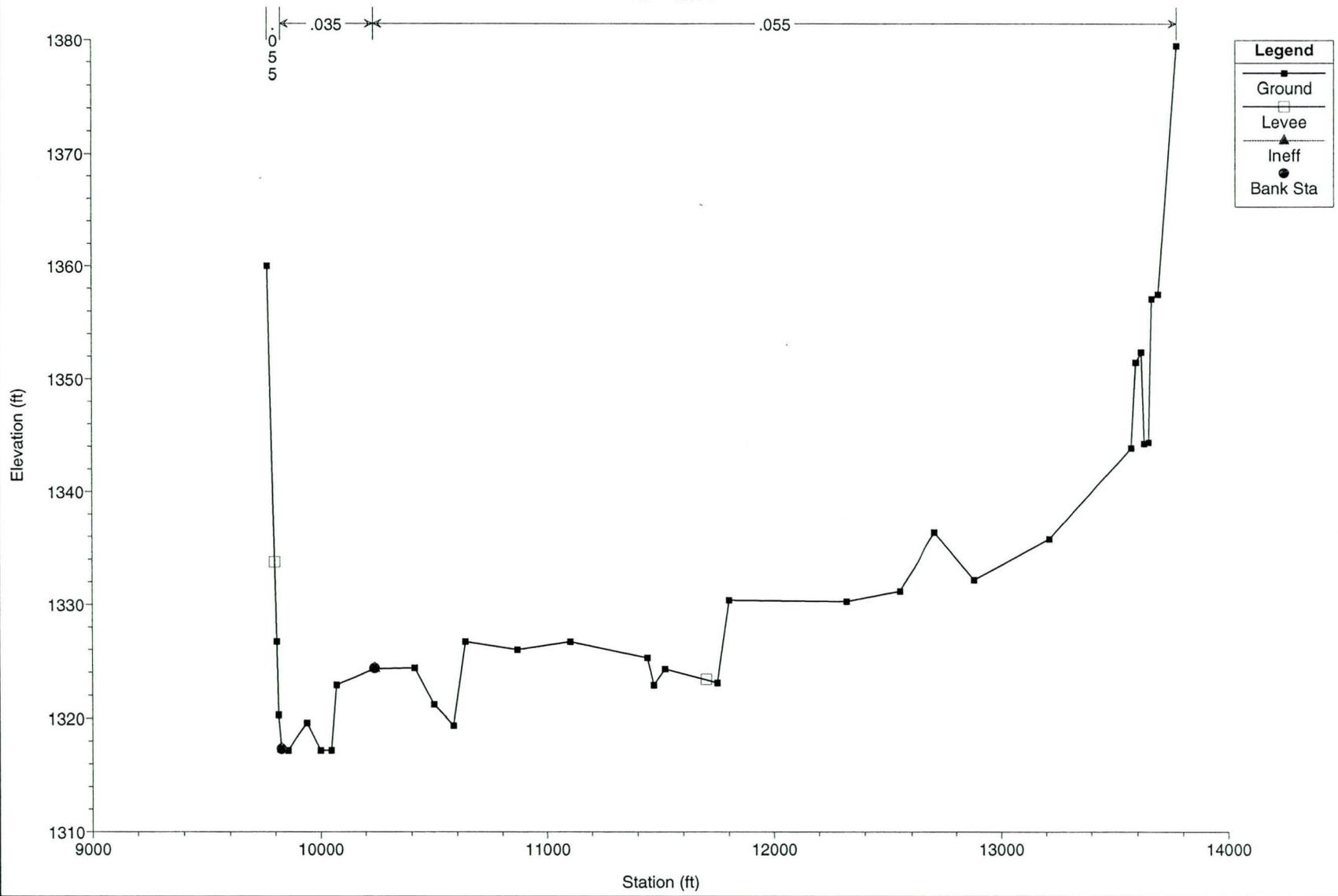
RS = 29.54



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

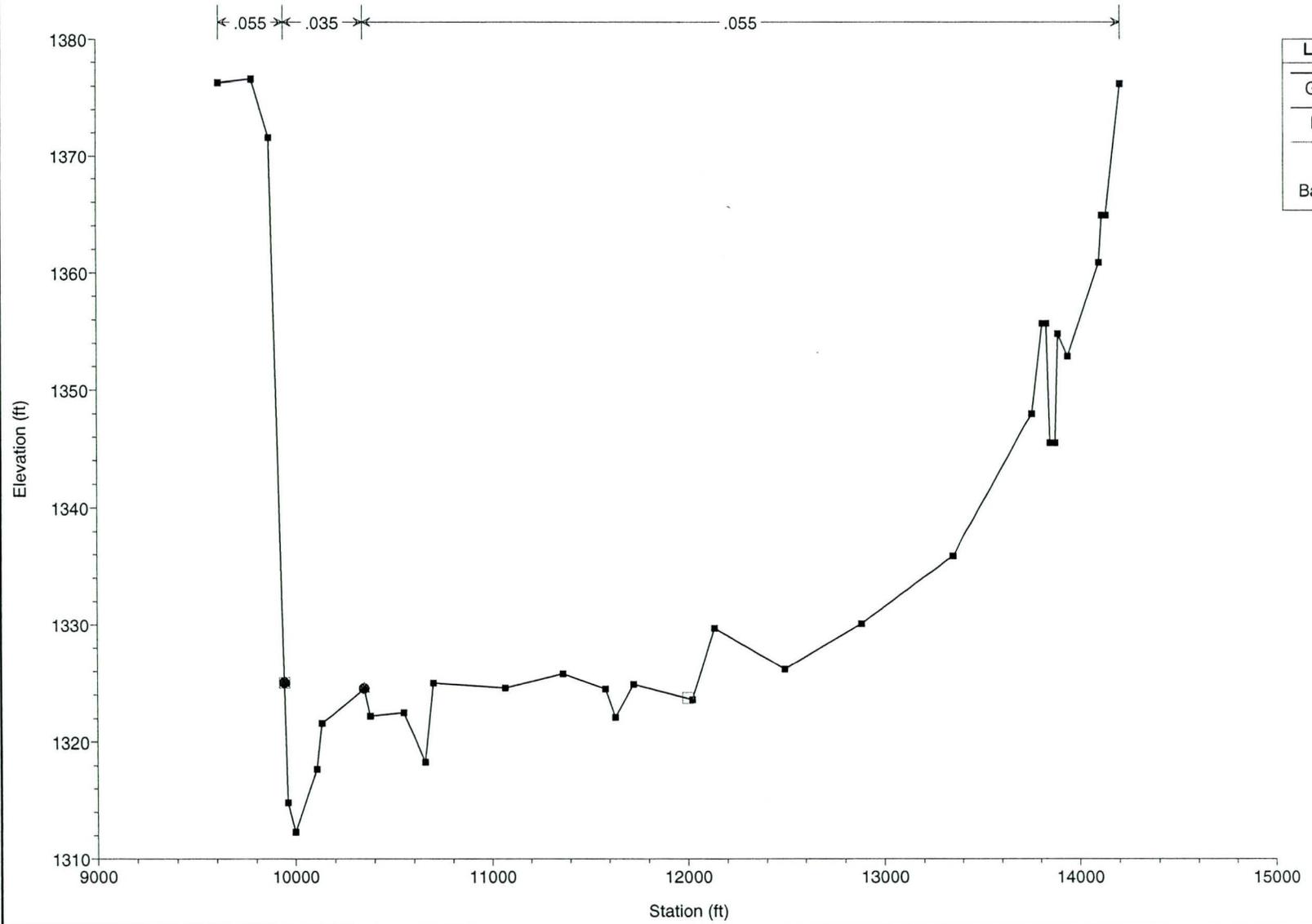
RS = 29.04



Agua Fria HEC-6 Review

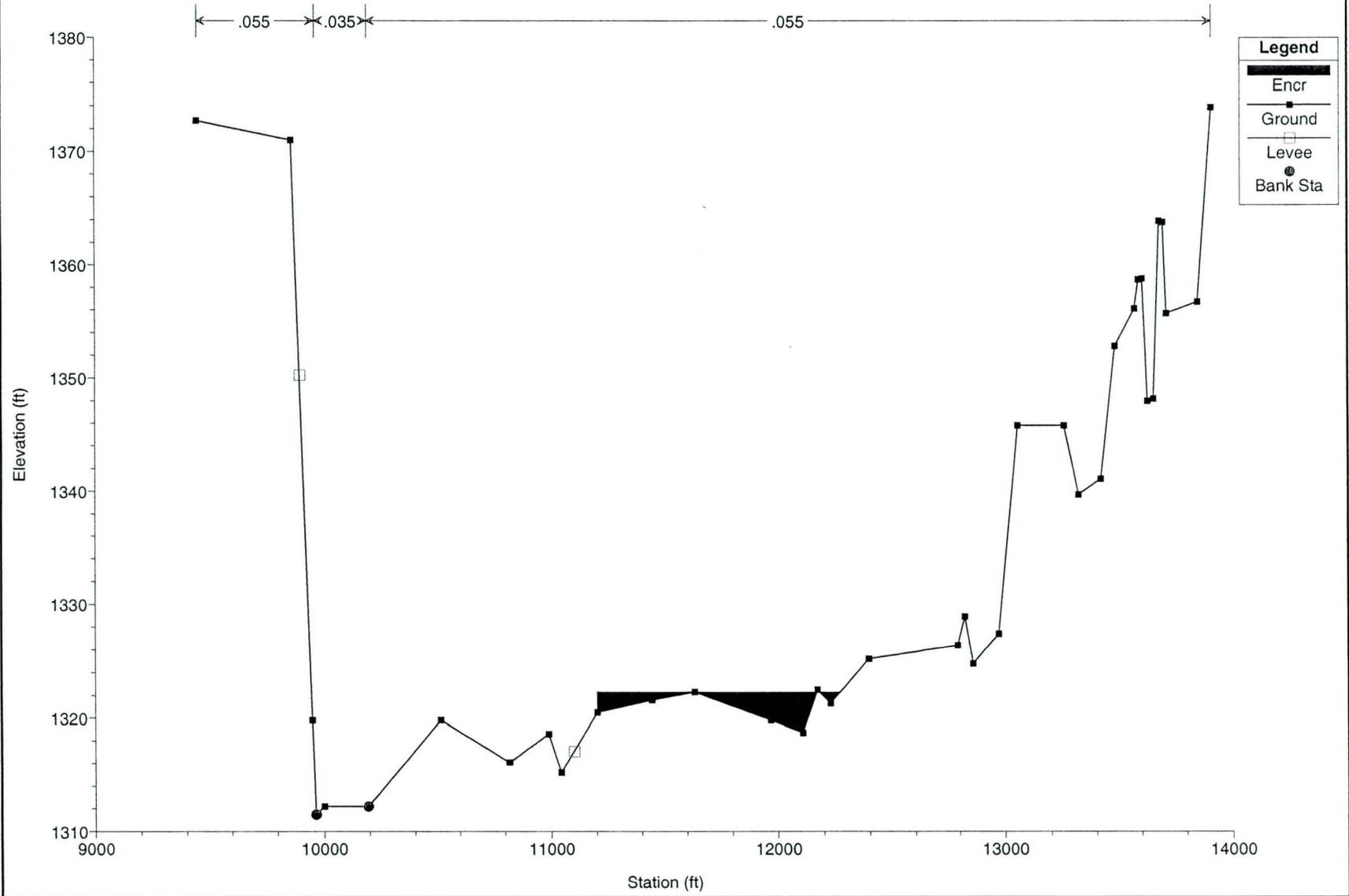
Geom: Existing conditions with movable bed limits as levees

RS = 28.95



Agua Fria HEC-6 Review

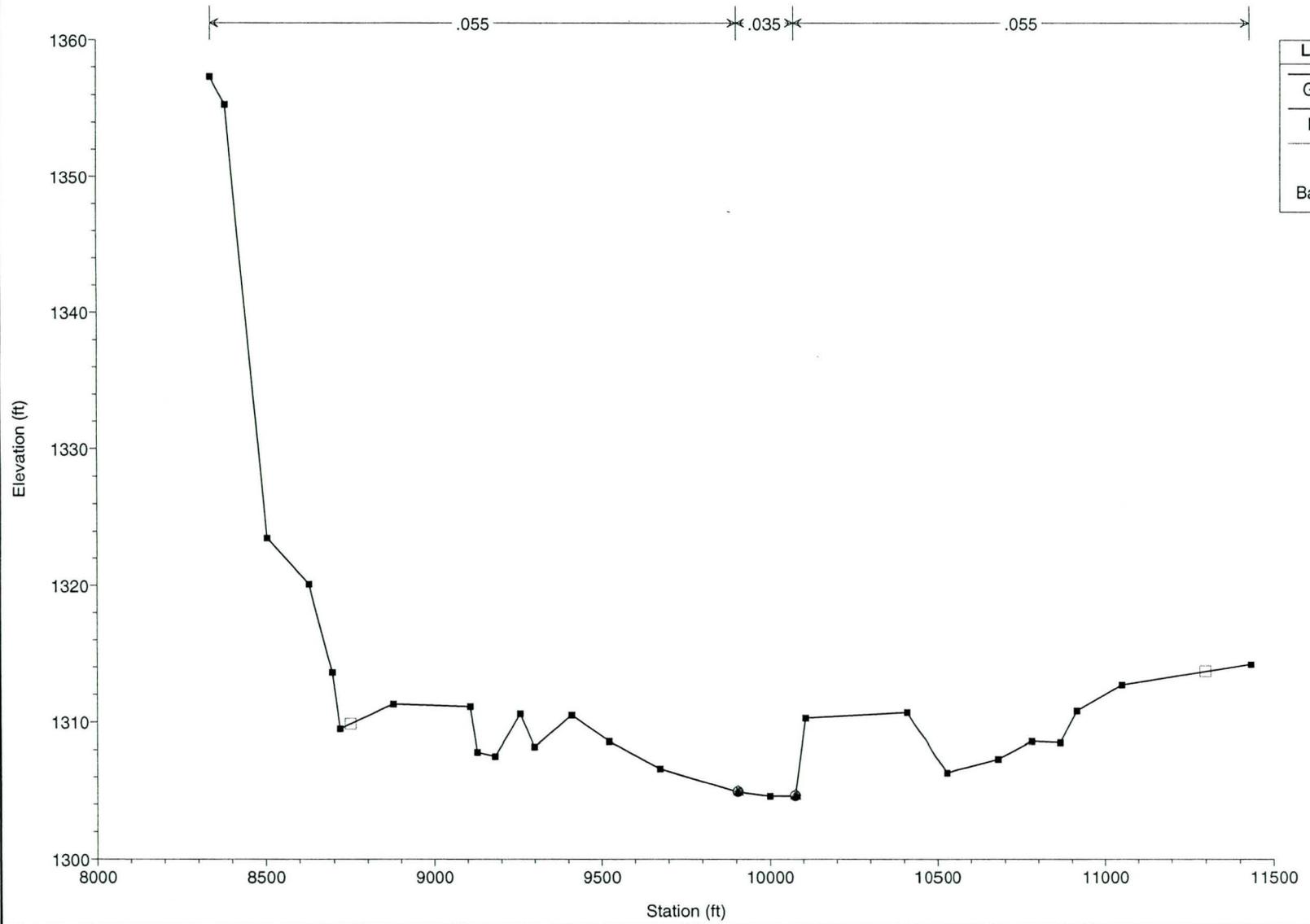
Geom: Existing conditions with movable bed limits as levees
RS = 28.67



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

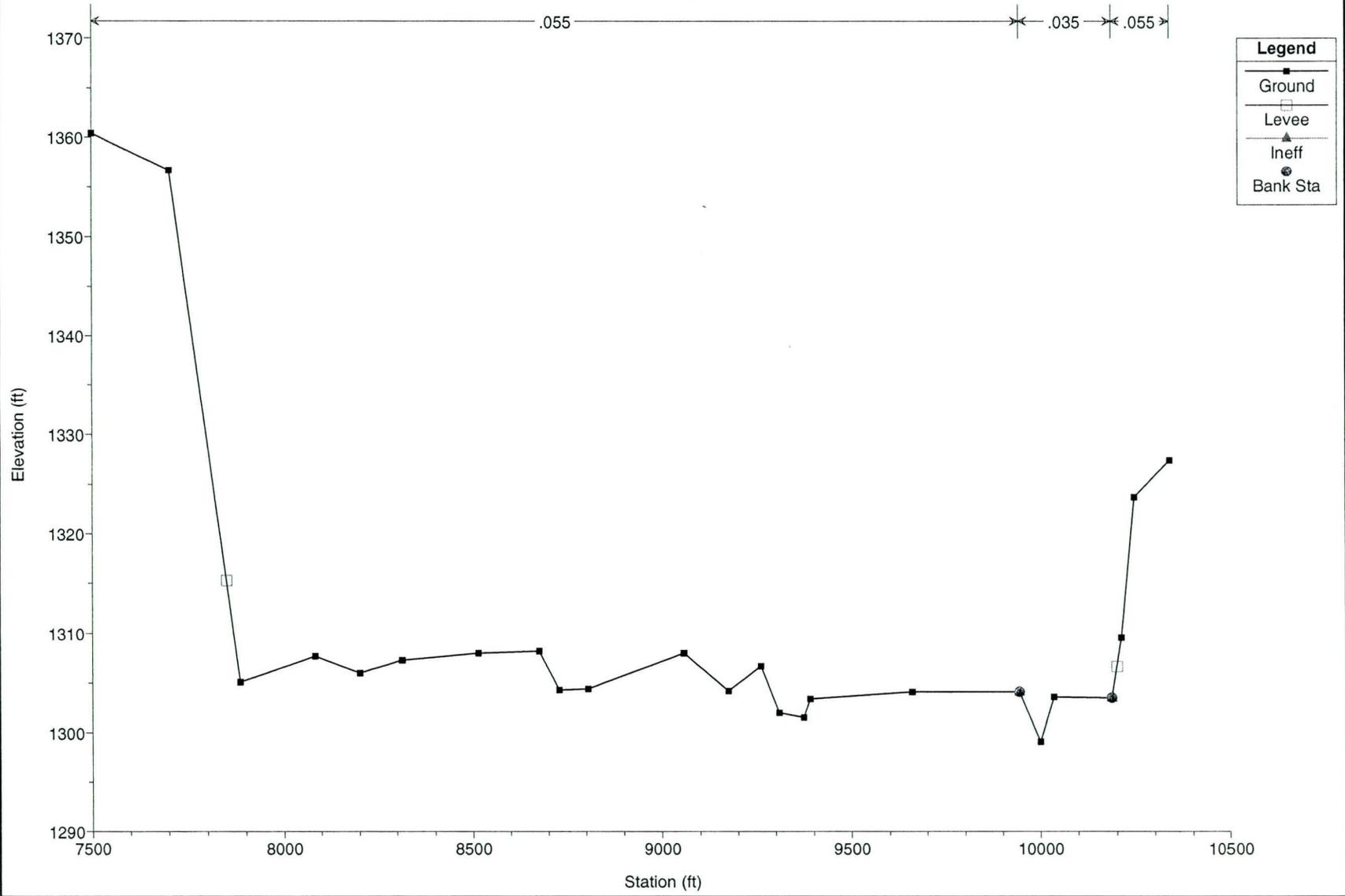
RS = 28.12



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

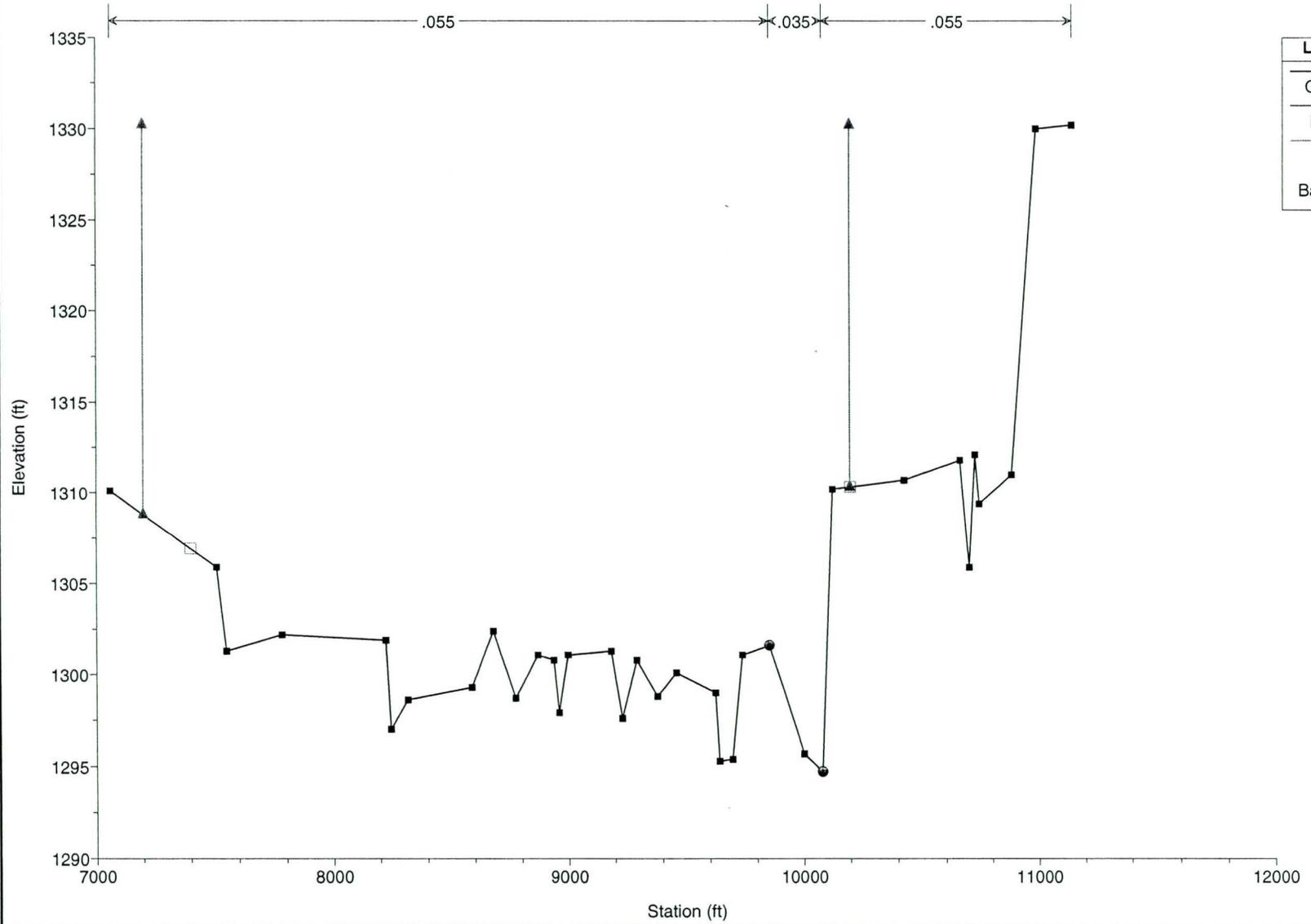
RS = 27.94



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

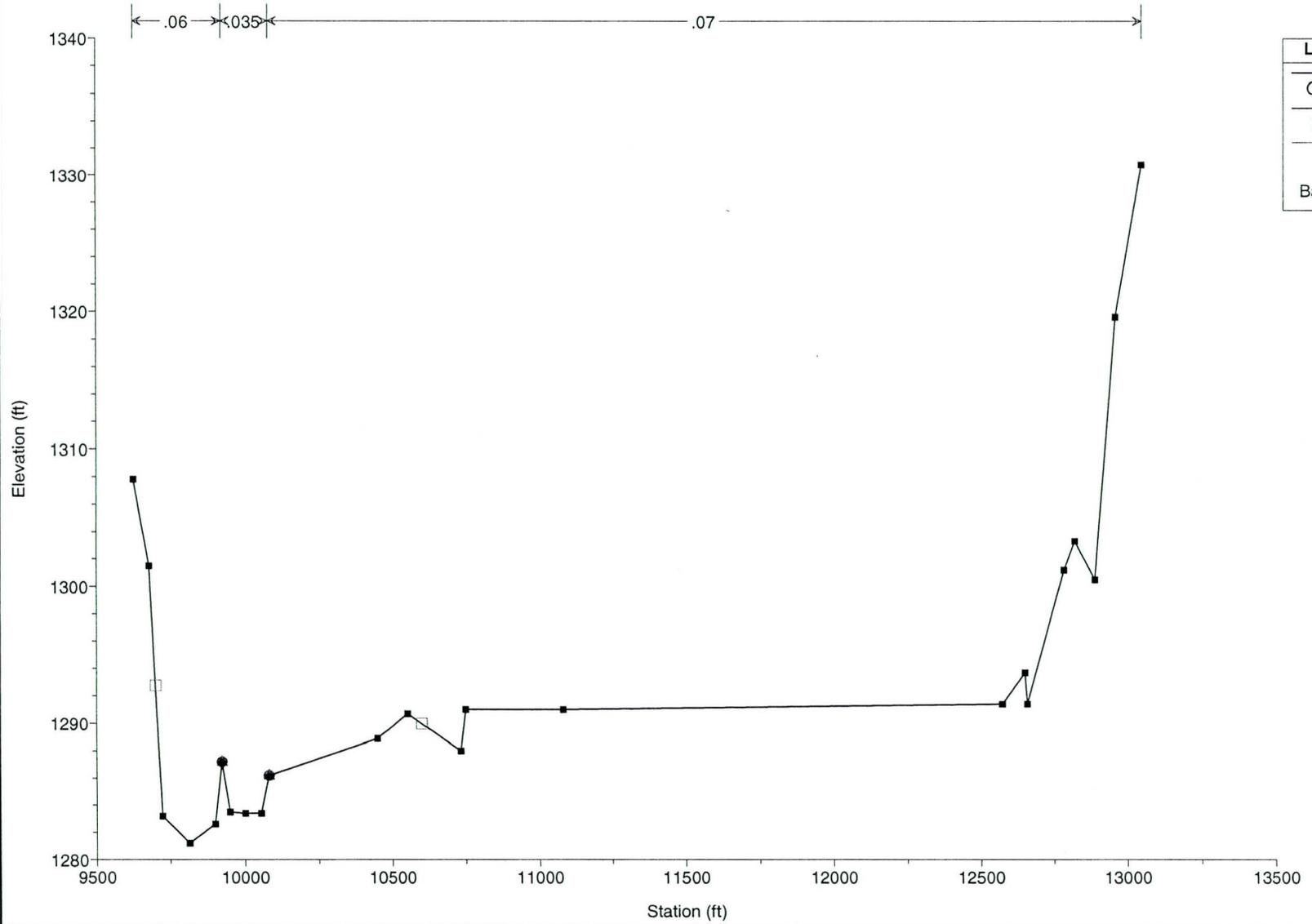
RS = 27.68



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

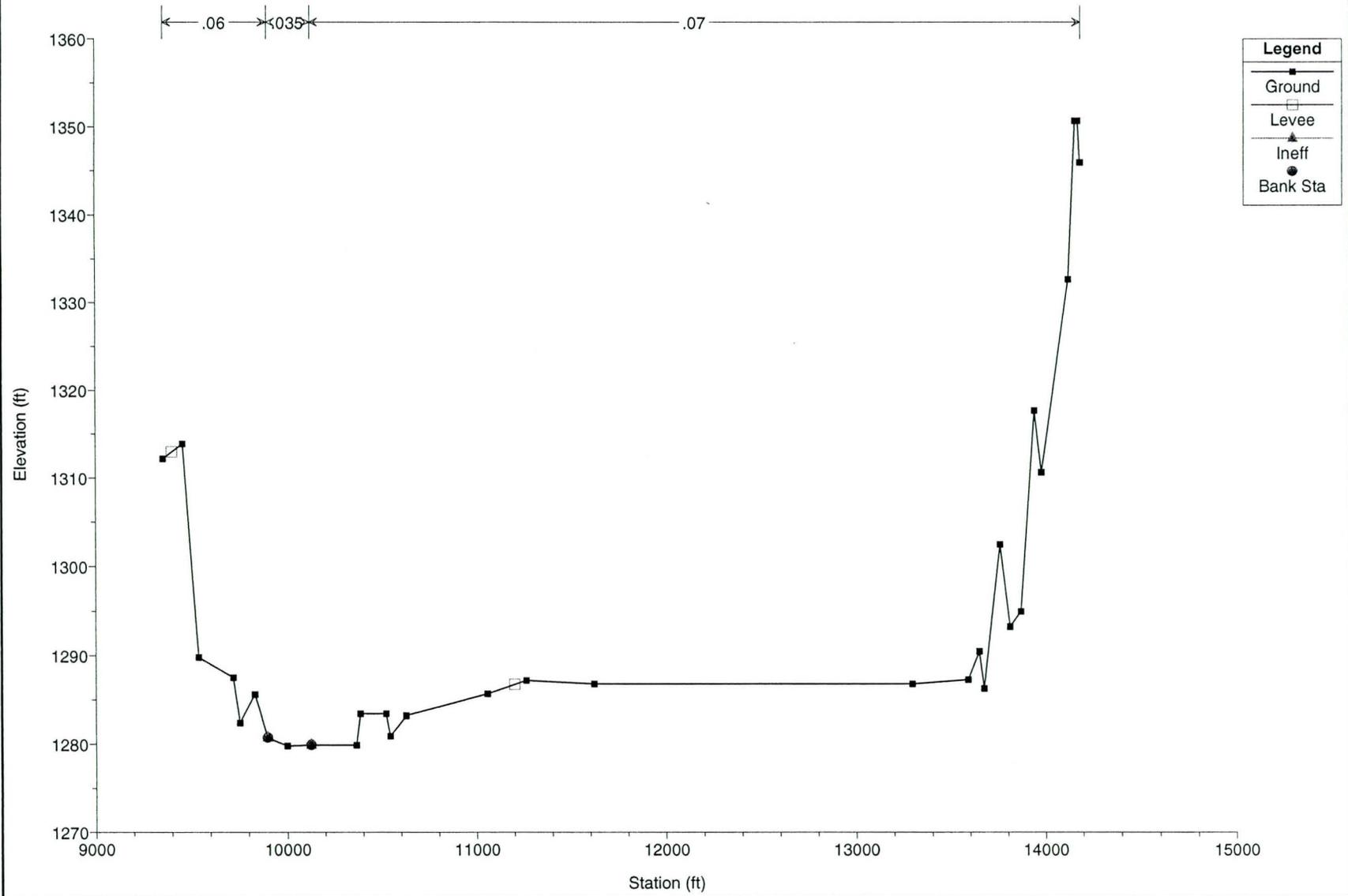
RS = 27.03



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

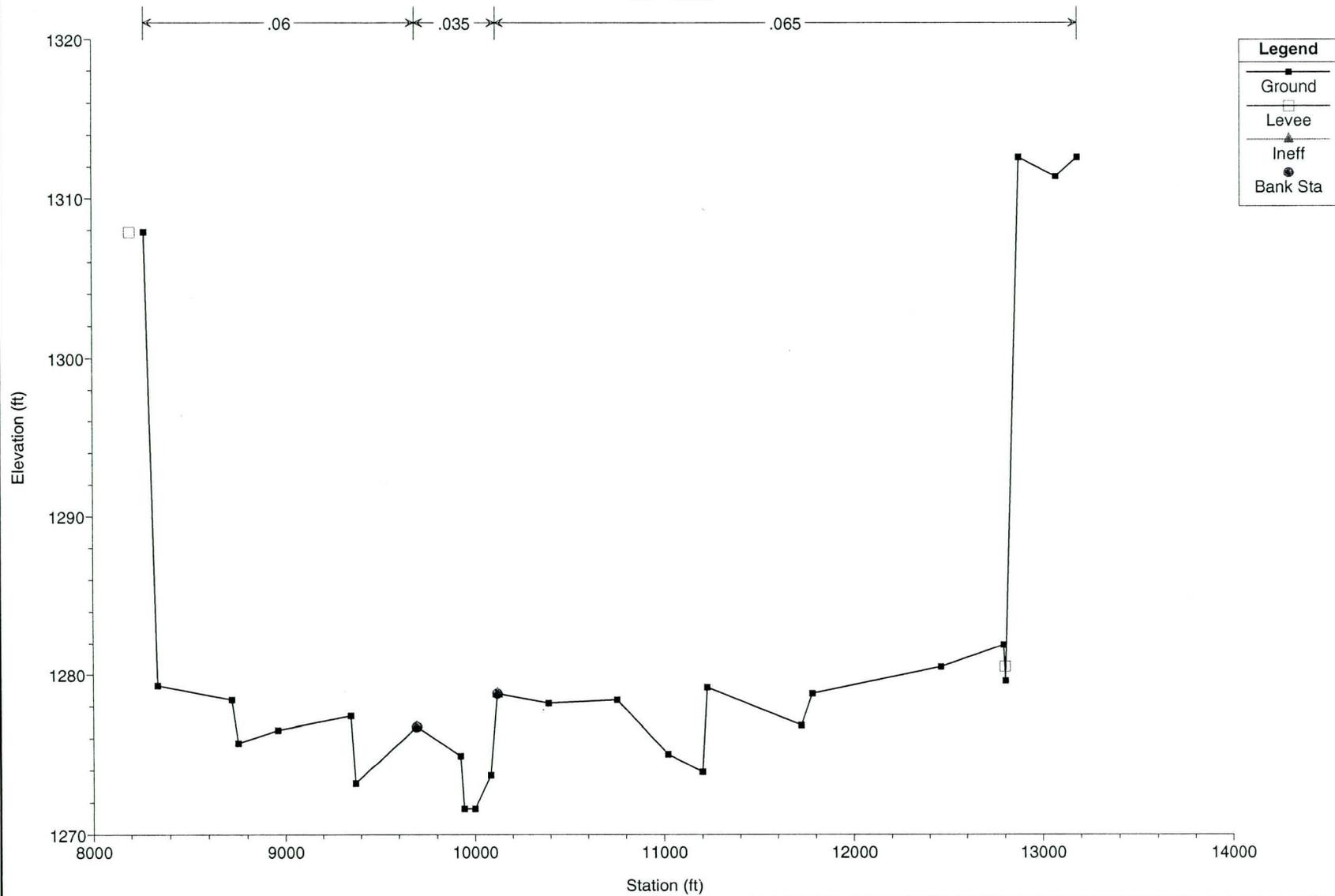
RS = 26.73



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

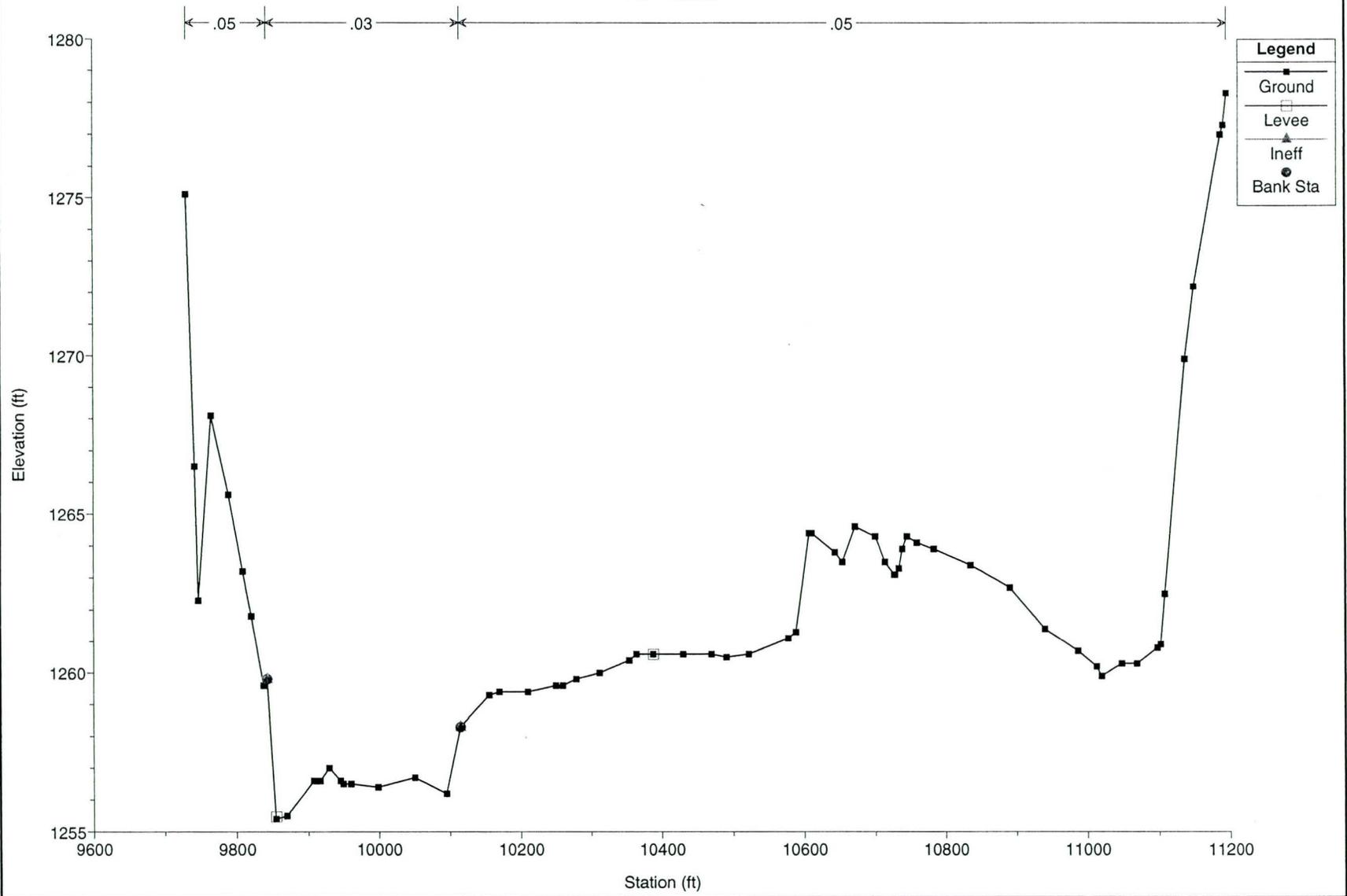
RS = 26.29



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

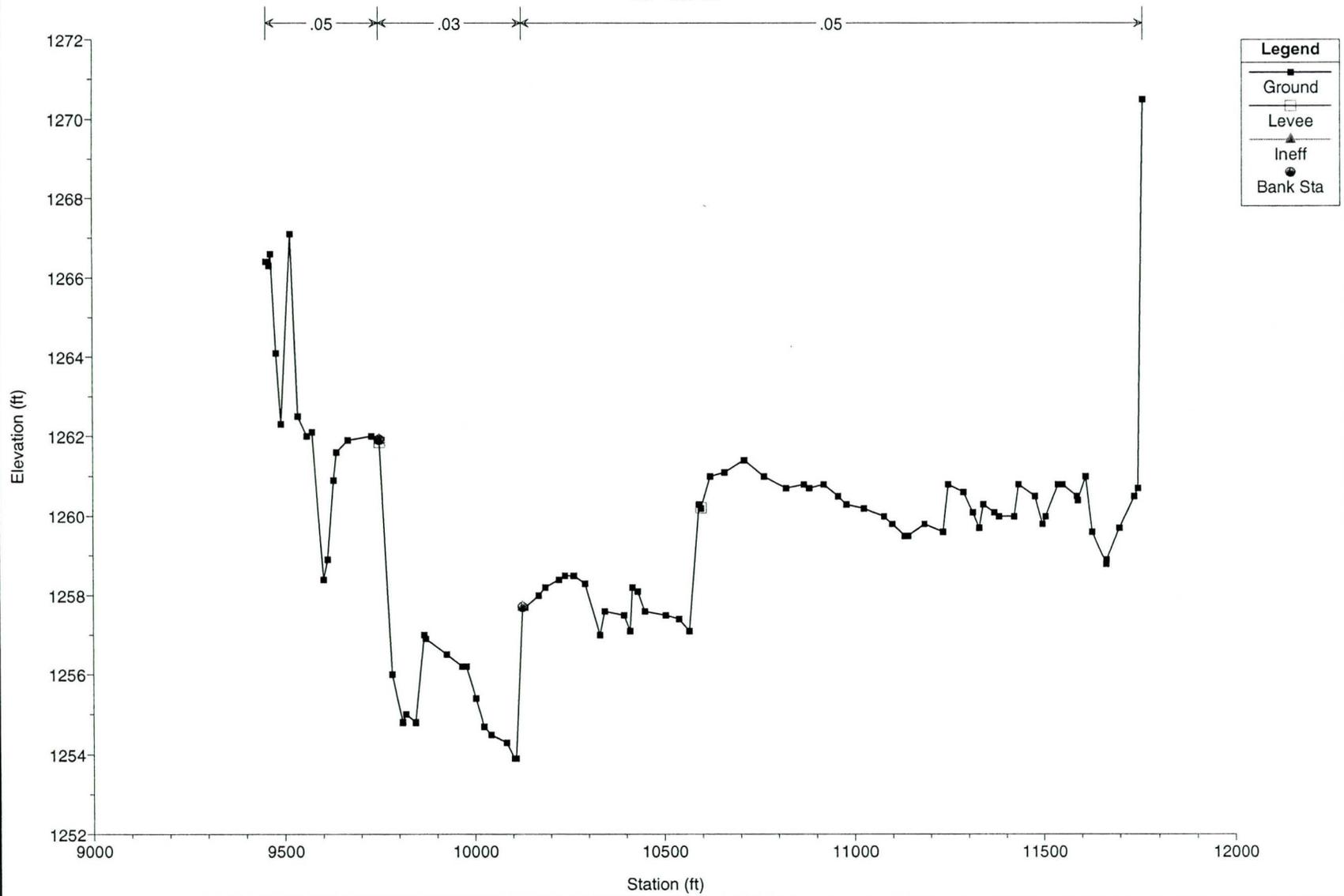
RS = 25.666



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

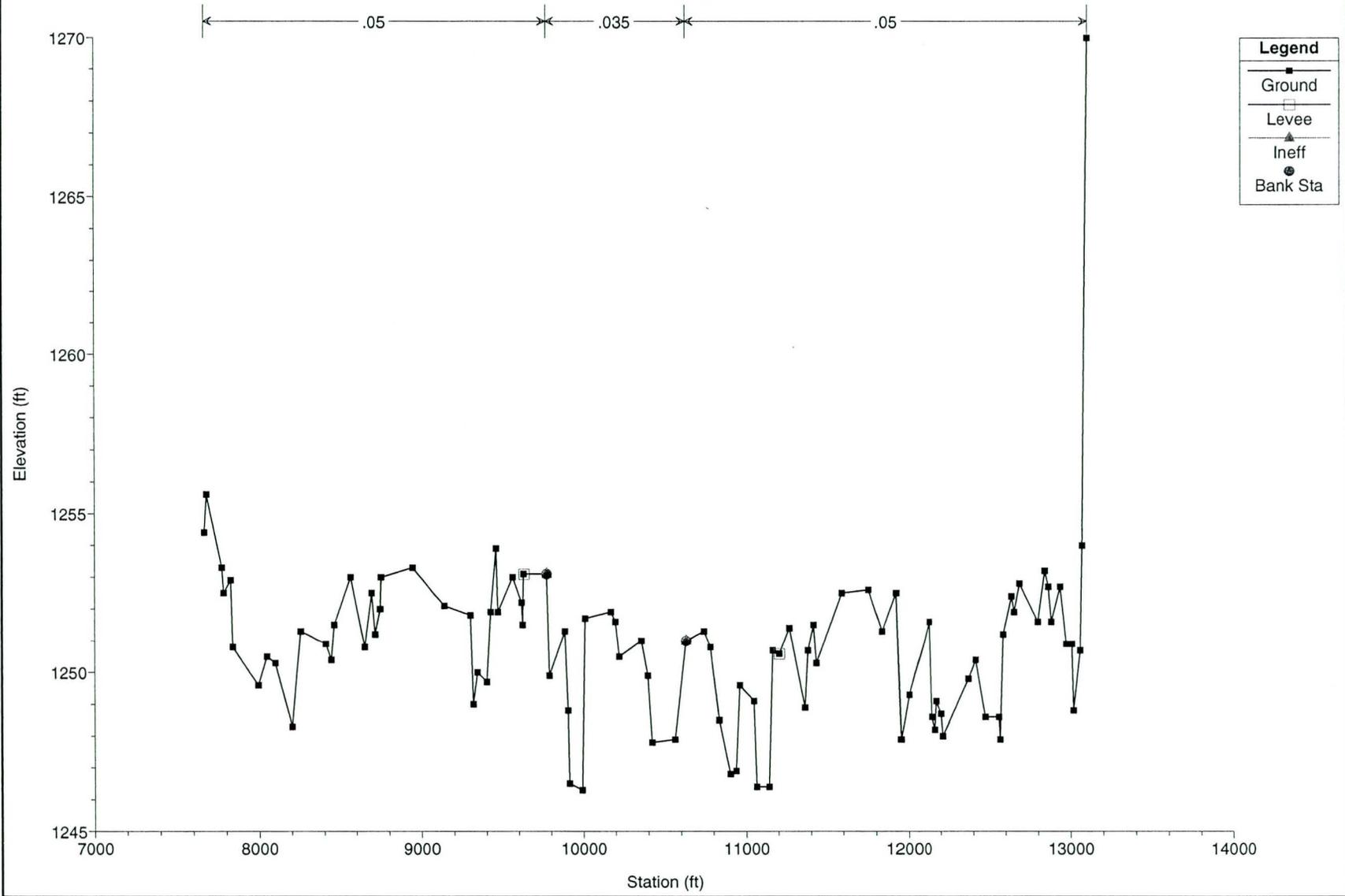
RS = 25.478



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

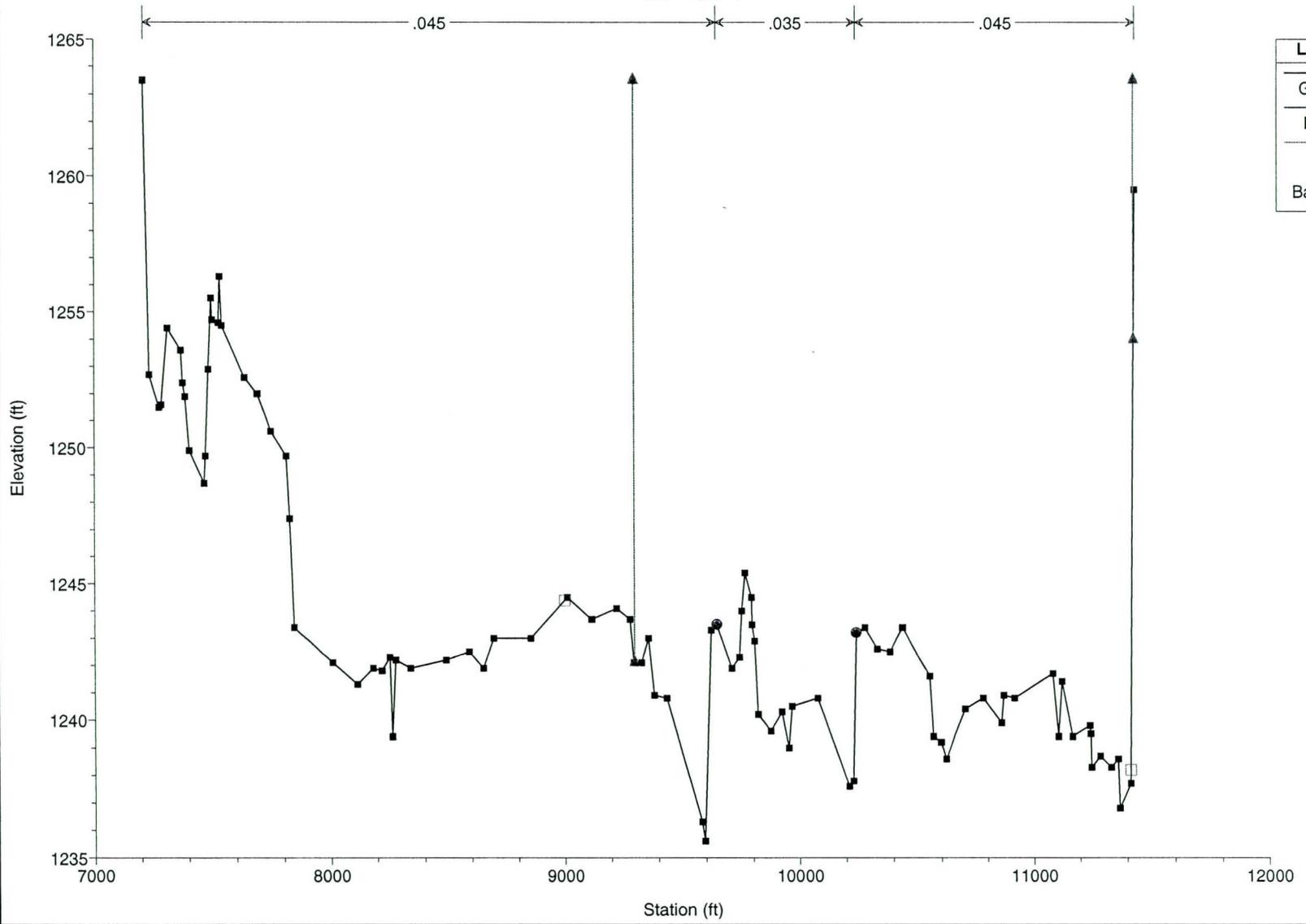
RS = 24.996



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

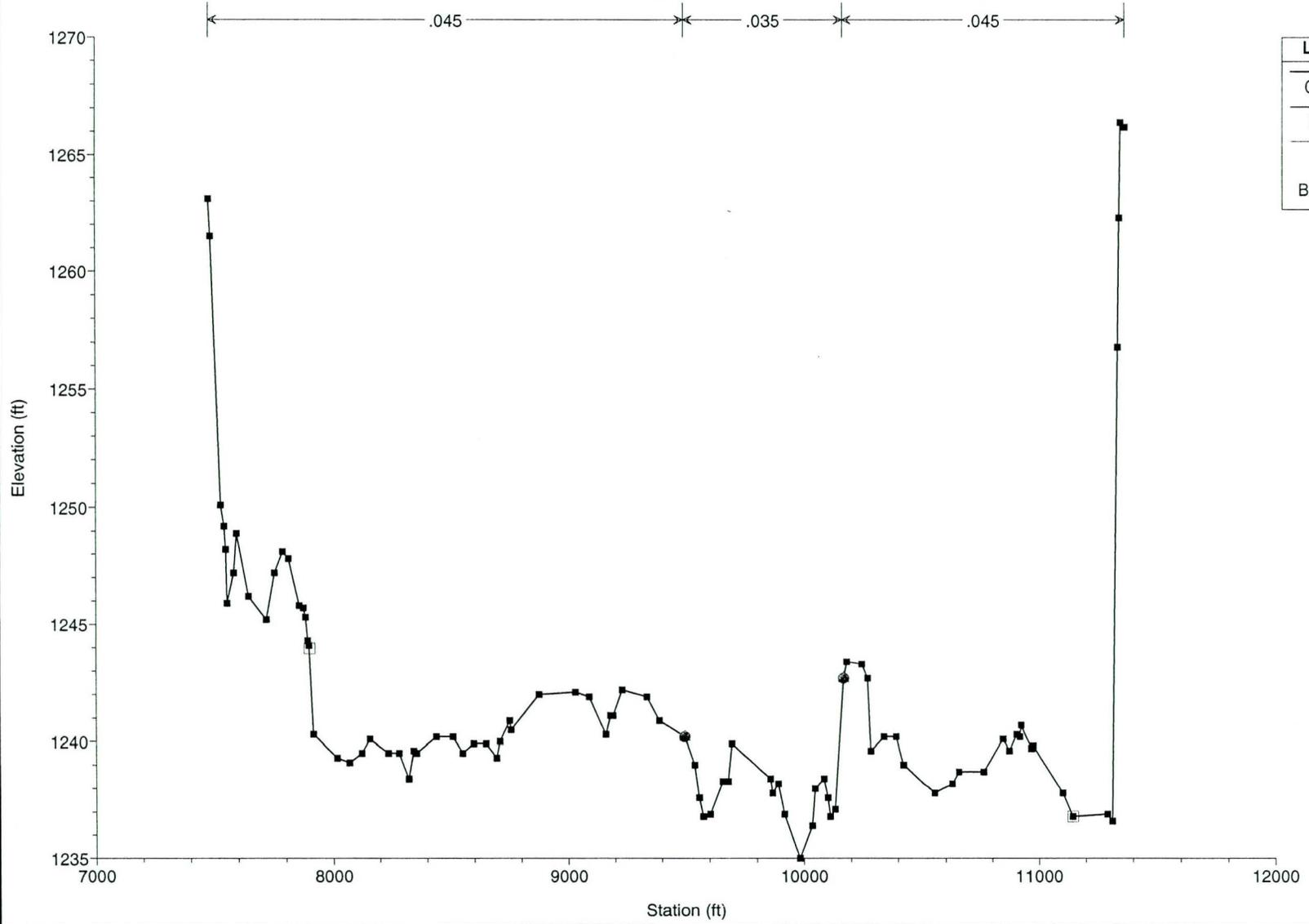
RS = 24.543



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

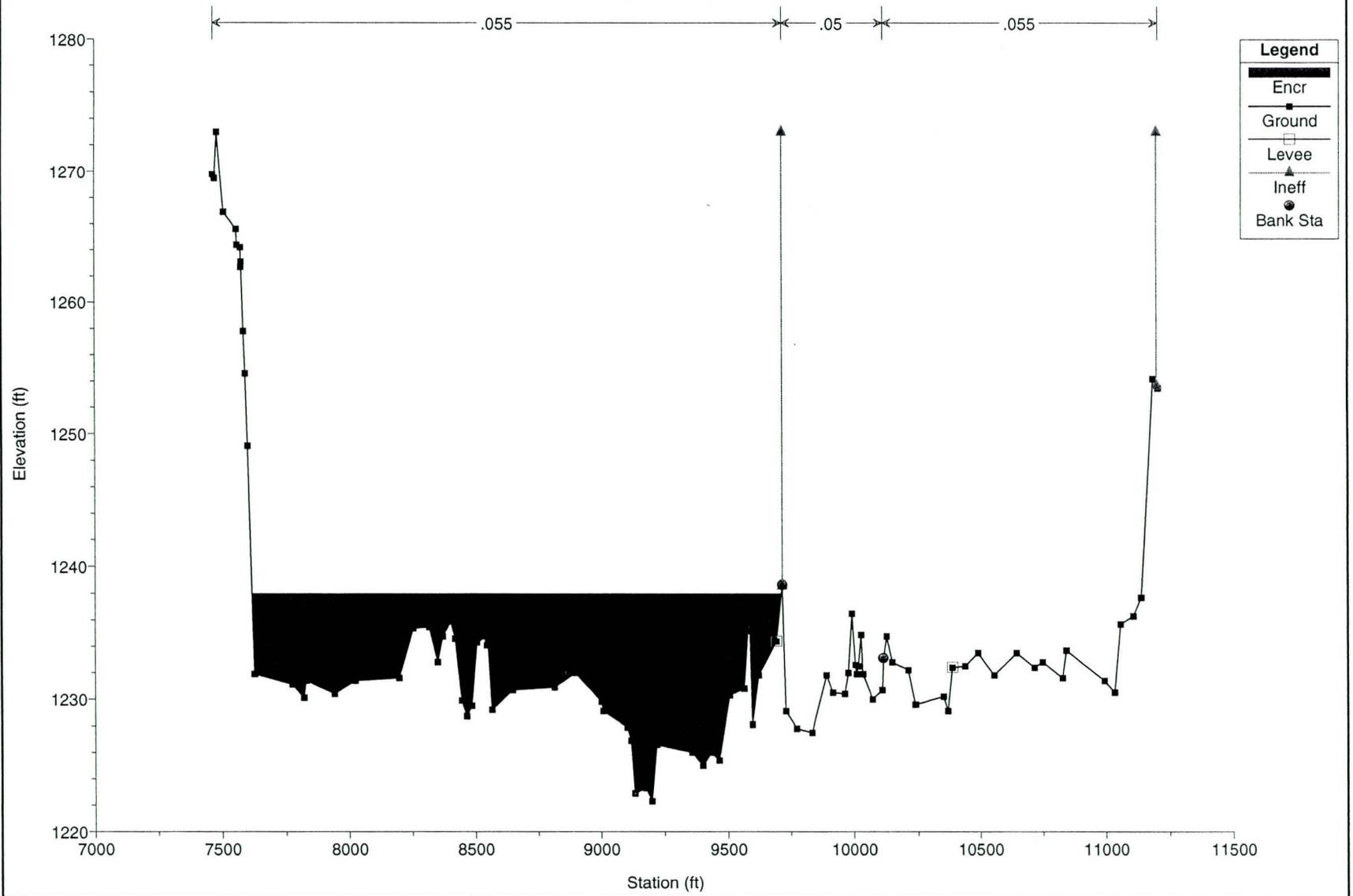
RS = 24.449



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

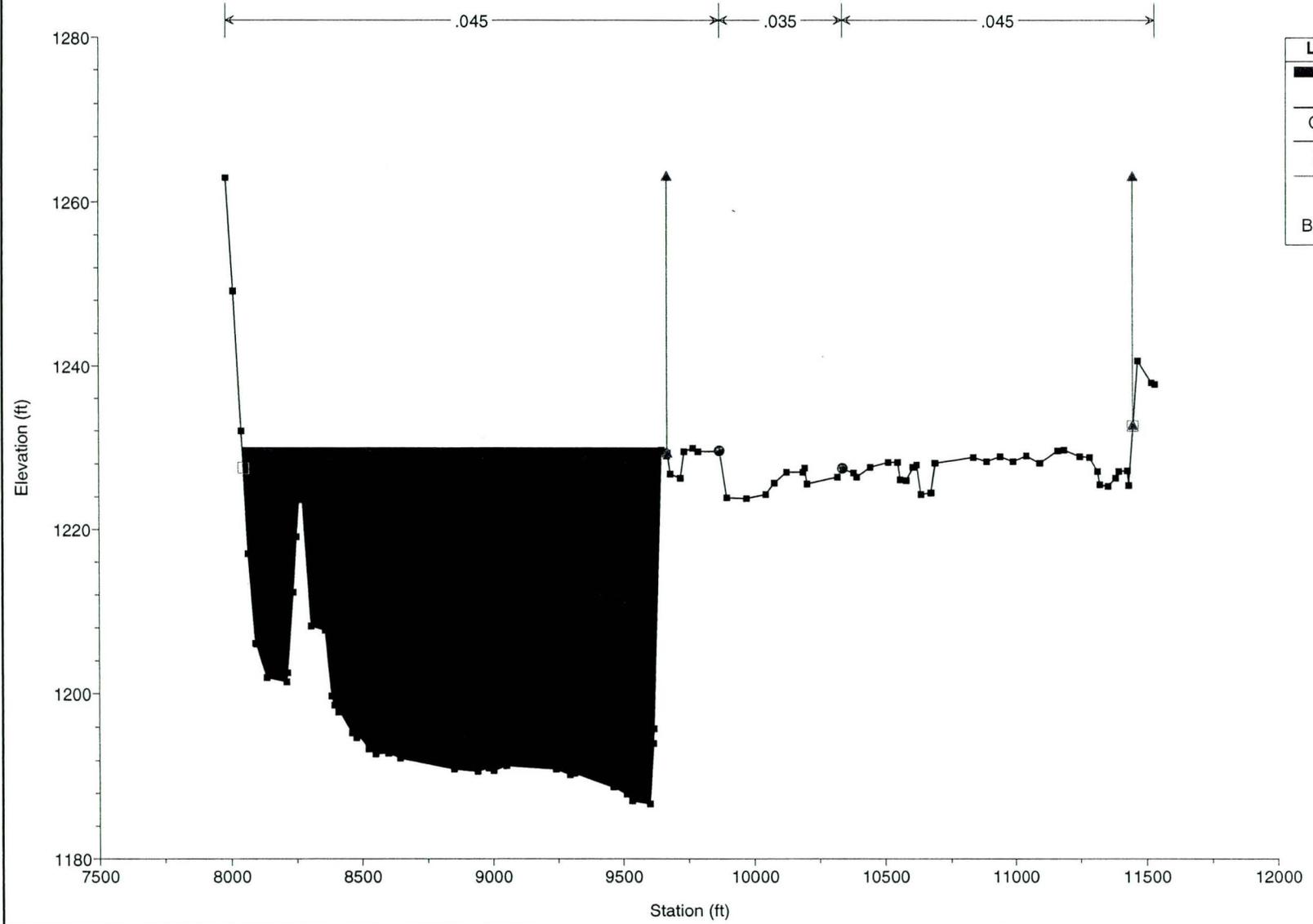
RS = 23.974



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

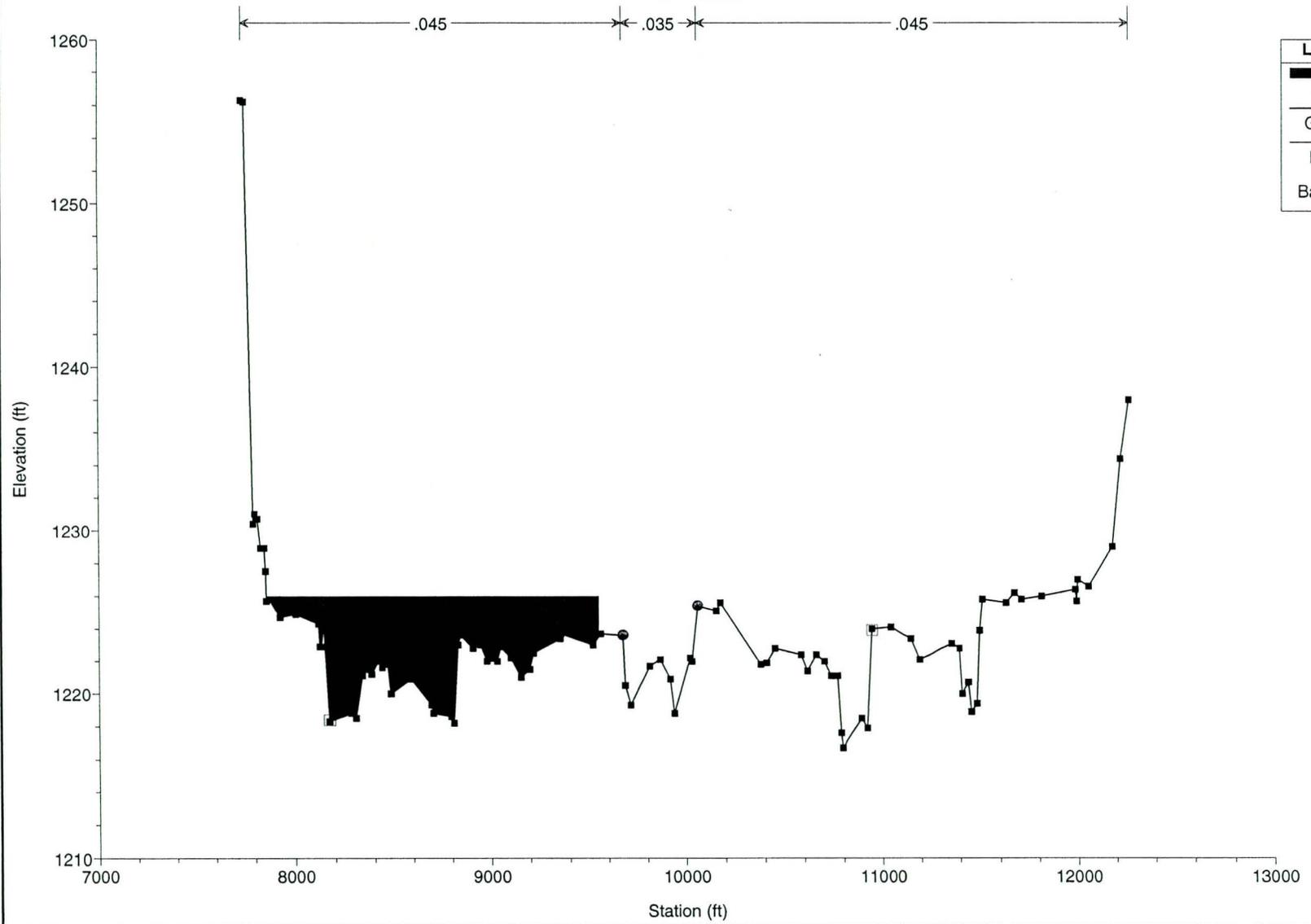
RS = 23.692



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

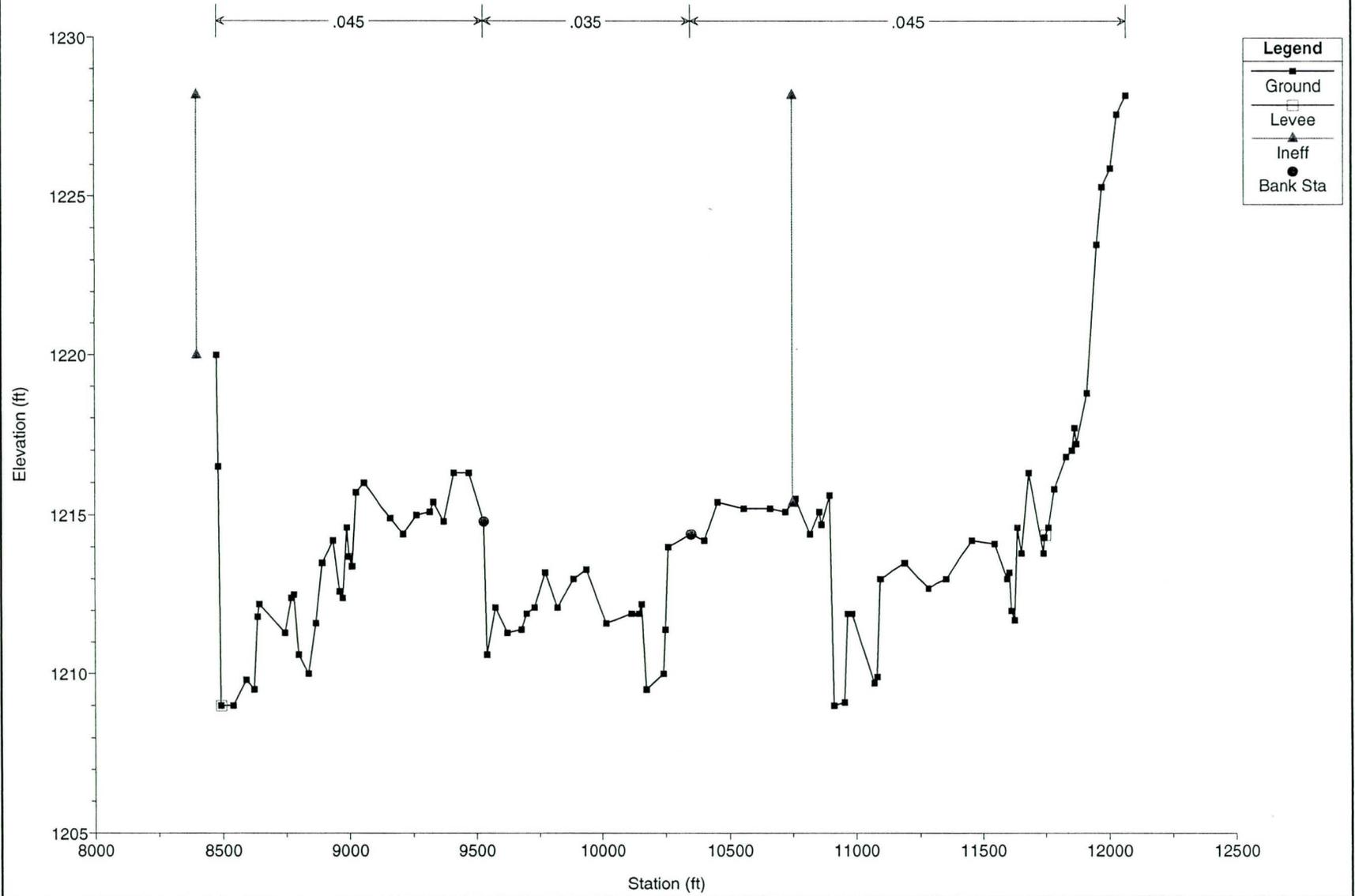
RS = 23.409



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

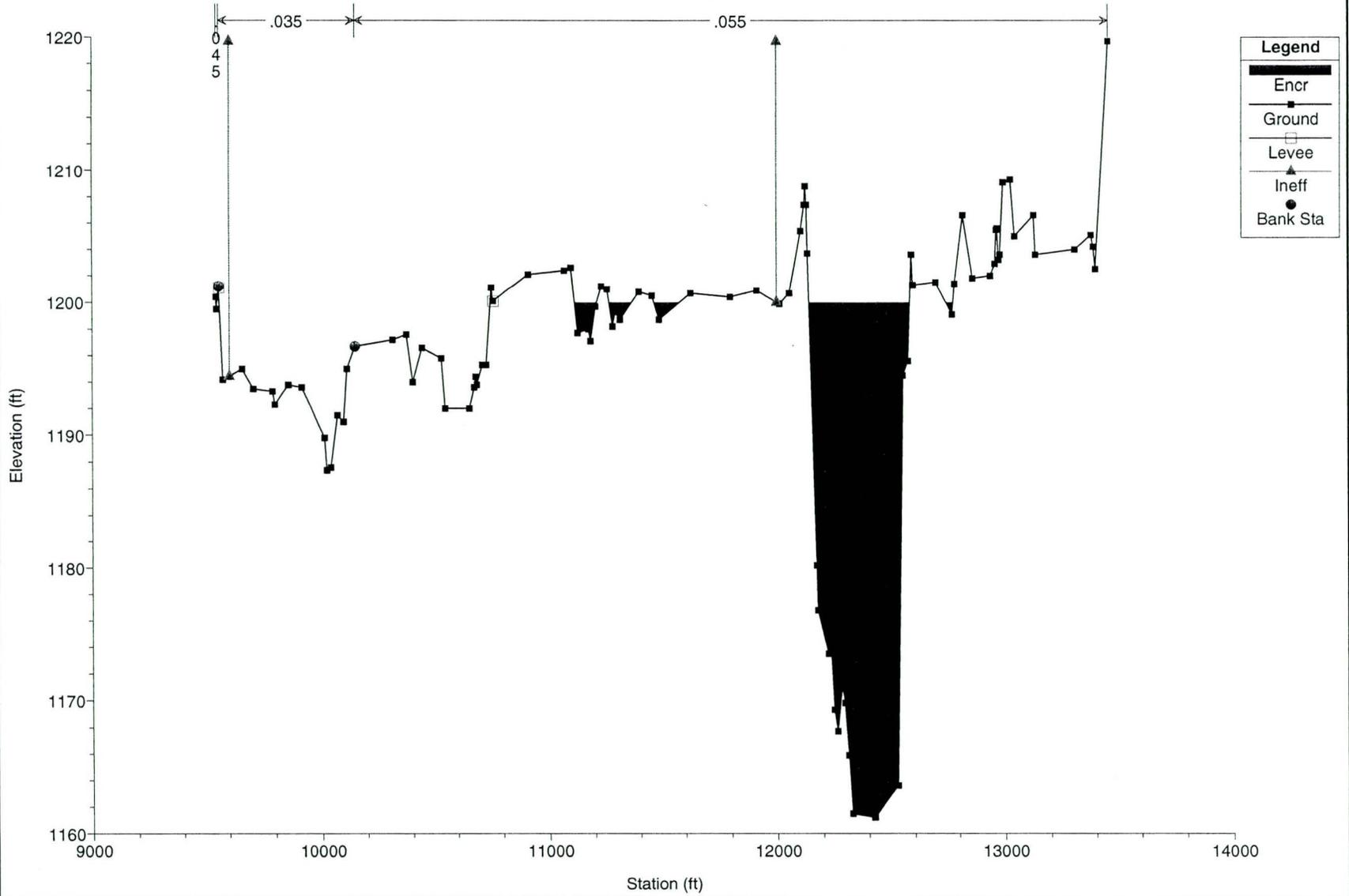
RS = 22.935



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

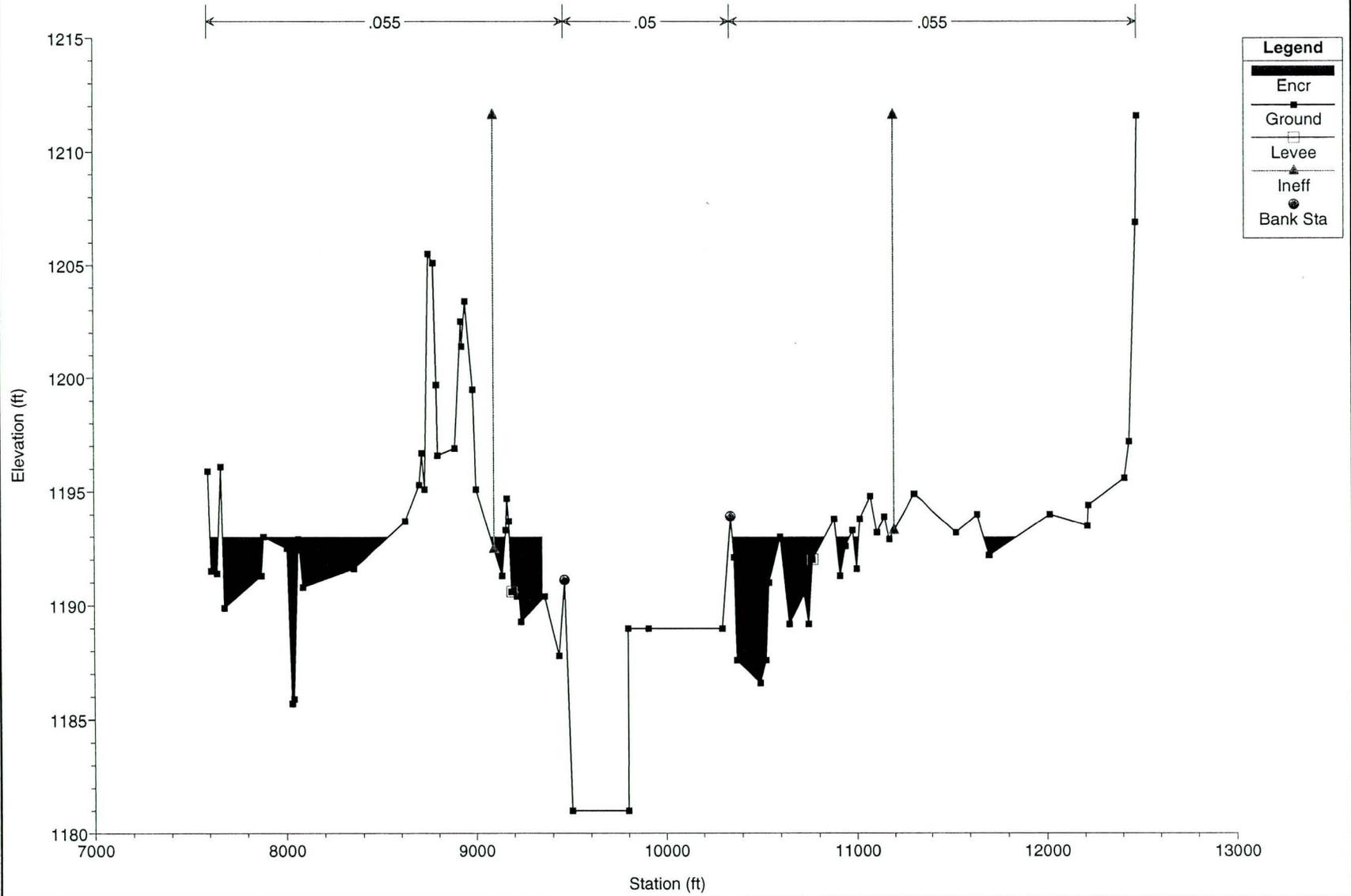
RS = 21.893



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

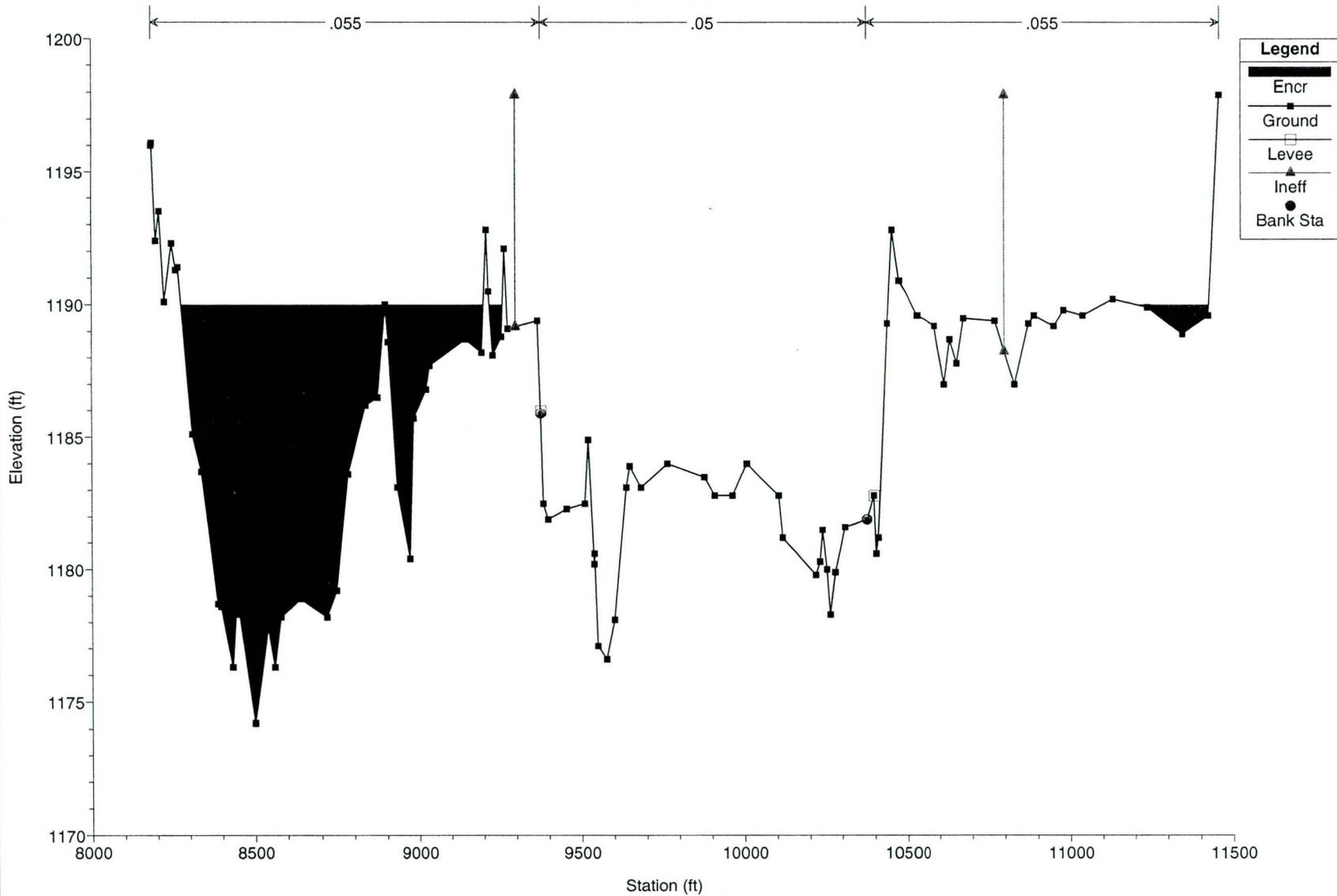
RS = 21.431



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

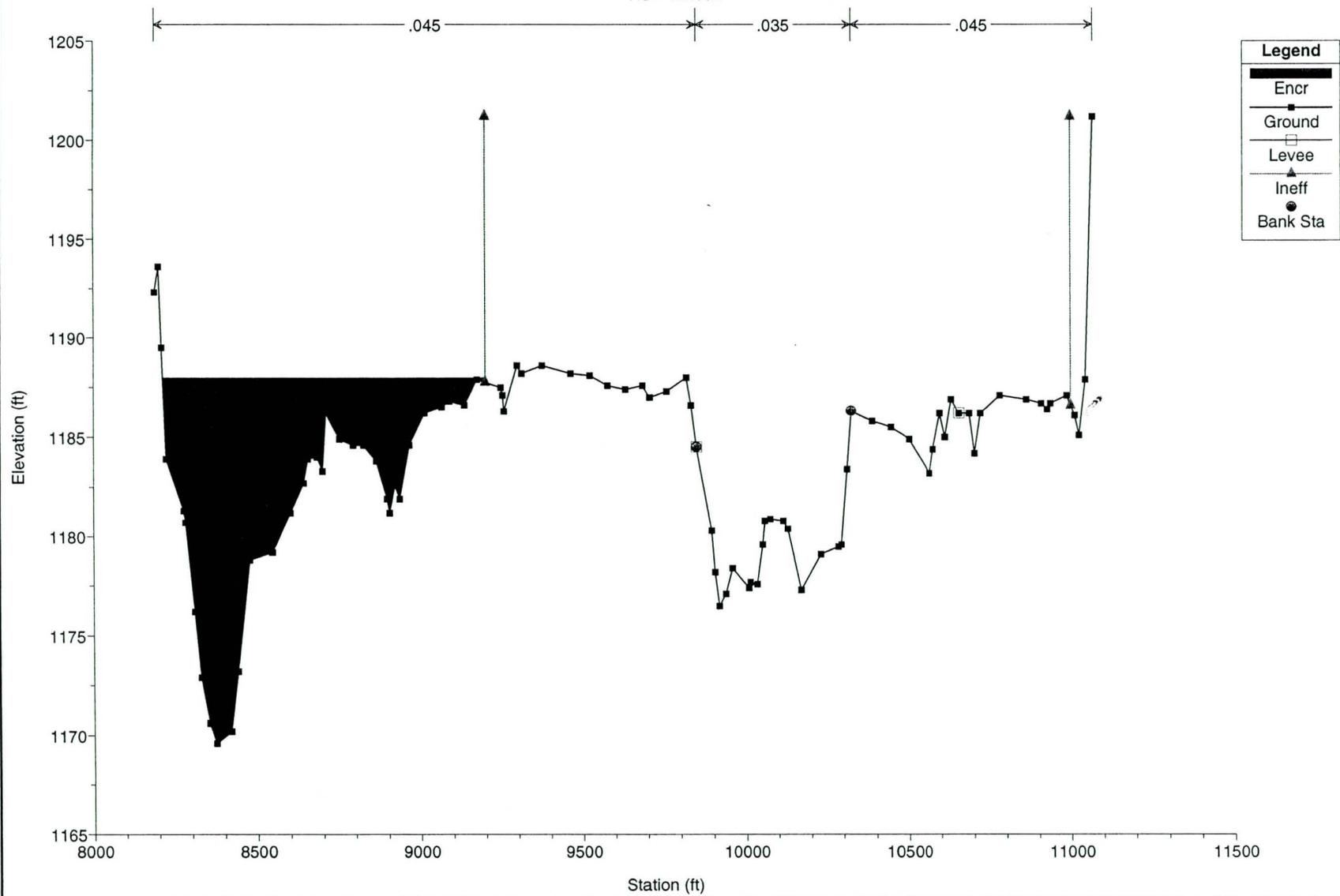
RS = 21.157



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

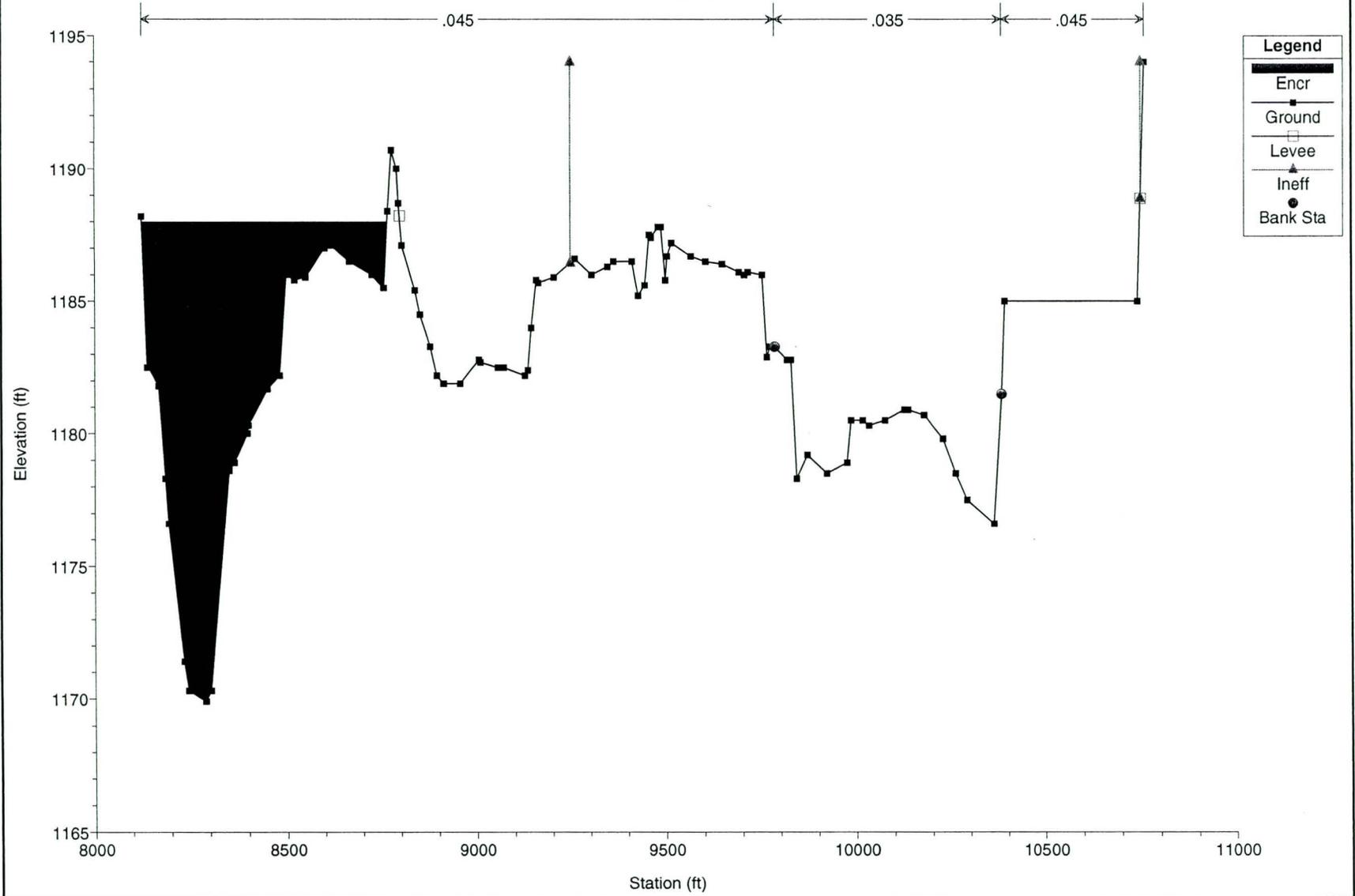
RS = 21.061



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

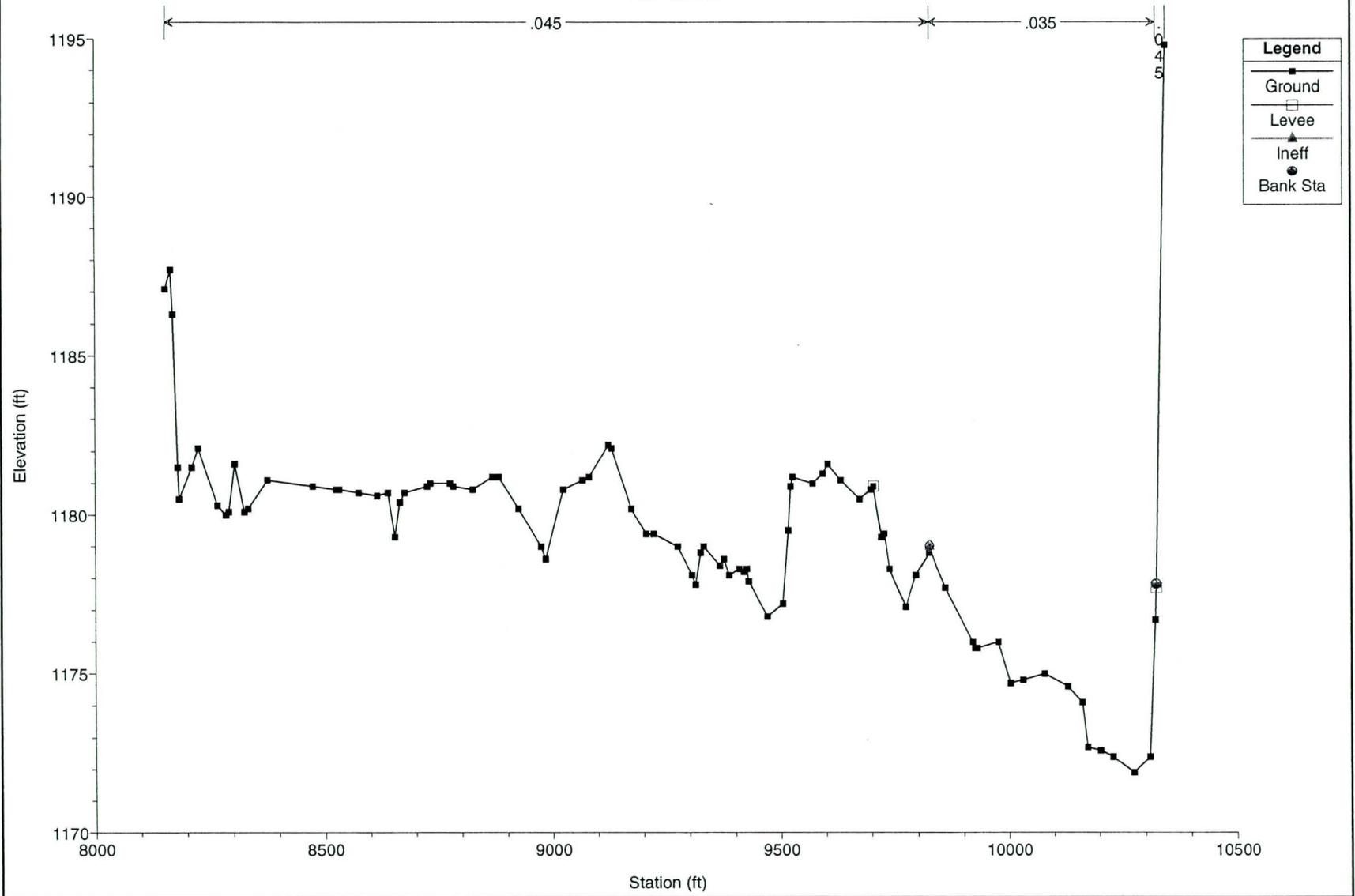
RS = 20.958



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

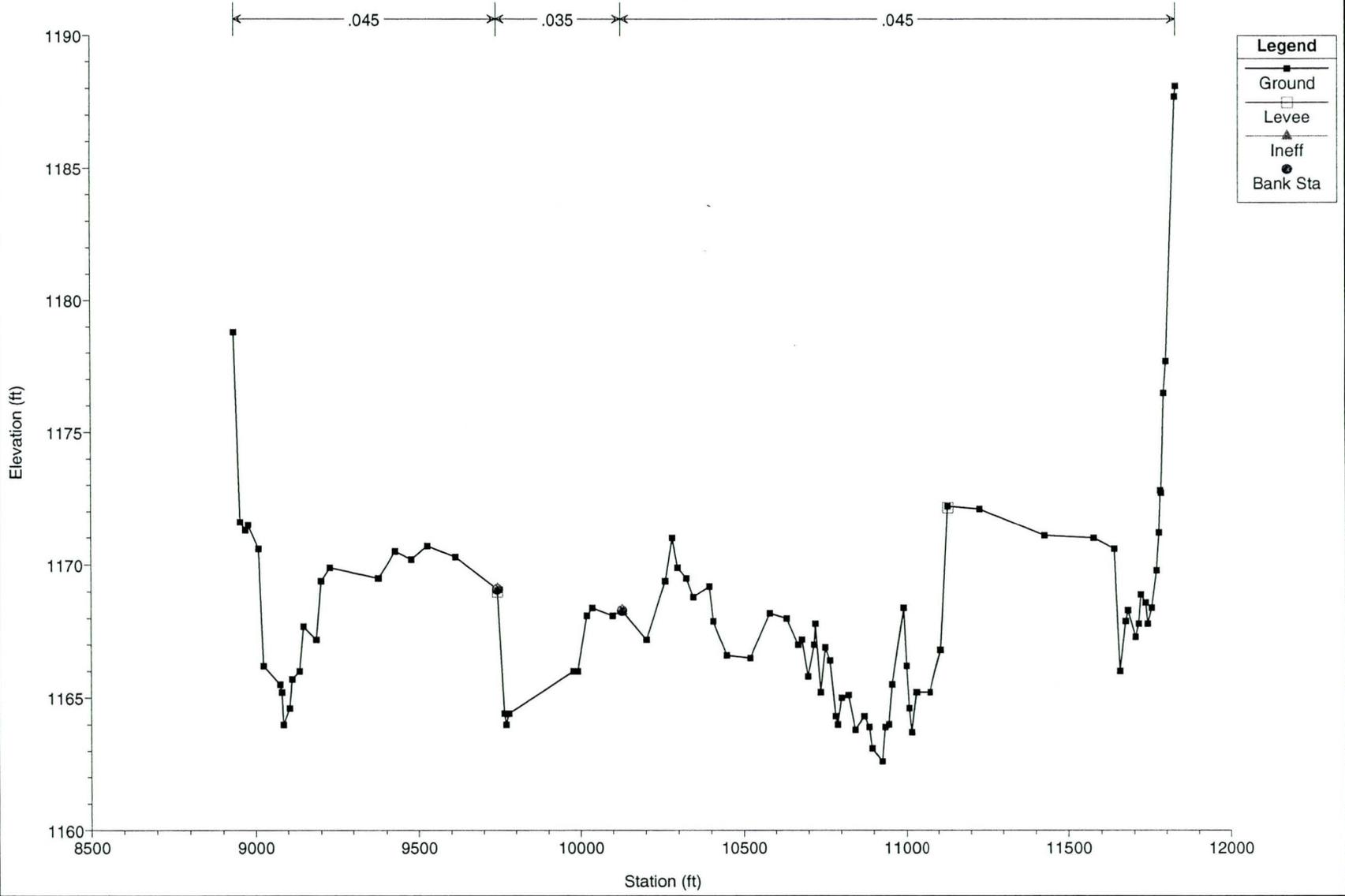
RS = 20.579



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

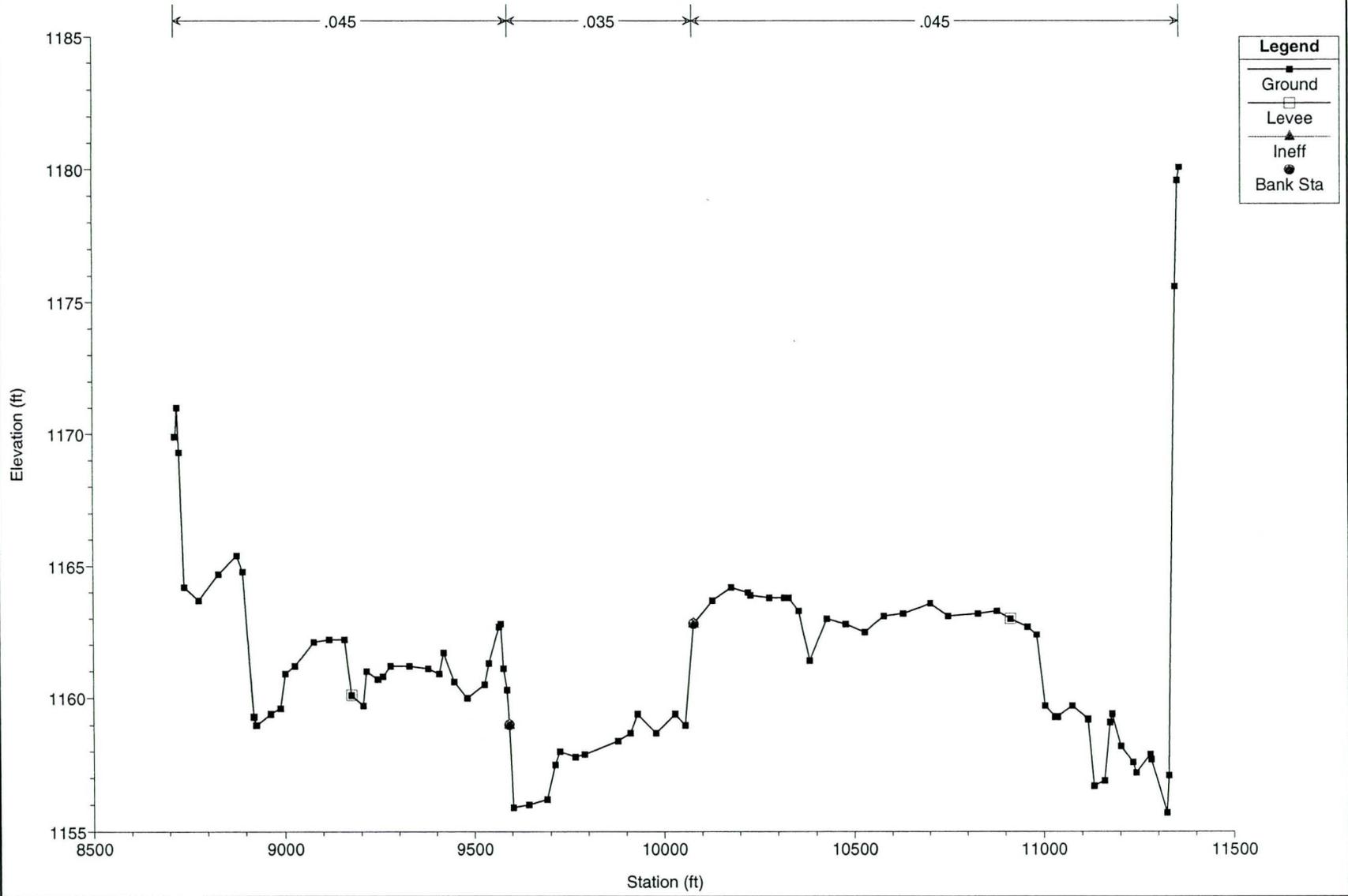
RS = 19.92



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

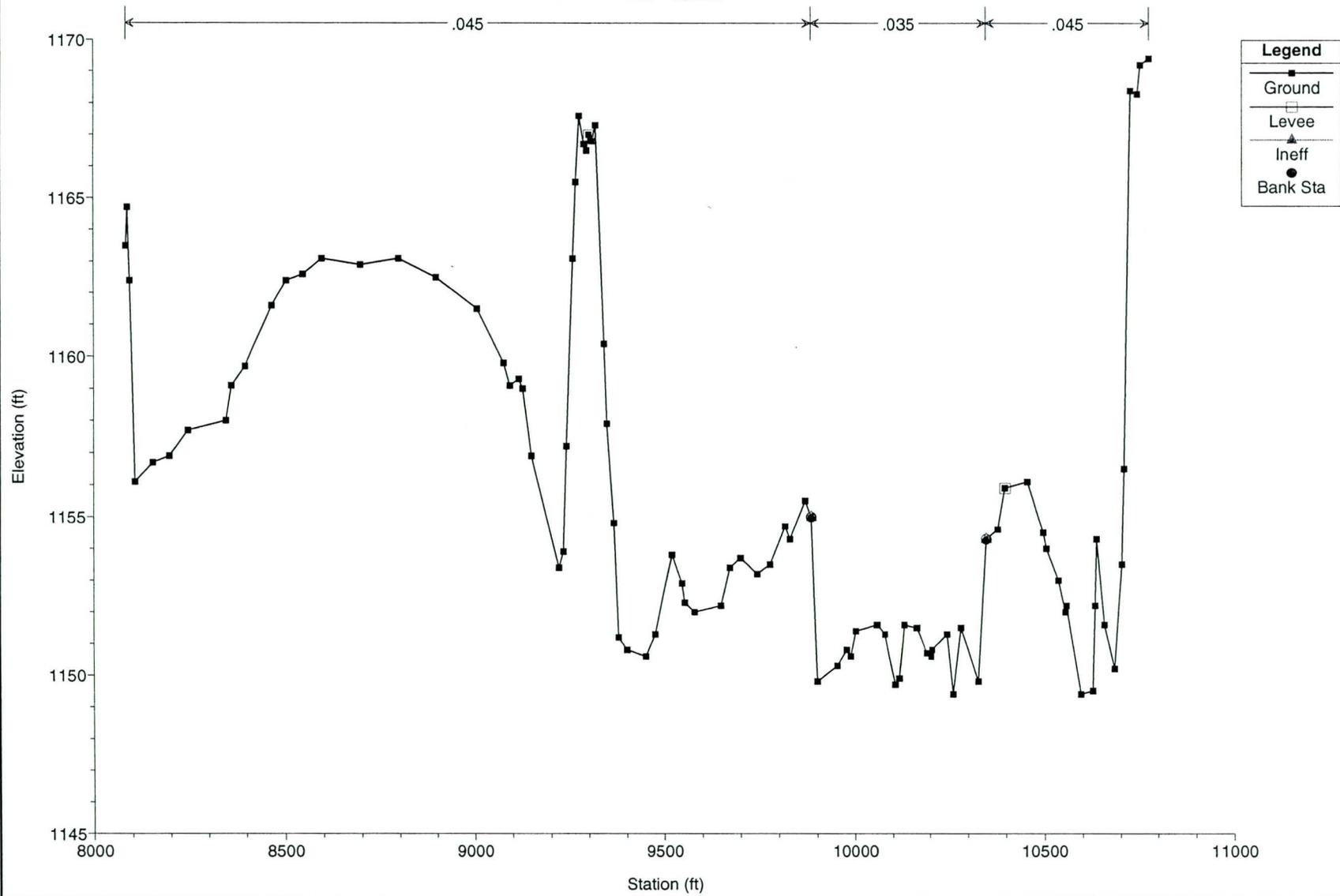
RS = 19.446



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

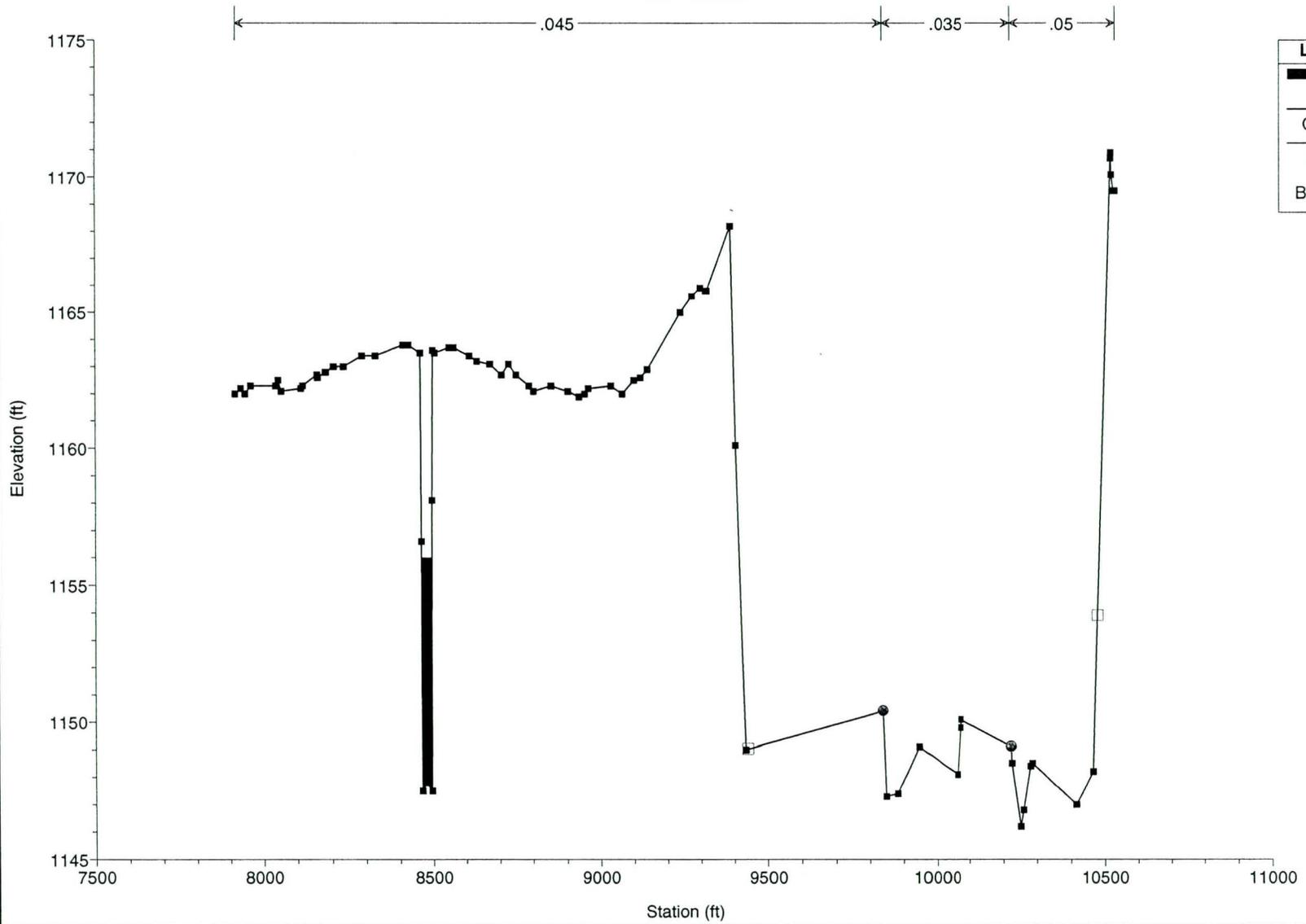
RS = 19.066



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

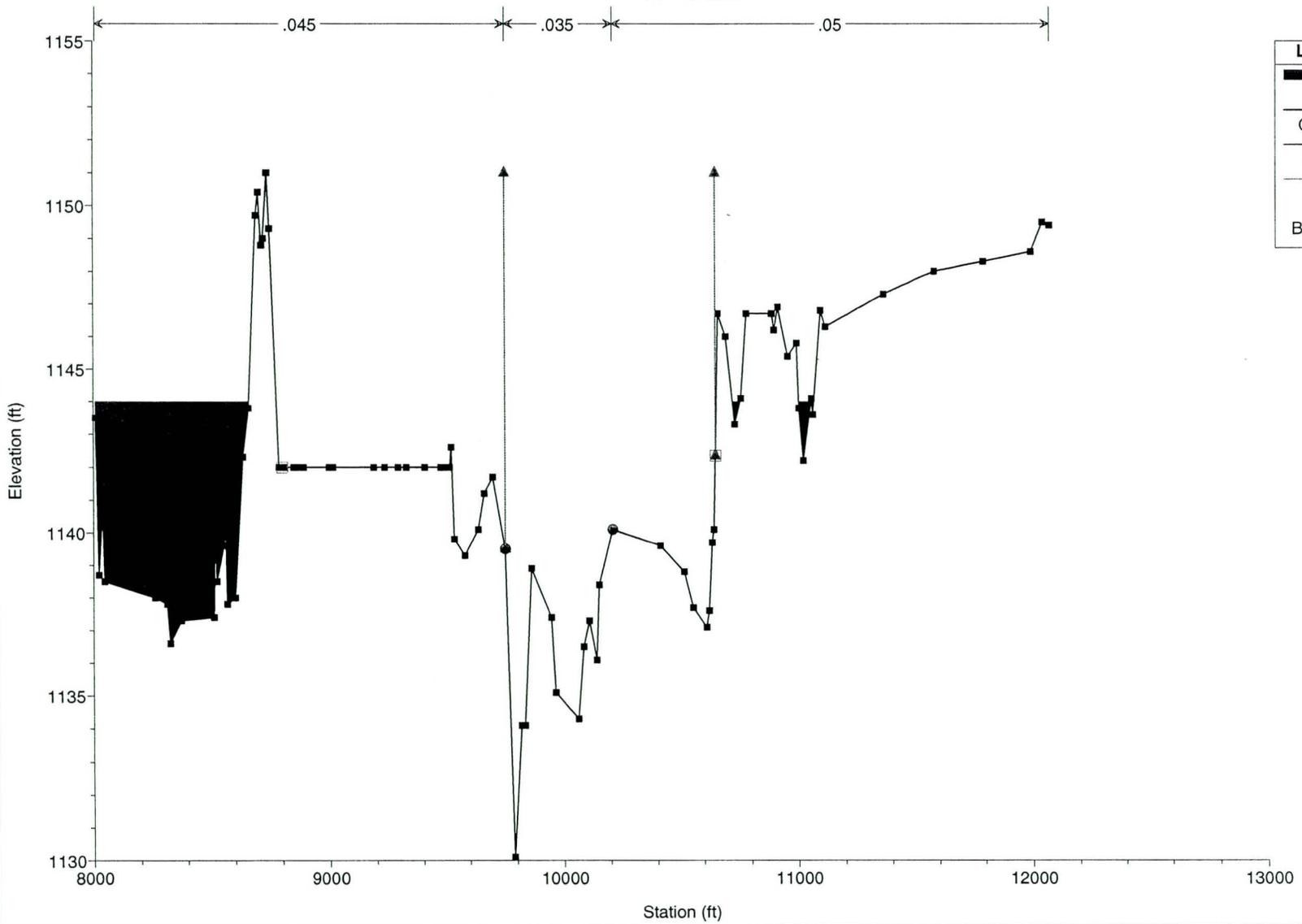
RS = 18.962



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

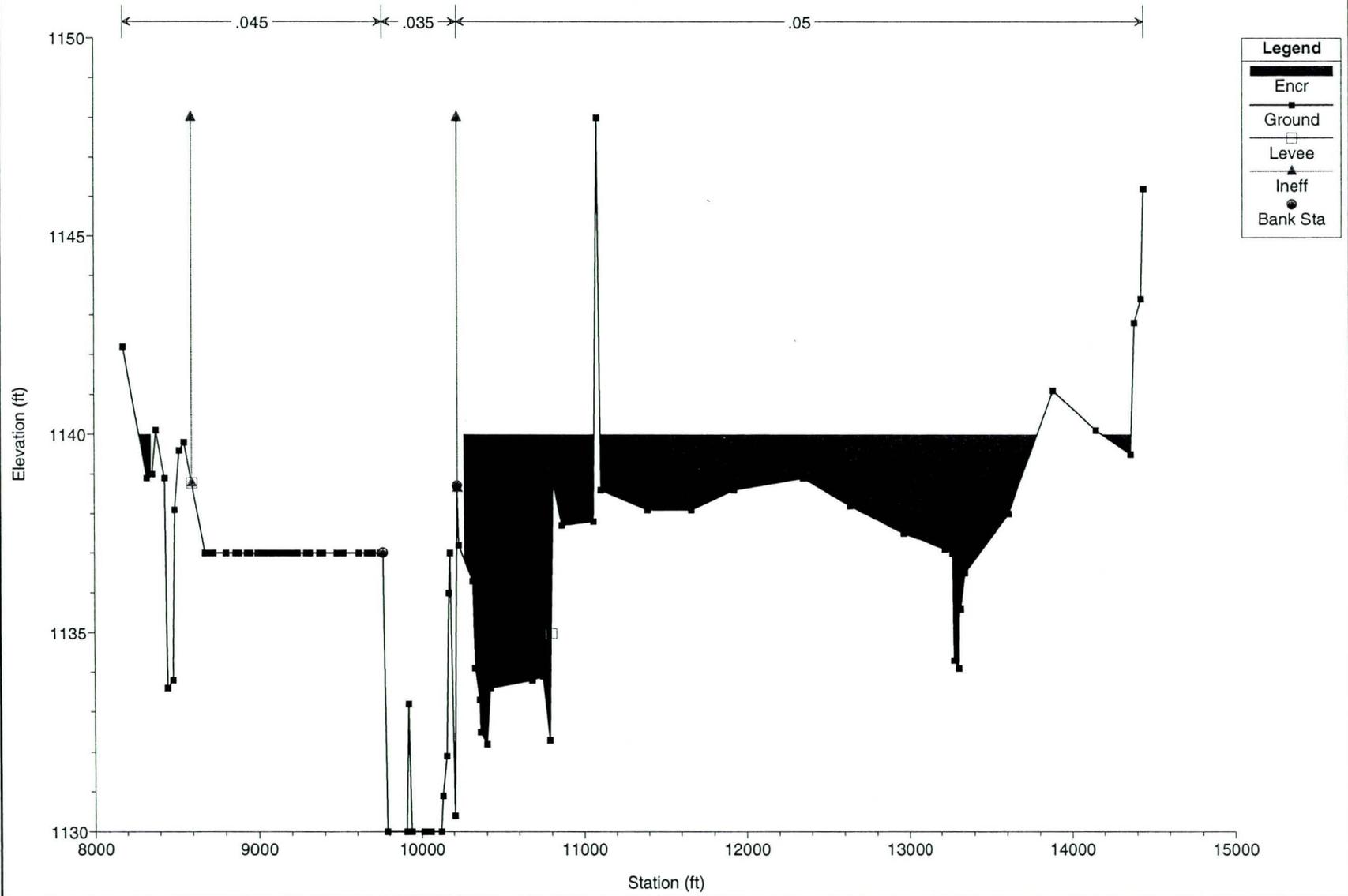
RS = 18.275



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

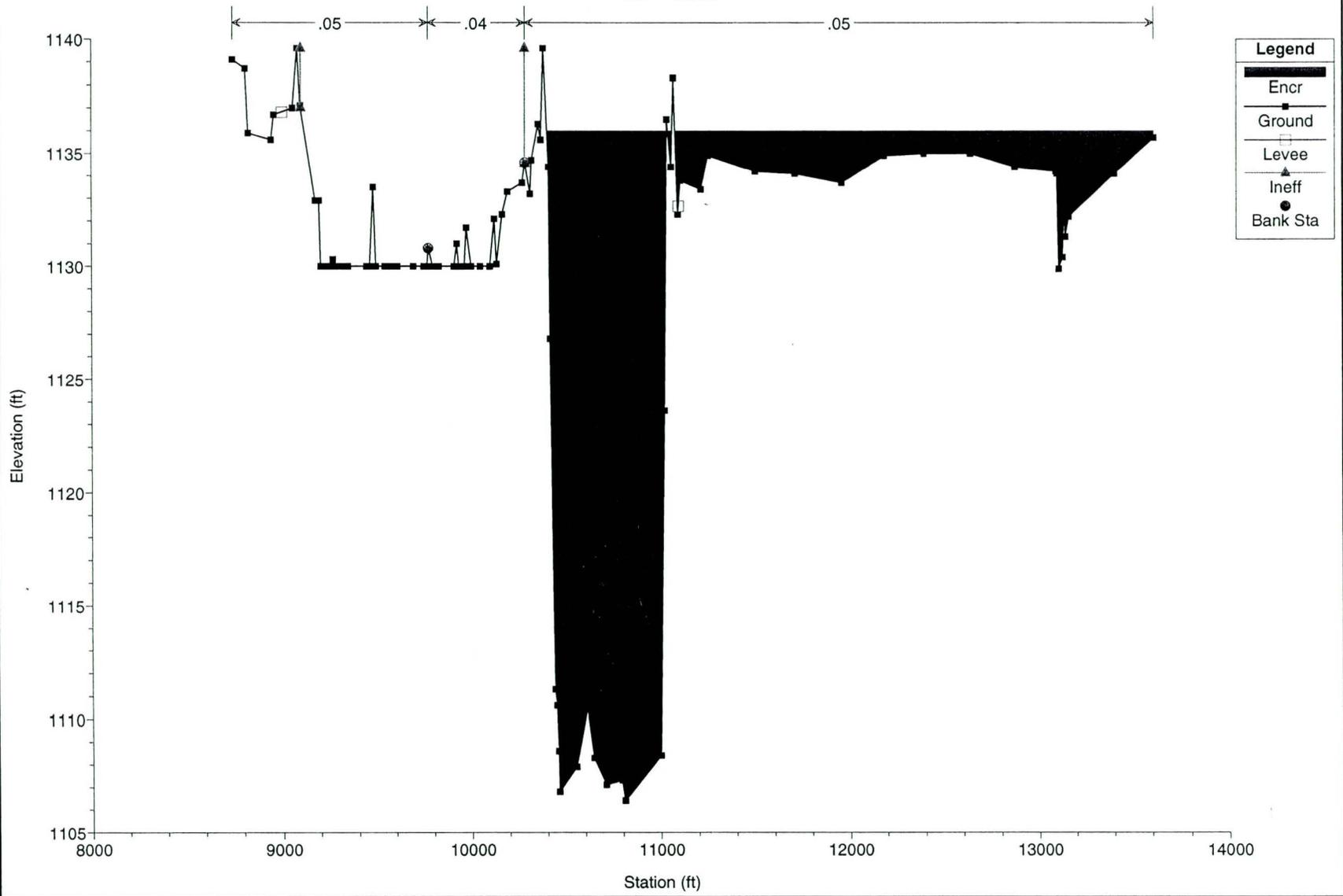
RS = 17.821



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

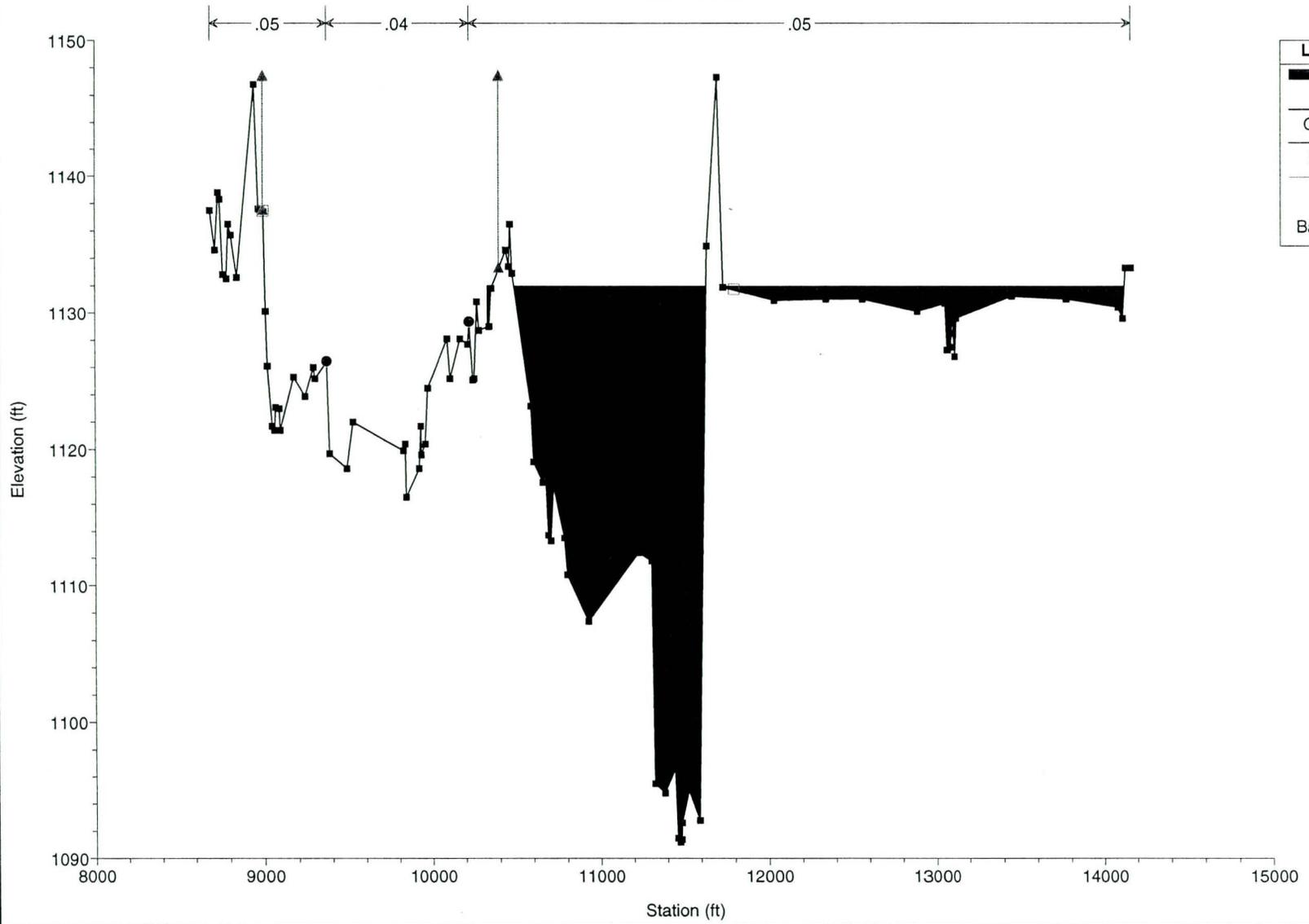
RS = 17.638



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

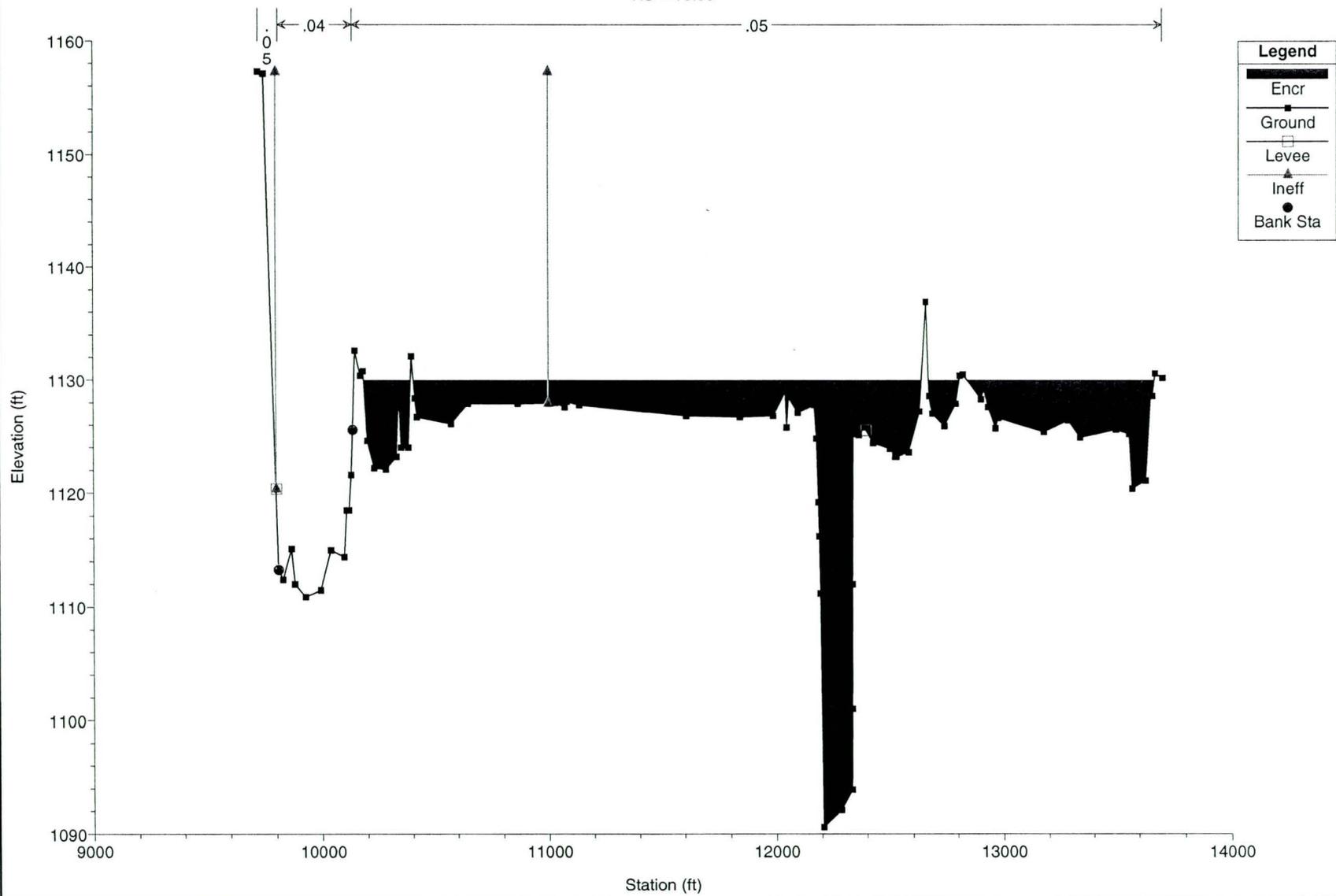
RS = 17.458



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

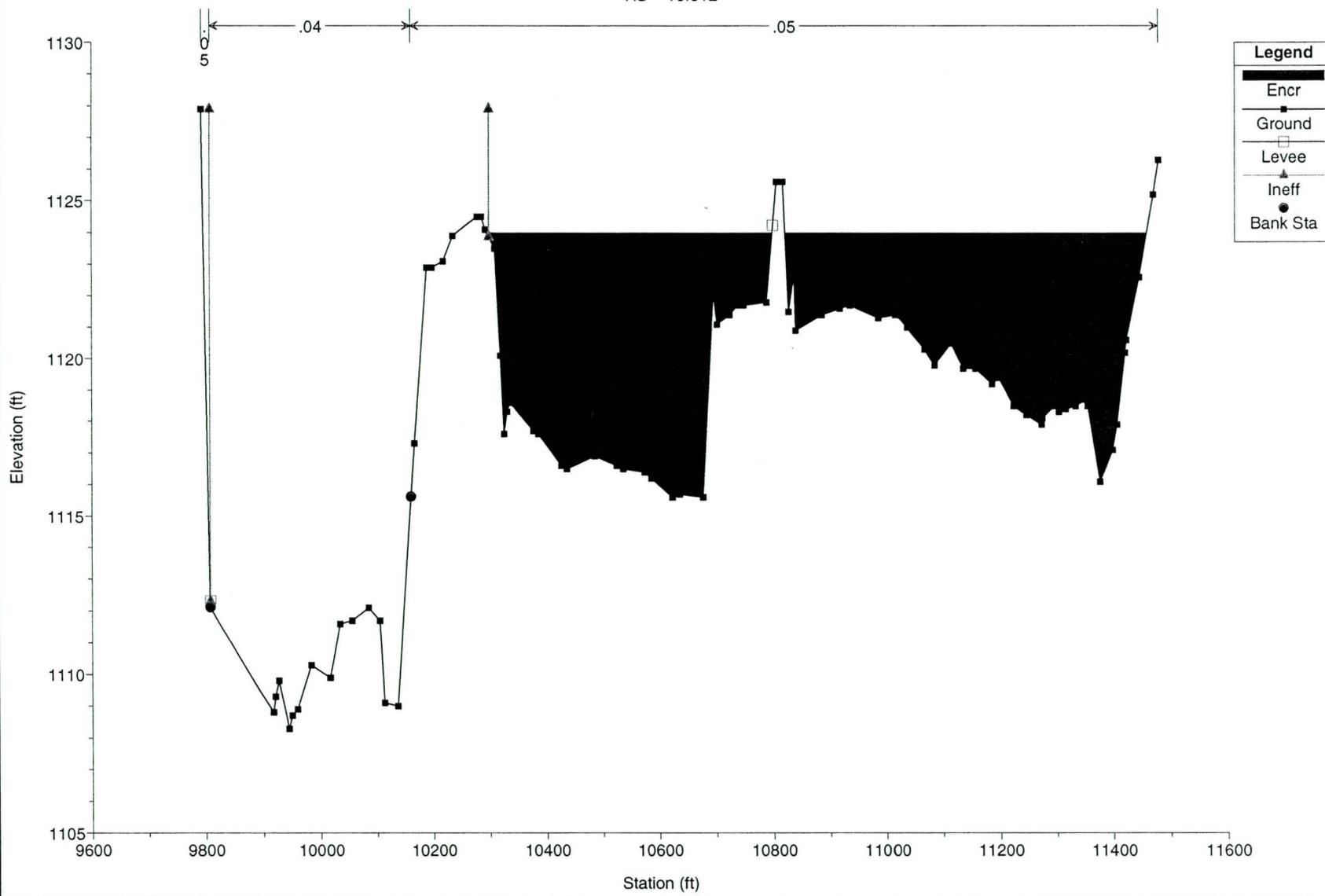
RS = 16.99



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

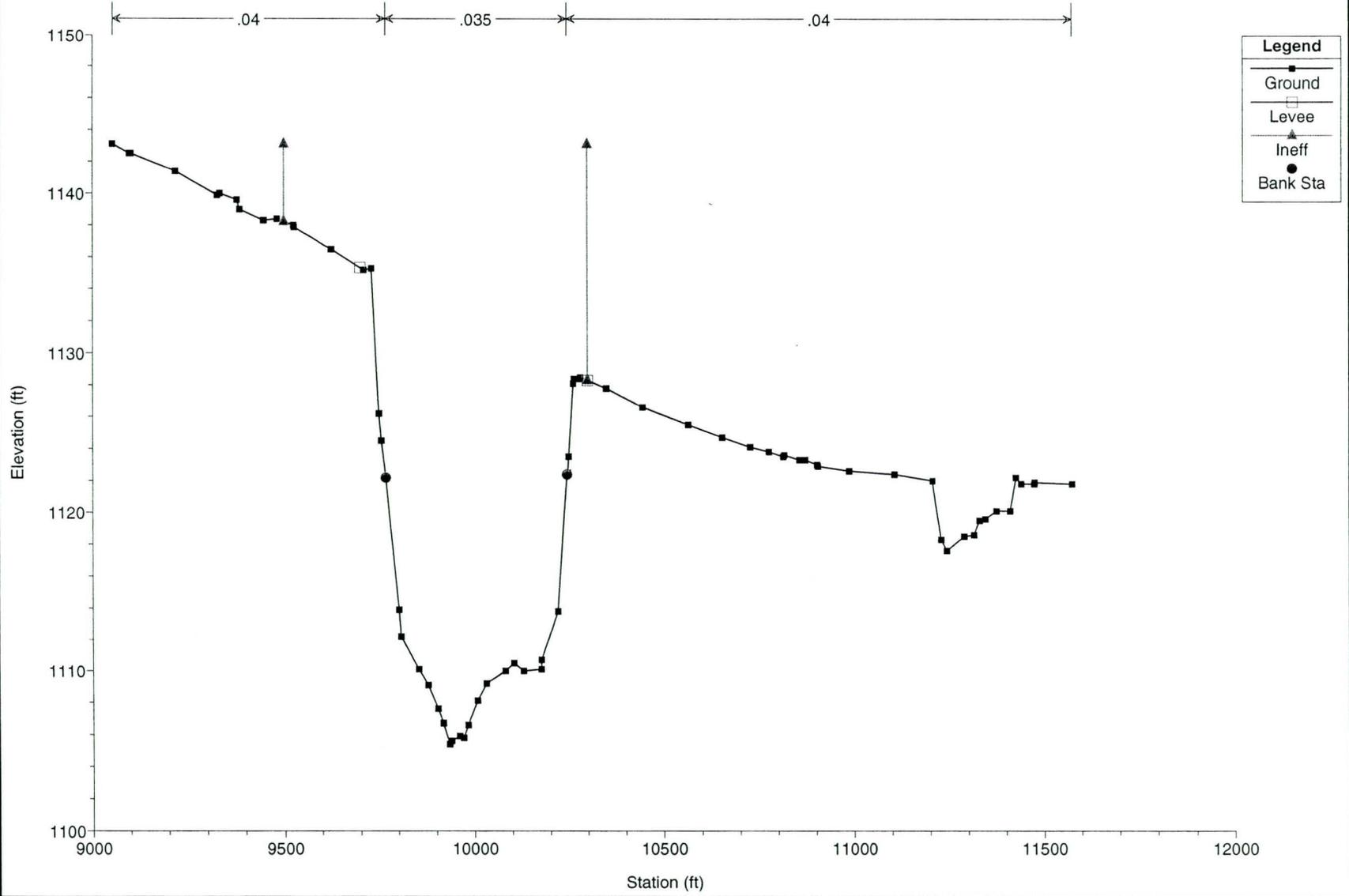
RS = 16.612



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

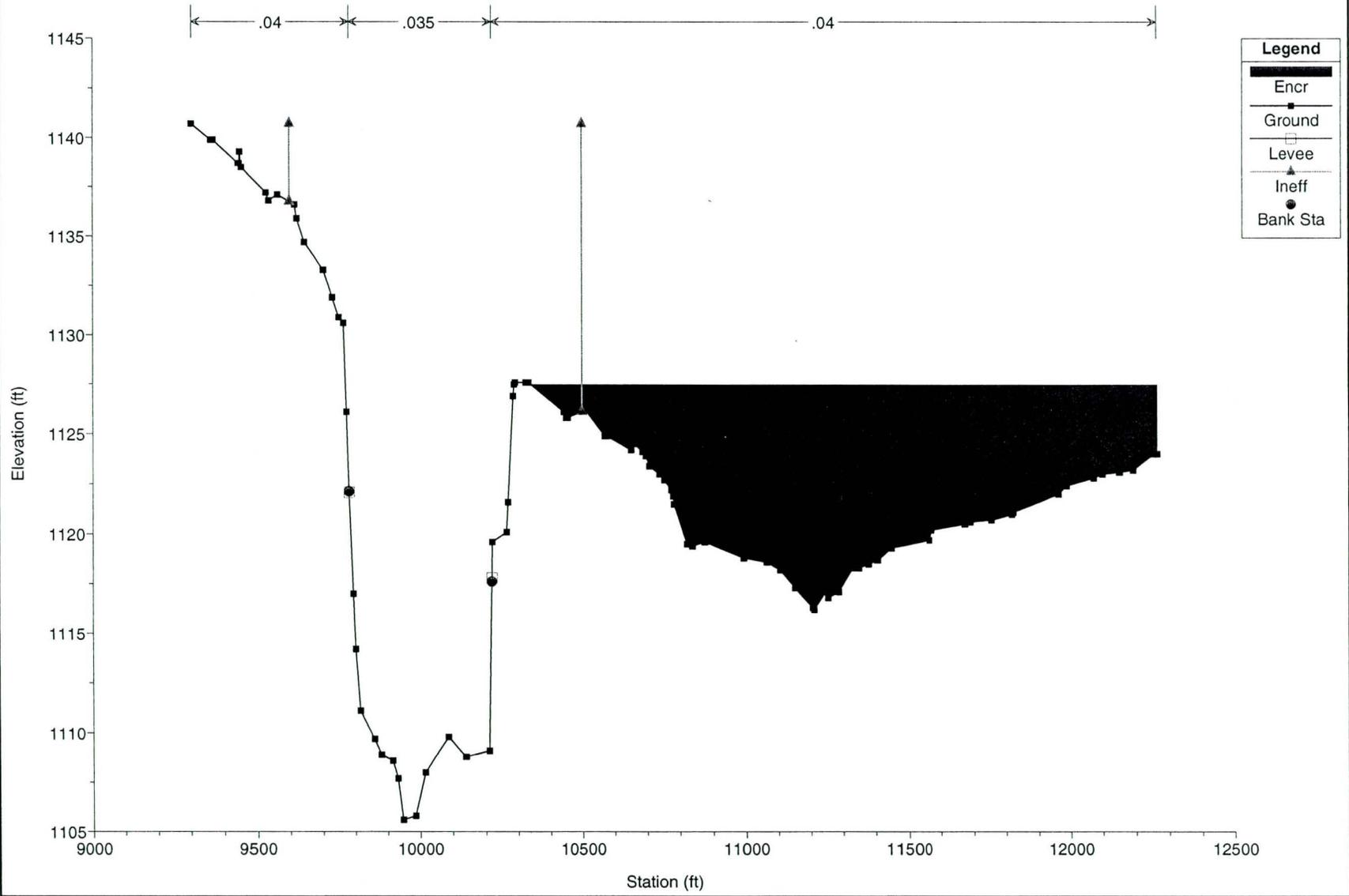
RS = 16.506



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

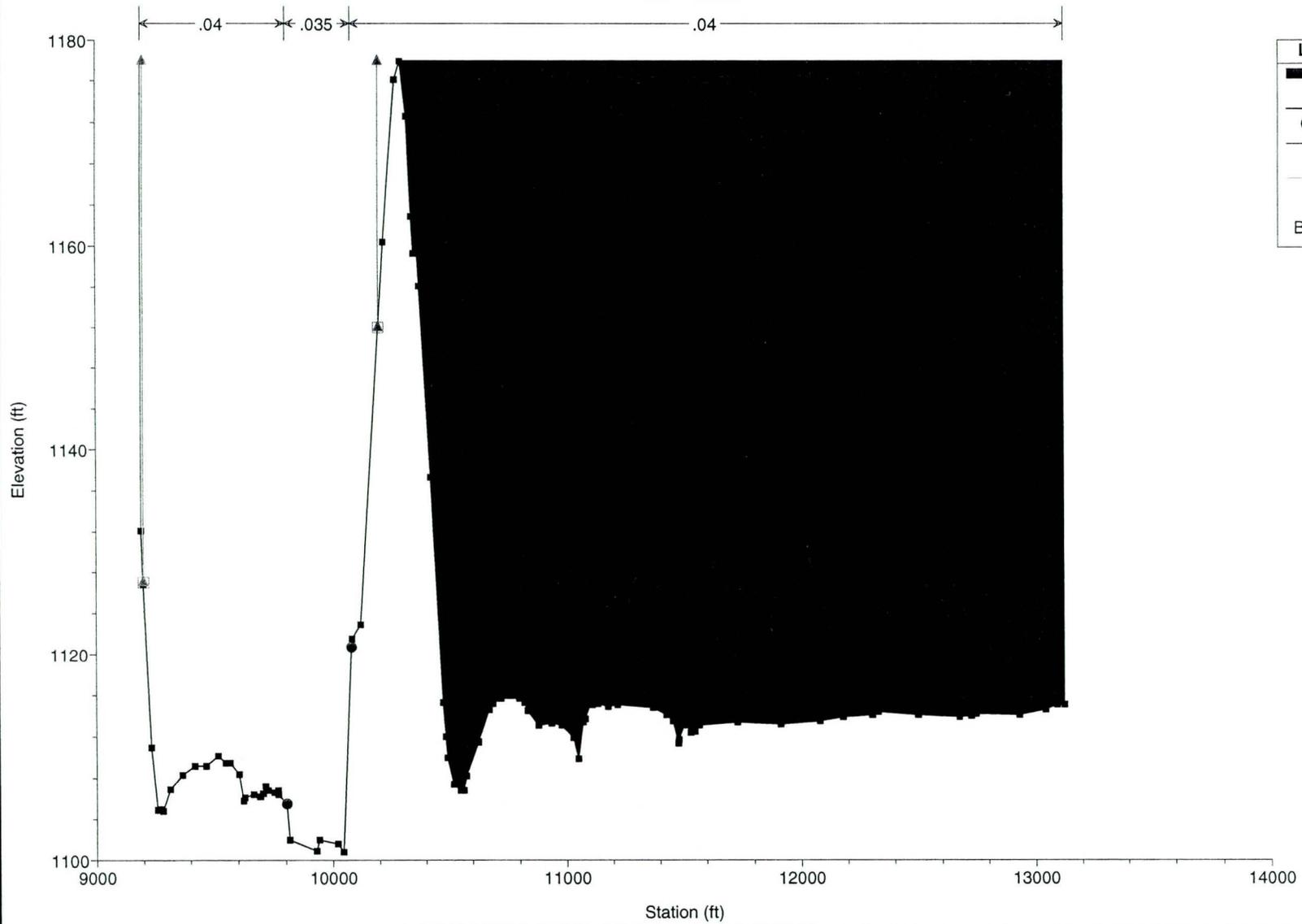
RS = 16.482



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

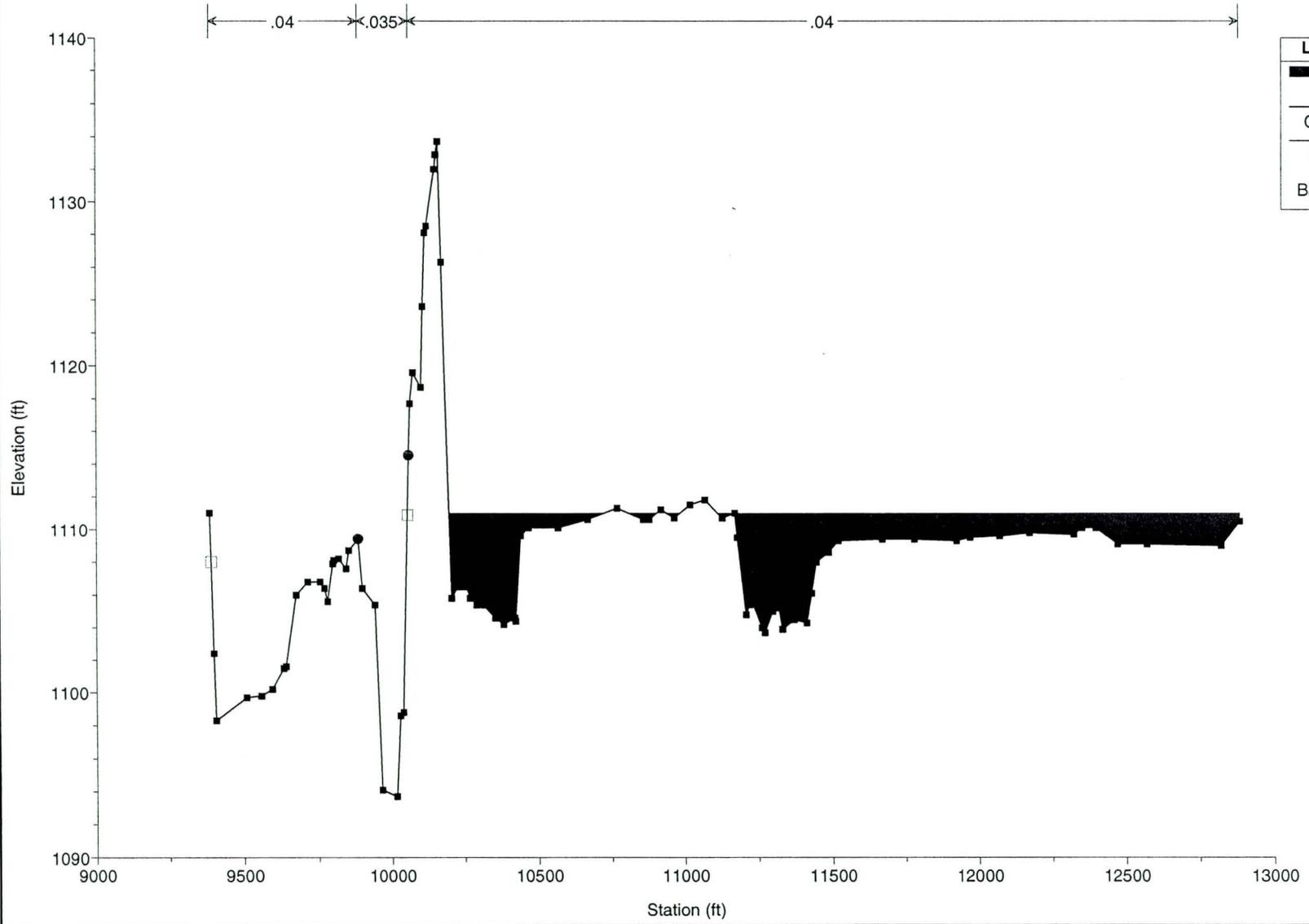
RS = 16.099



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

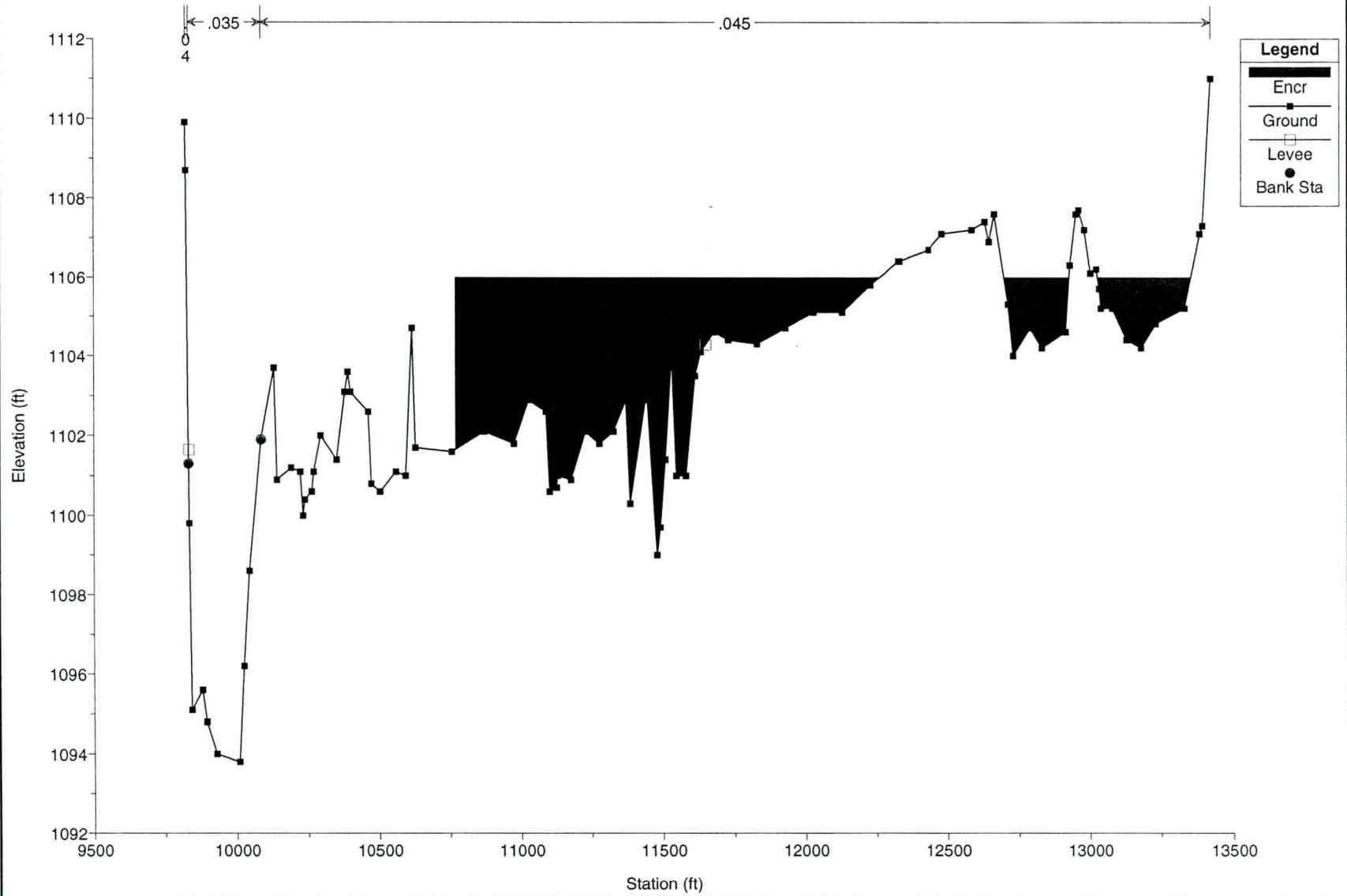
RS = 15.719



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

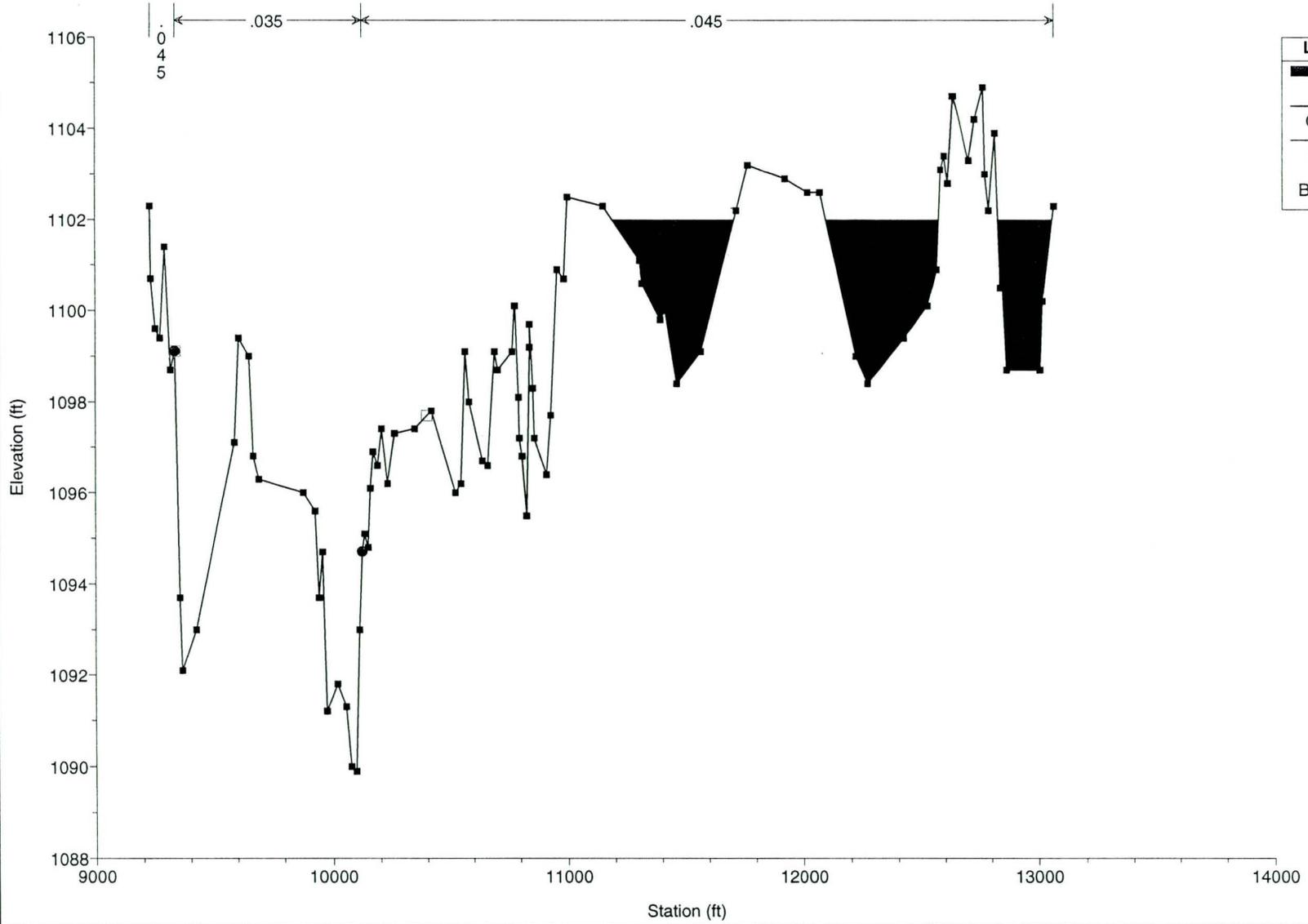
RS = 15.374



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

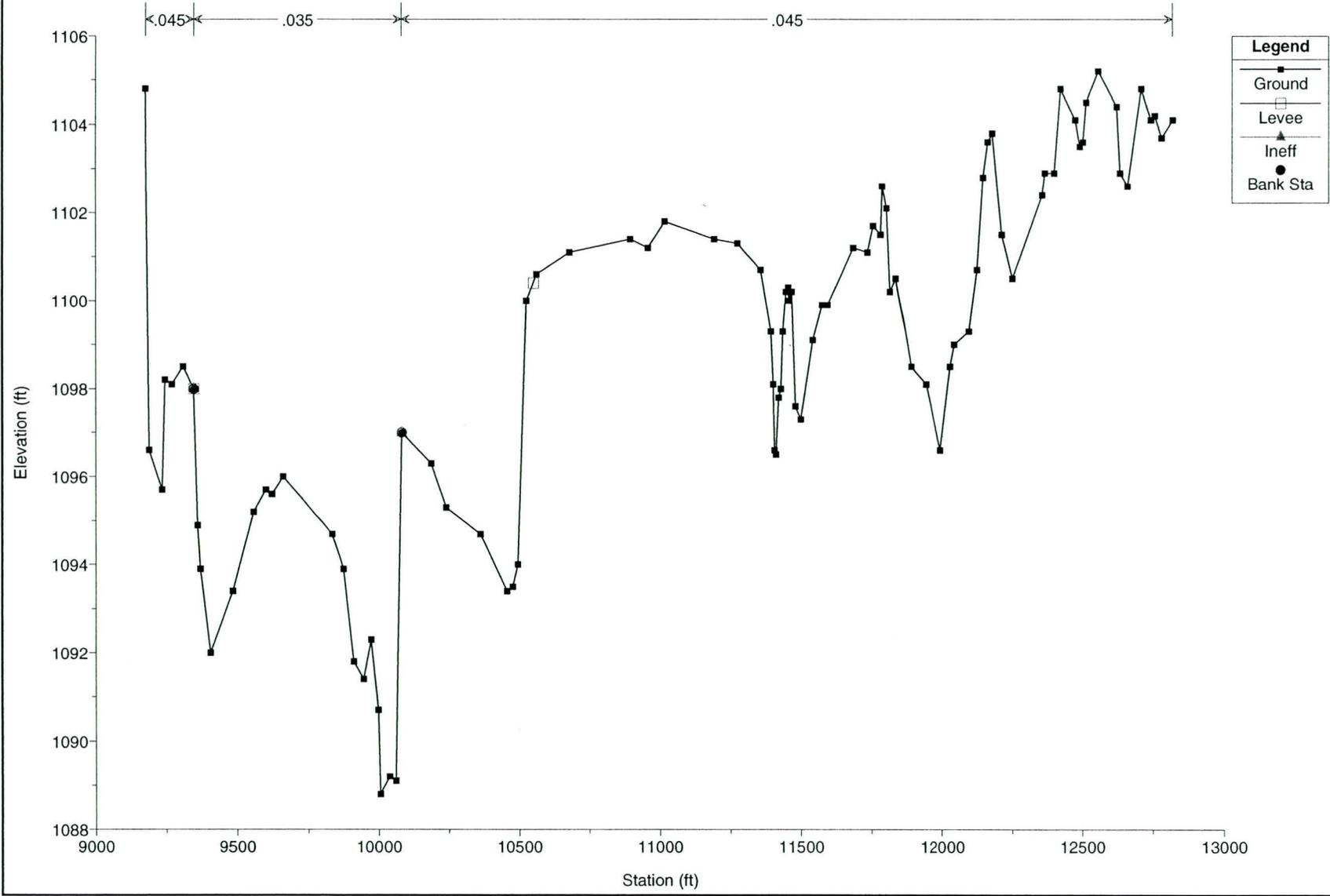
RS = 15.094



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

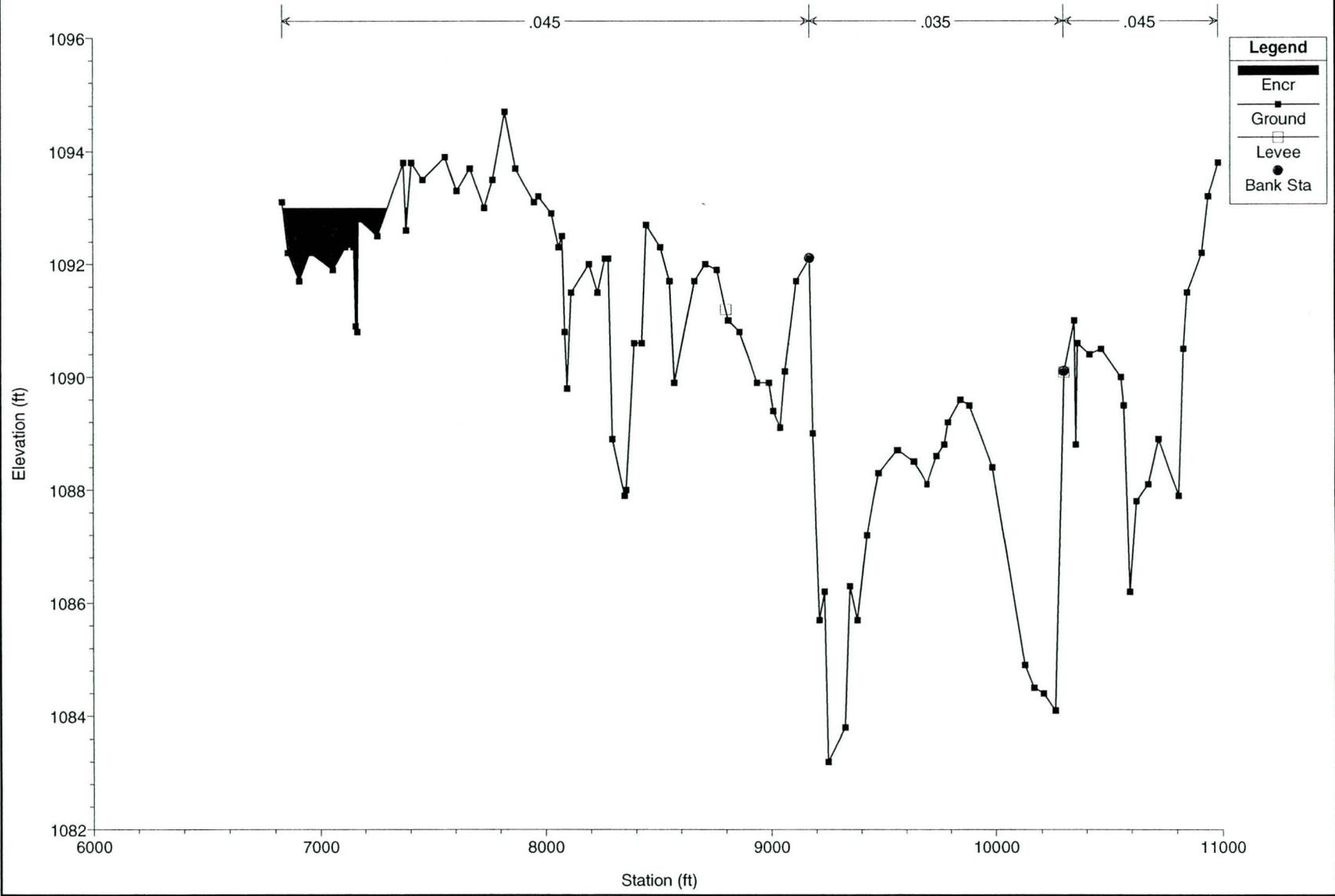
RS = 15



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

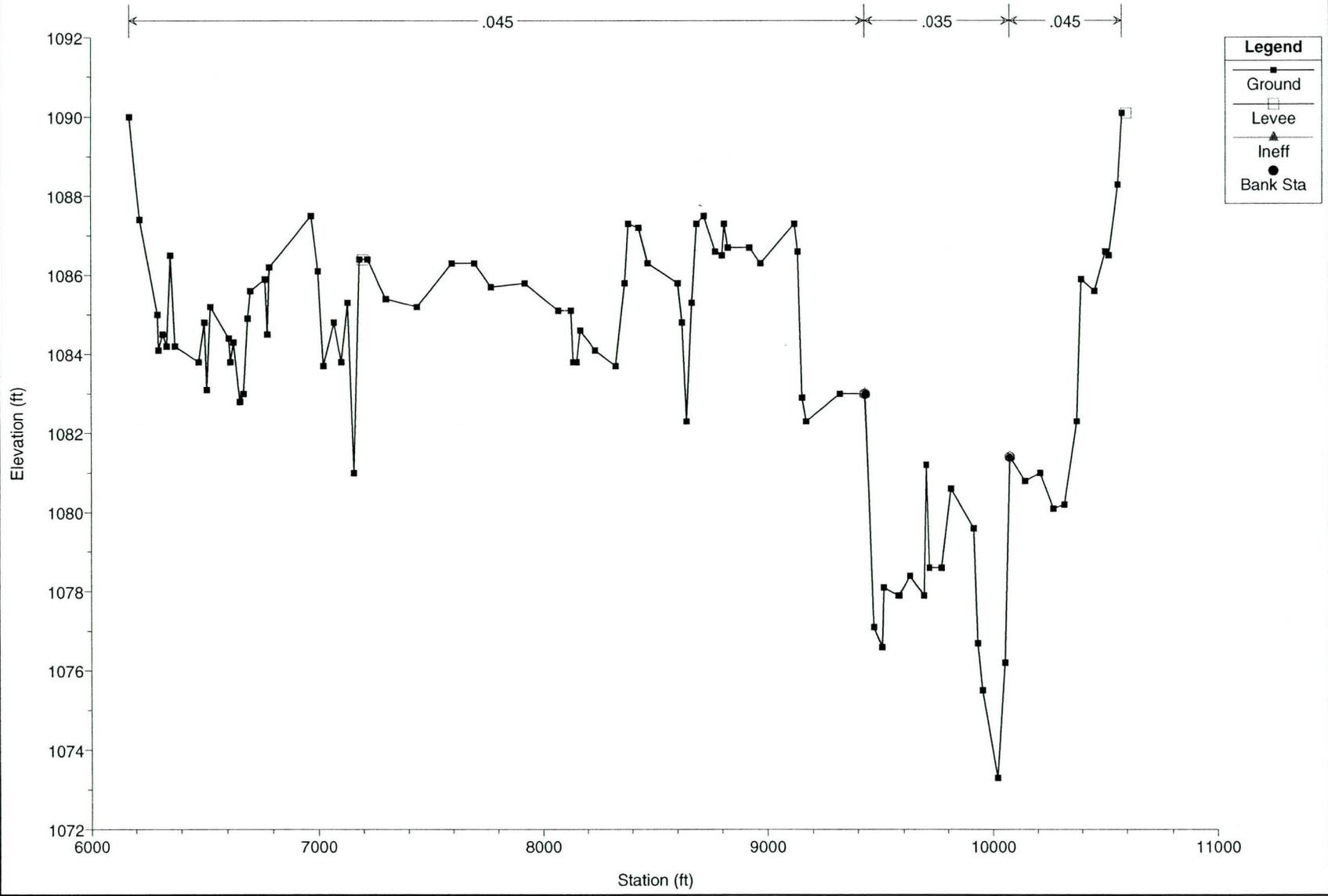
RS = 14.43



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

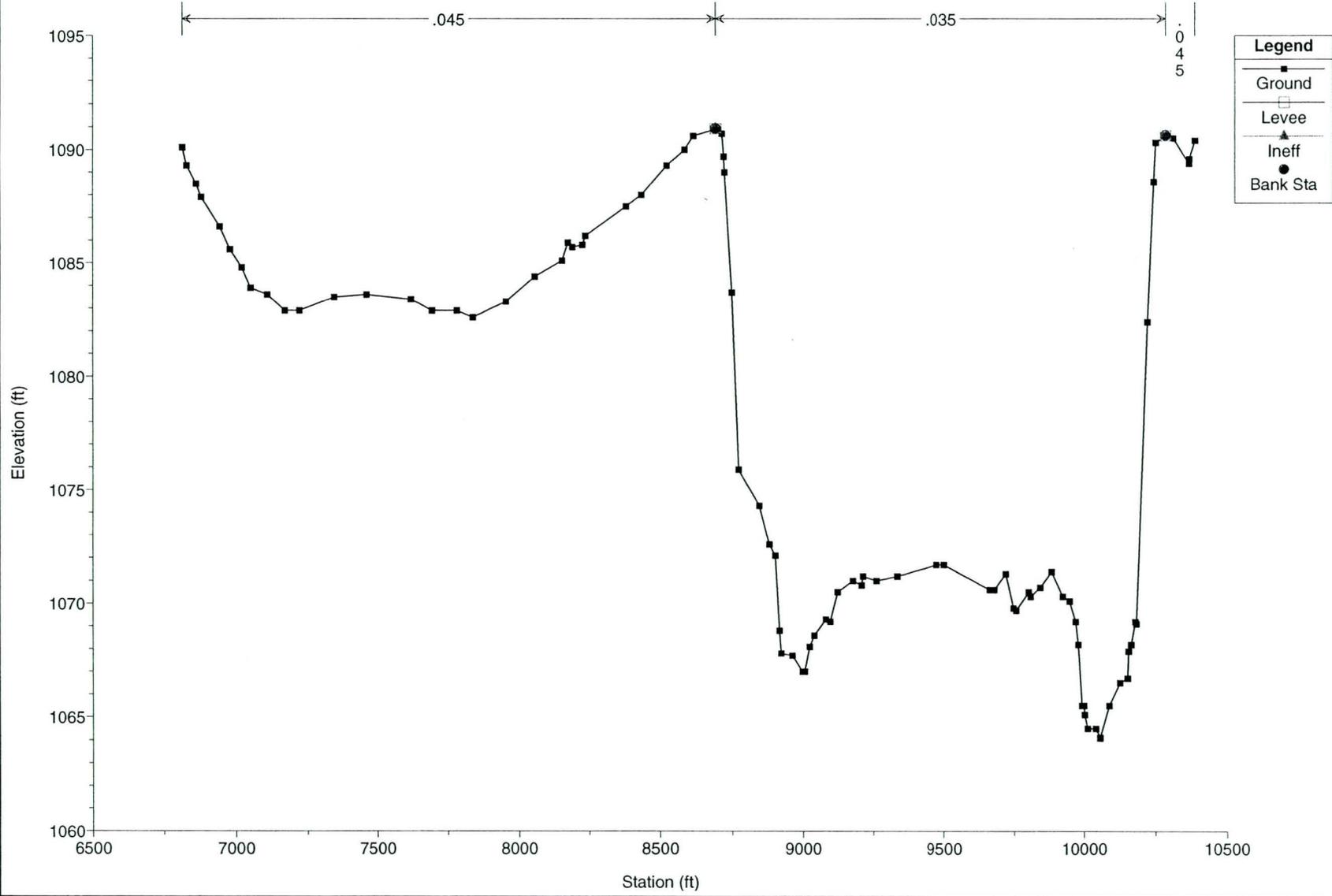
RS = 13.952



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

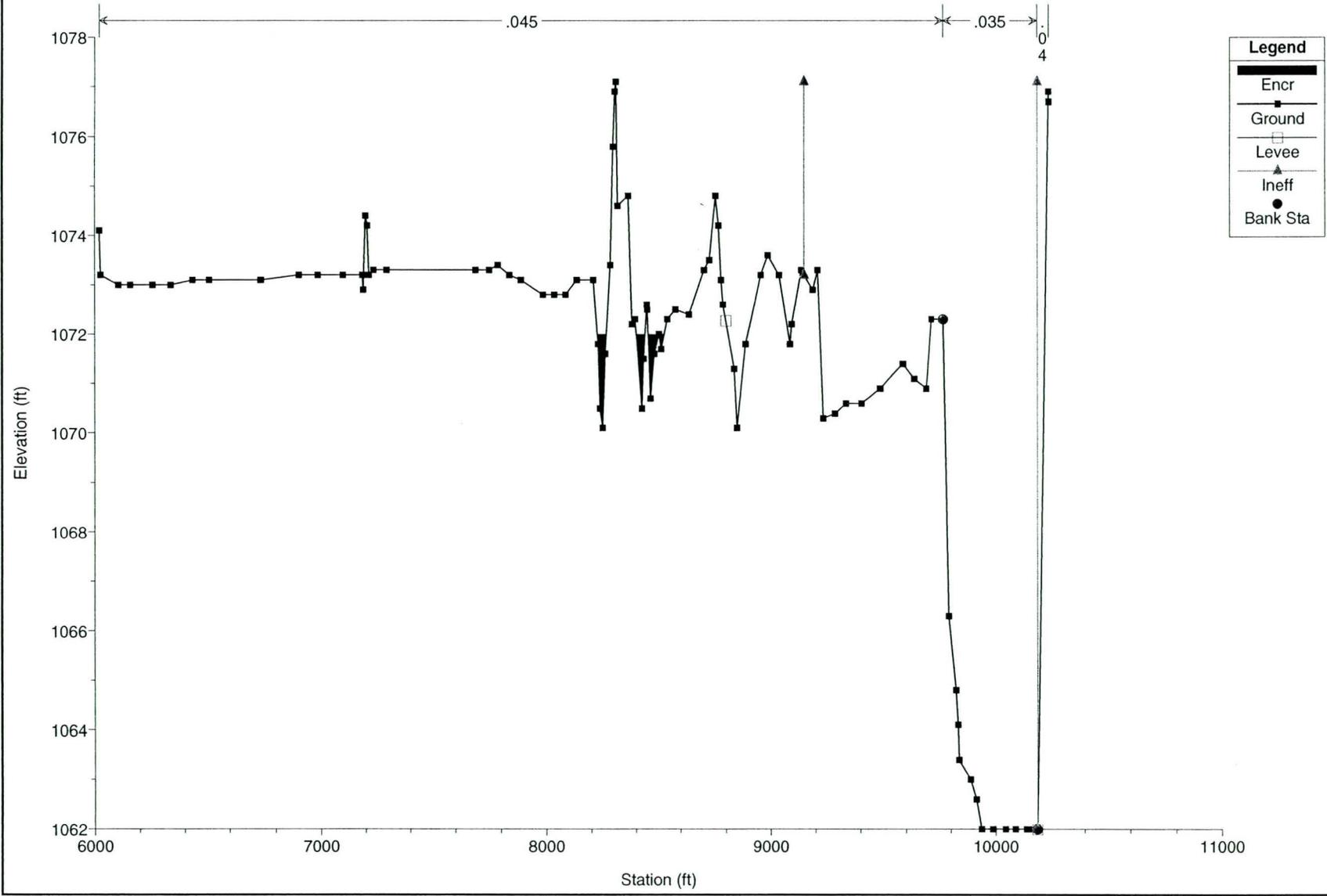
RS = 13.467



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

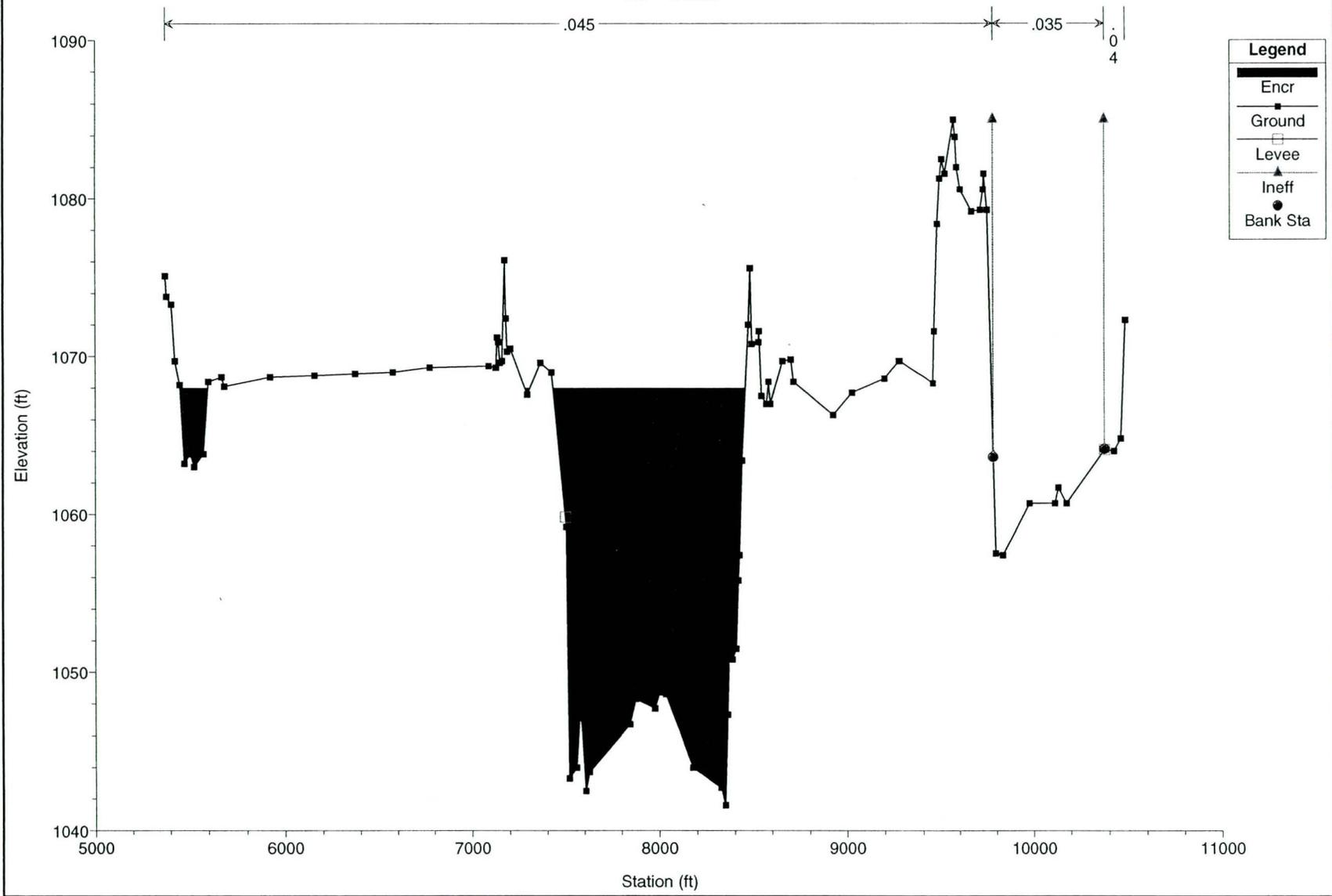
RS = 13.007



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

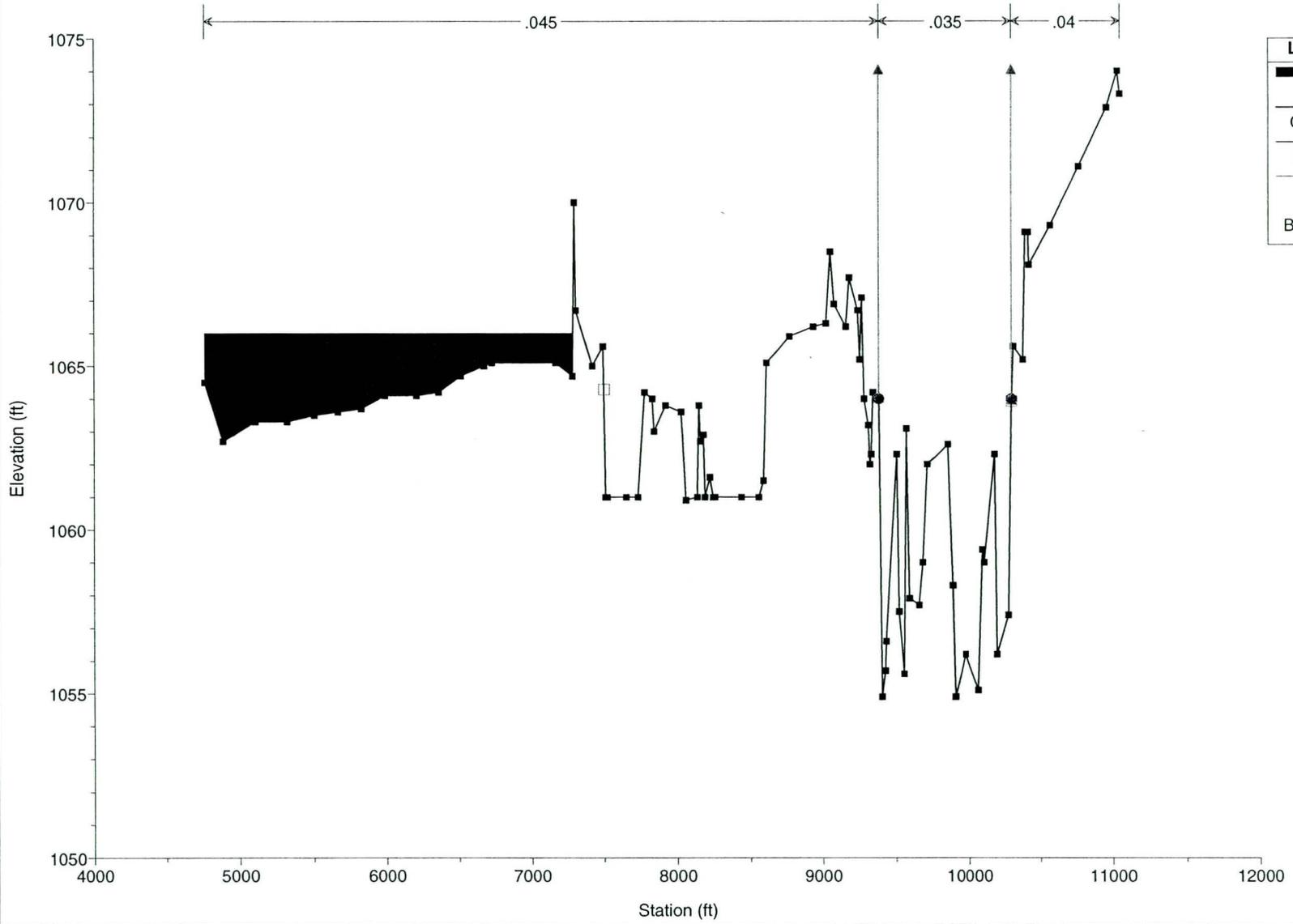
RS = 12.816



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 12.47



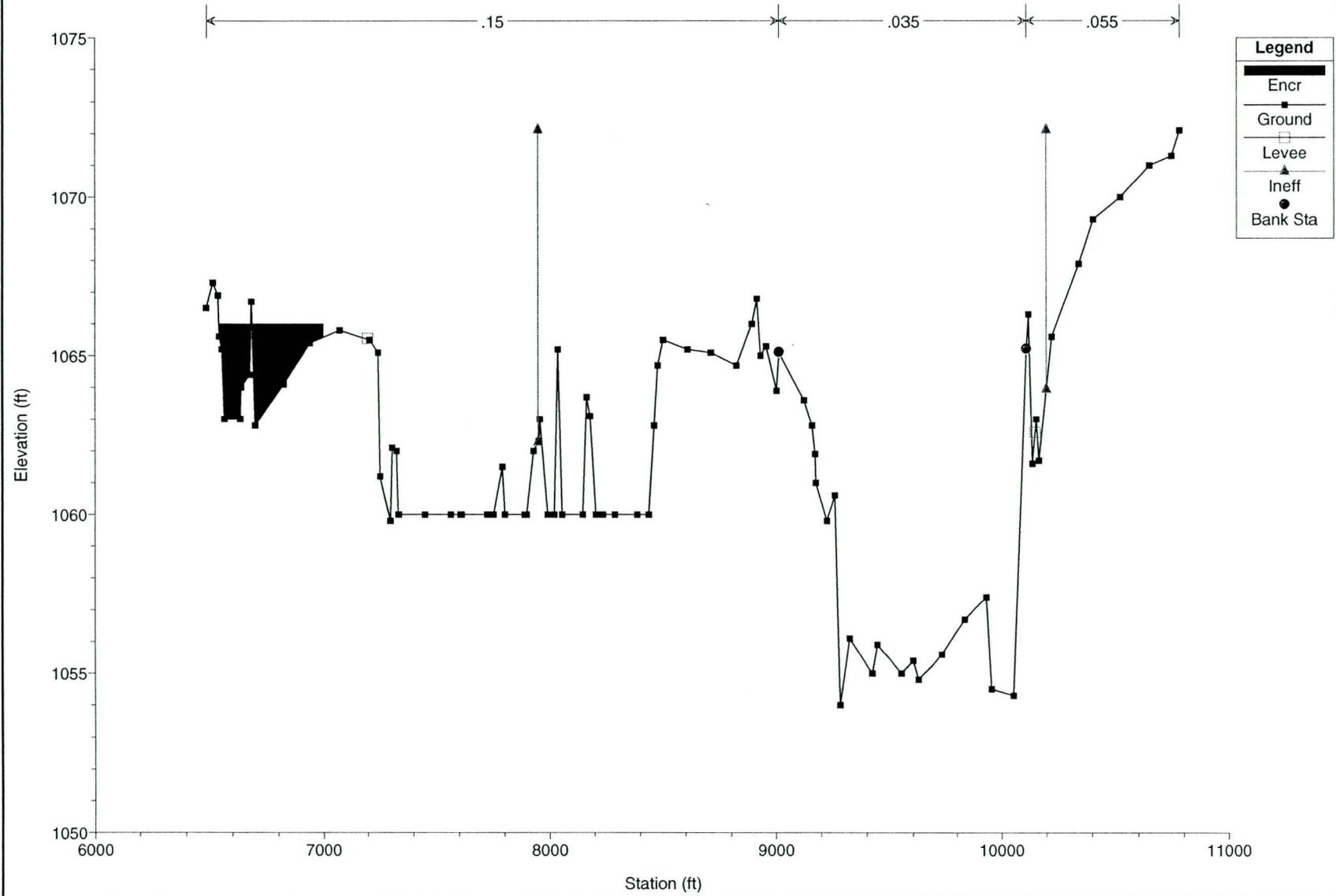
Legend

- Encr
- Ground
- Levee
- Ineff
- Bank Sta

Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

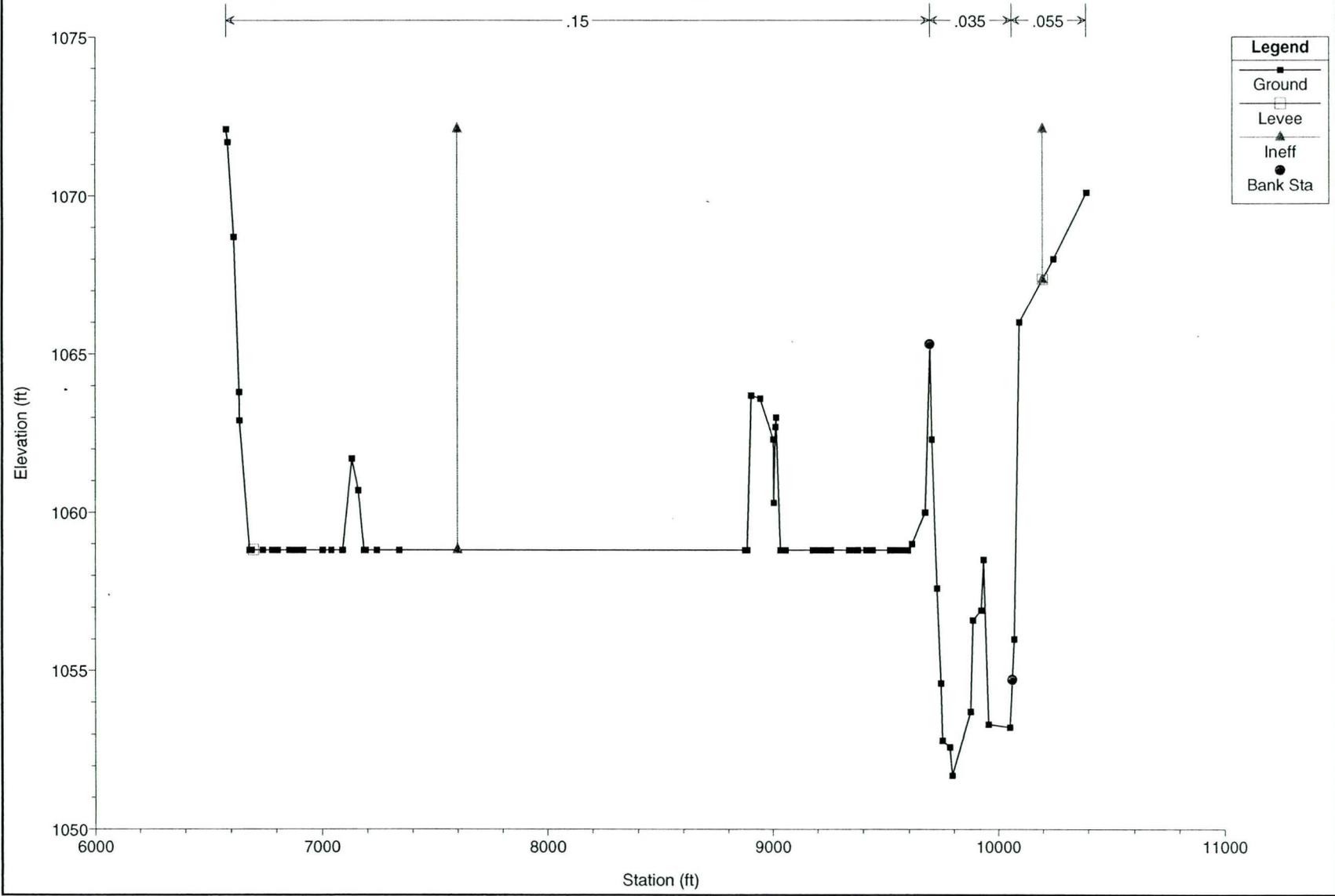
RS = 12.42



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

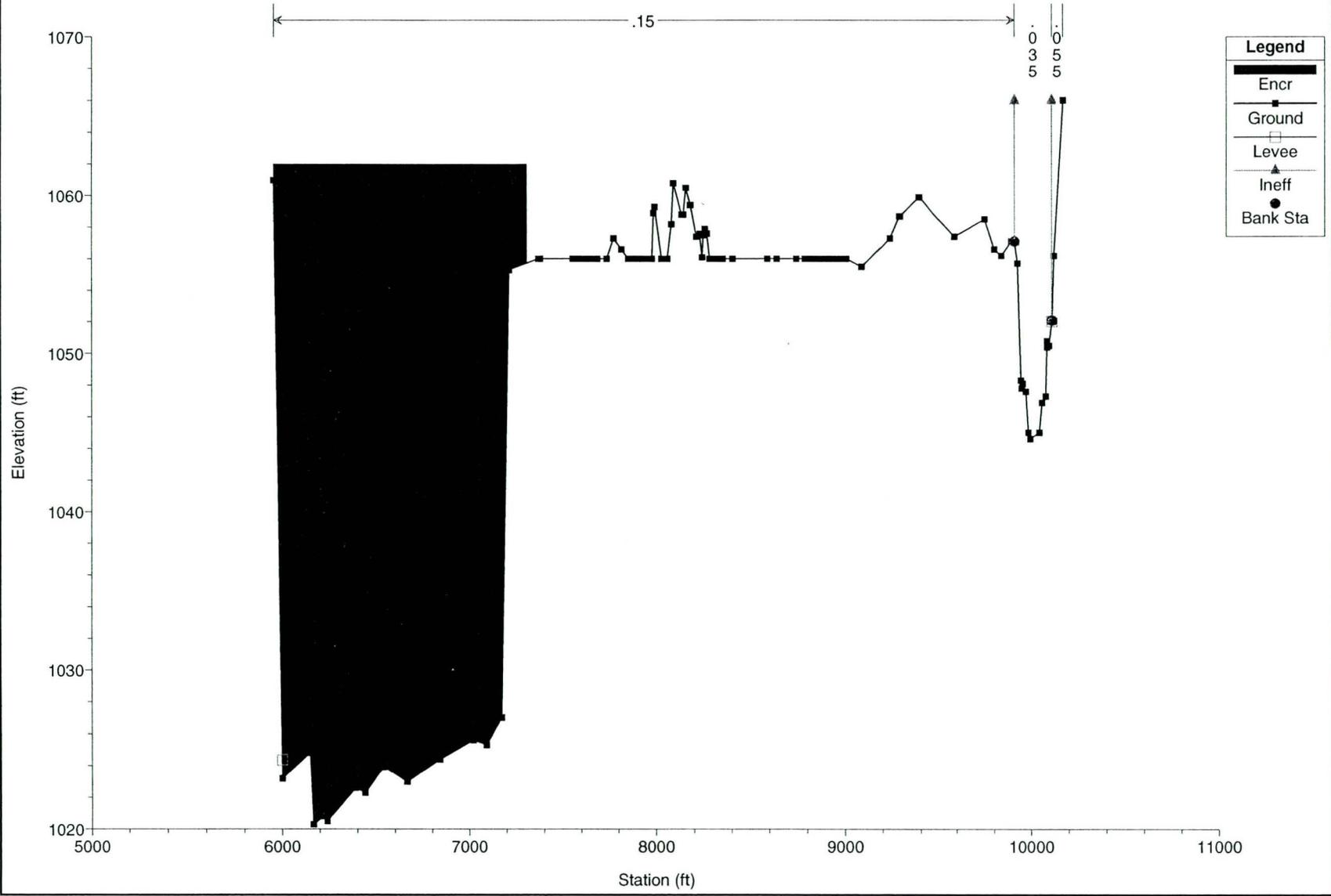
RS = 12.359



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

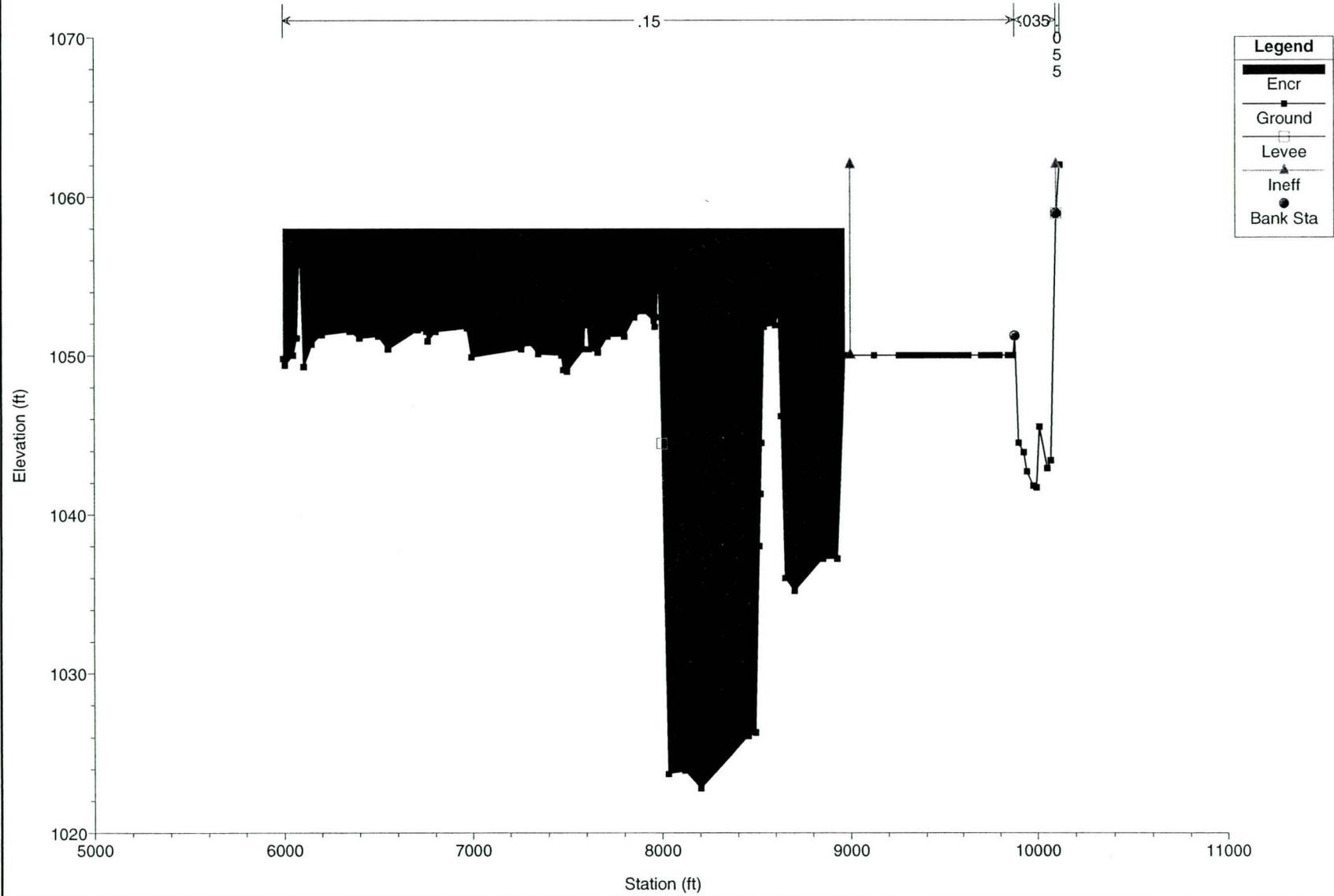
RS = 11.847



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

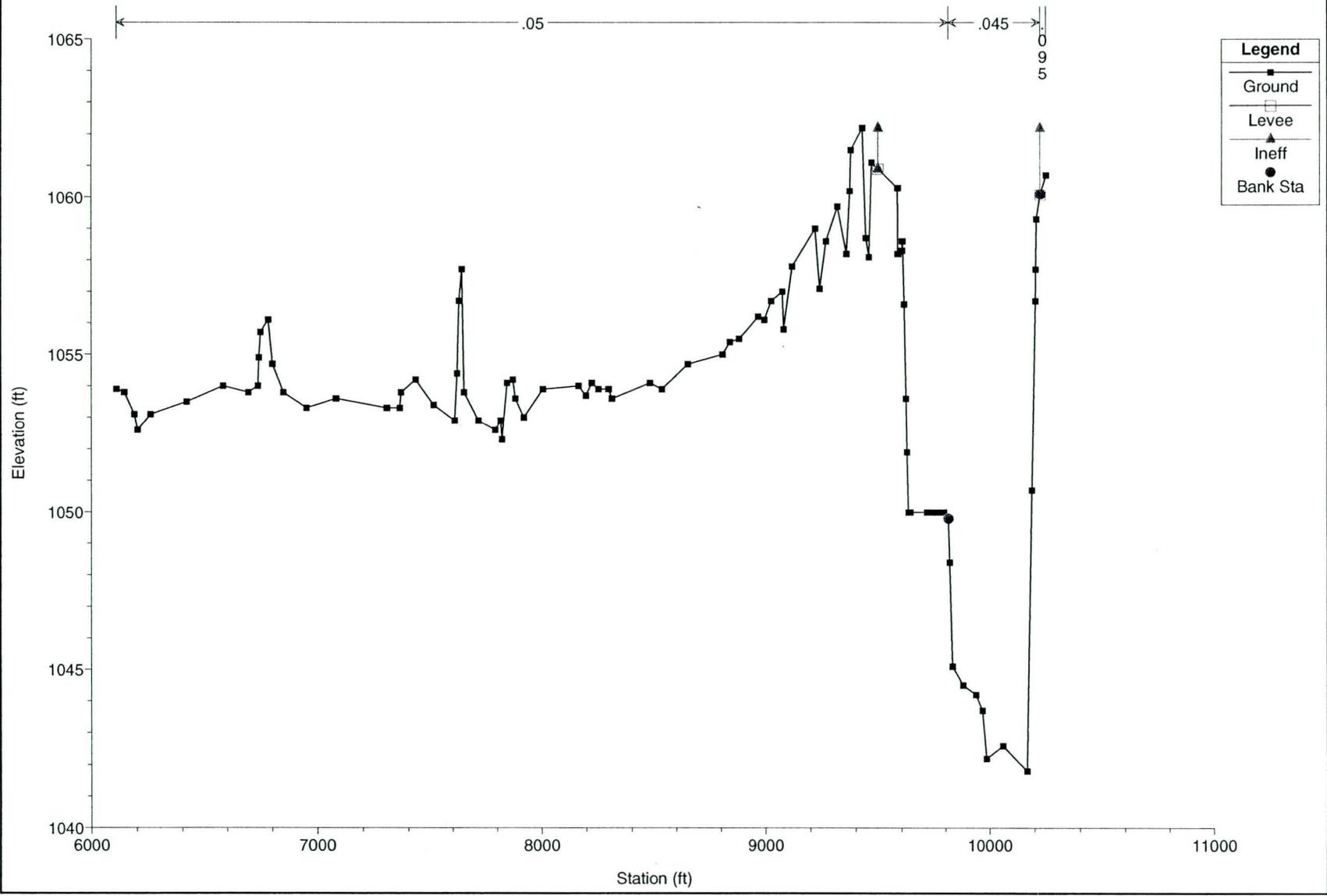
RS = 11.557



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

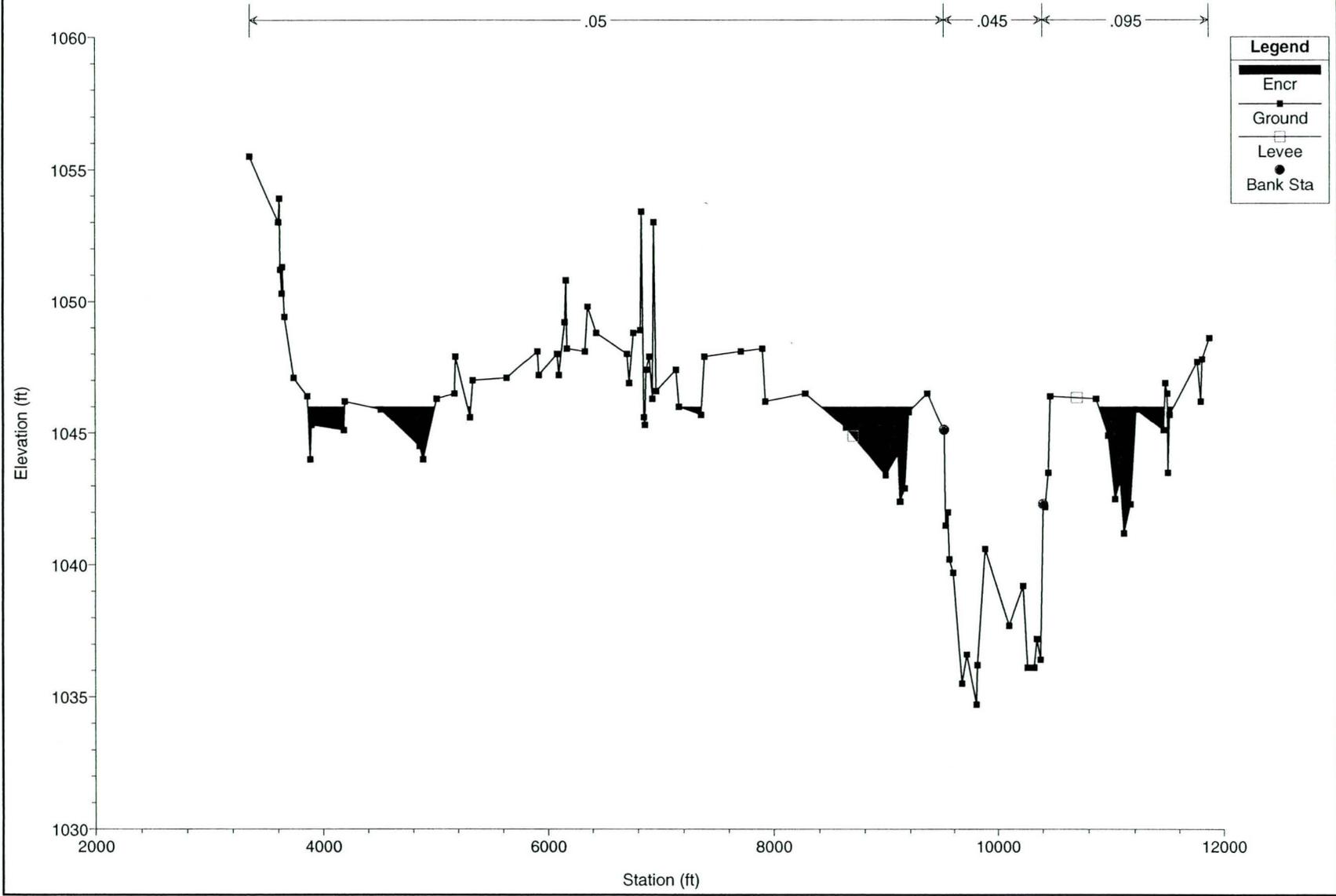
RS = 11.428



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

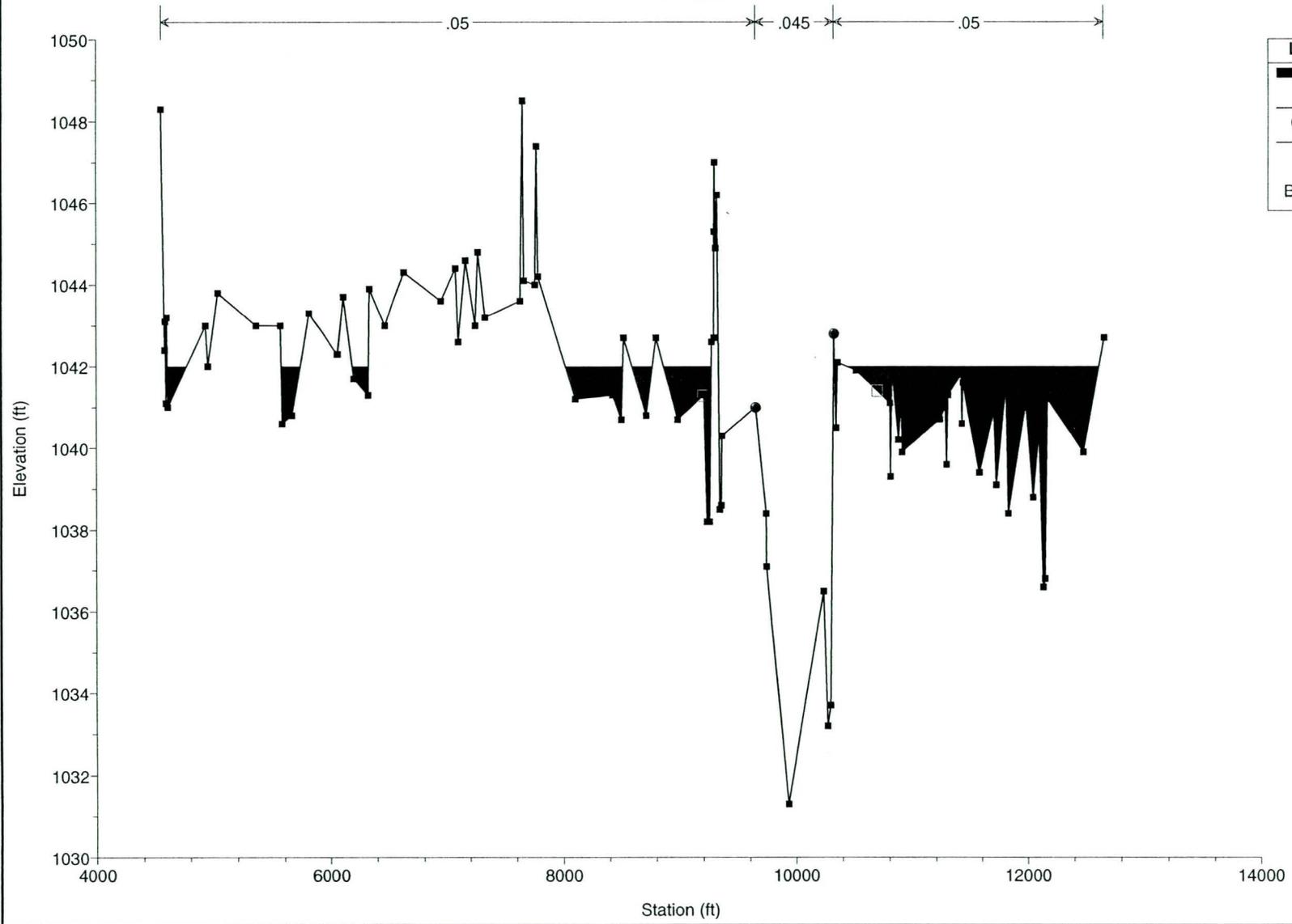
RS = 11.029



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

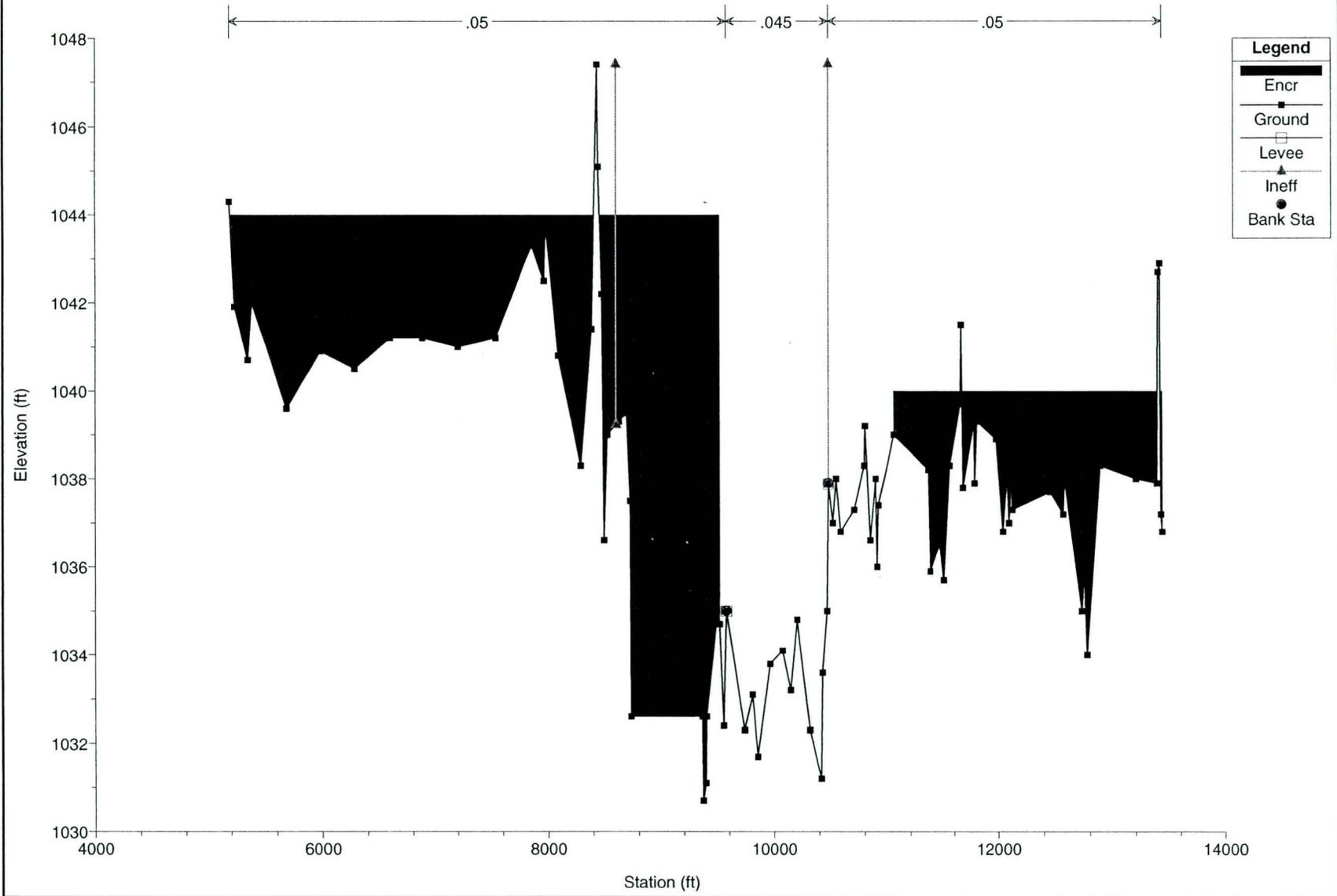
RS = 10.752



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

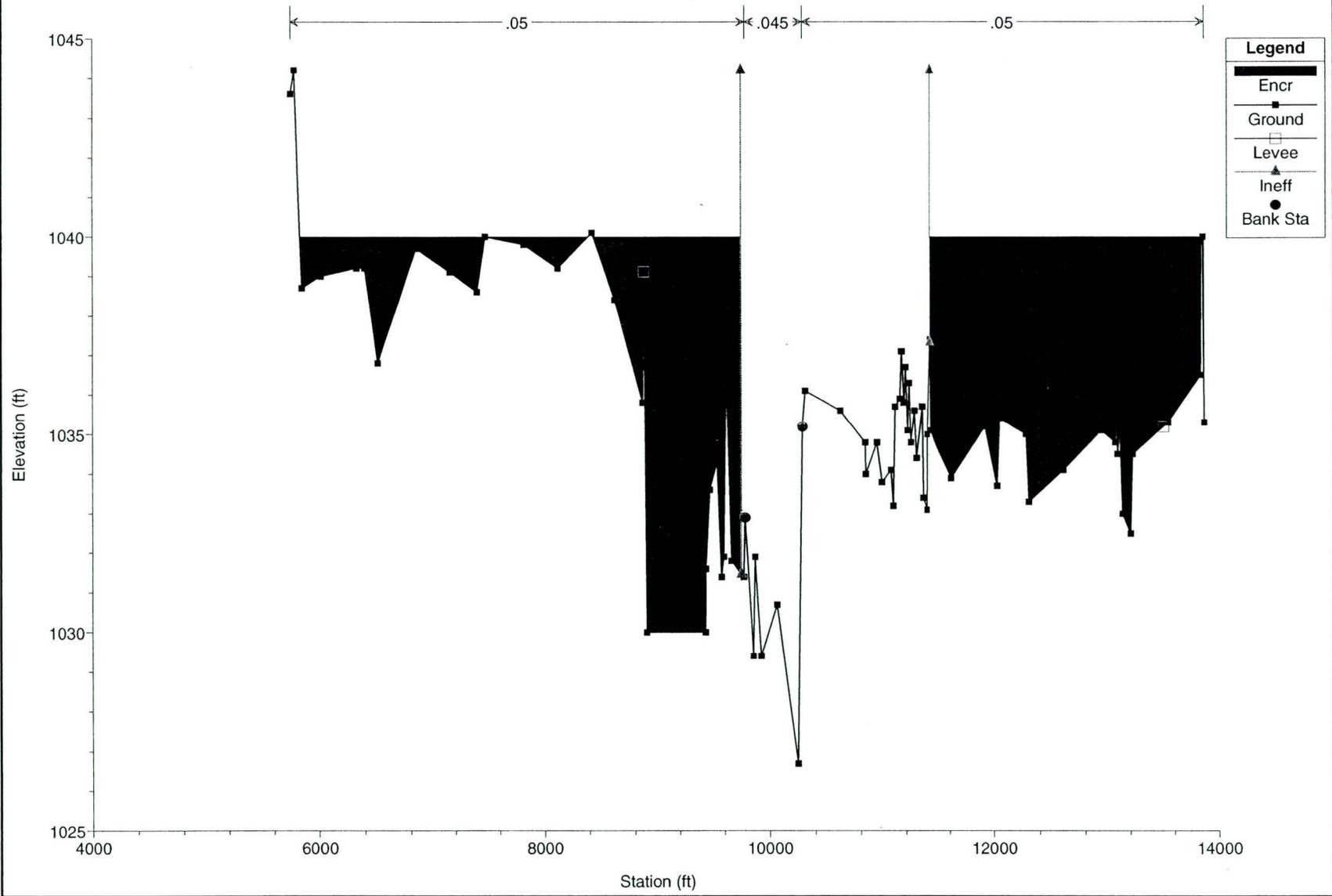
RS = 10.538



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

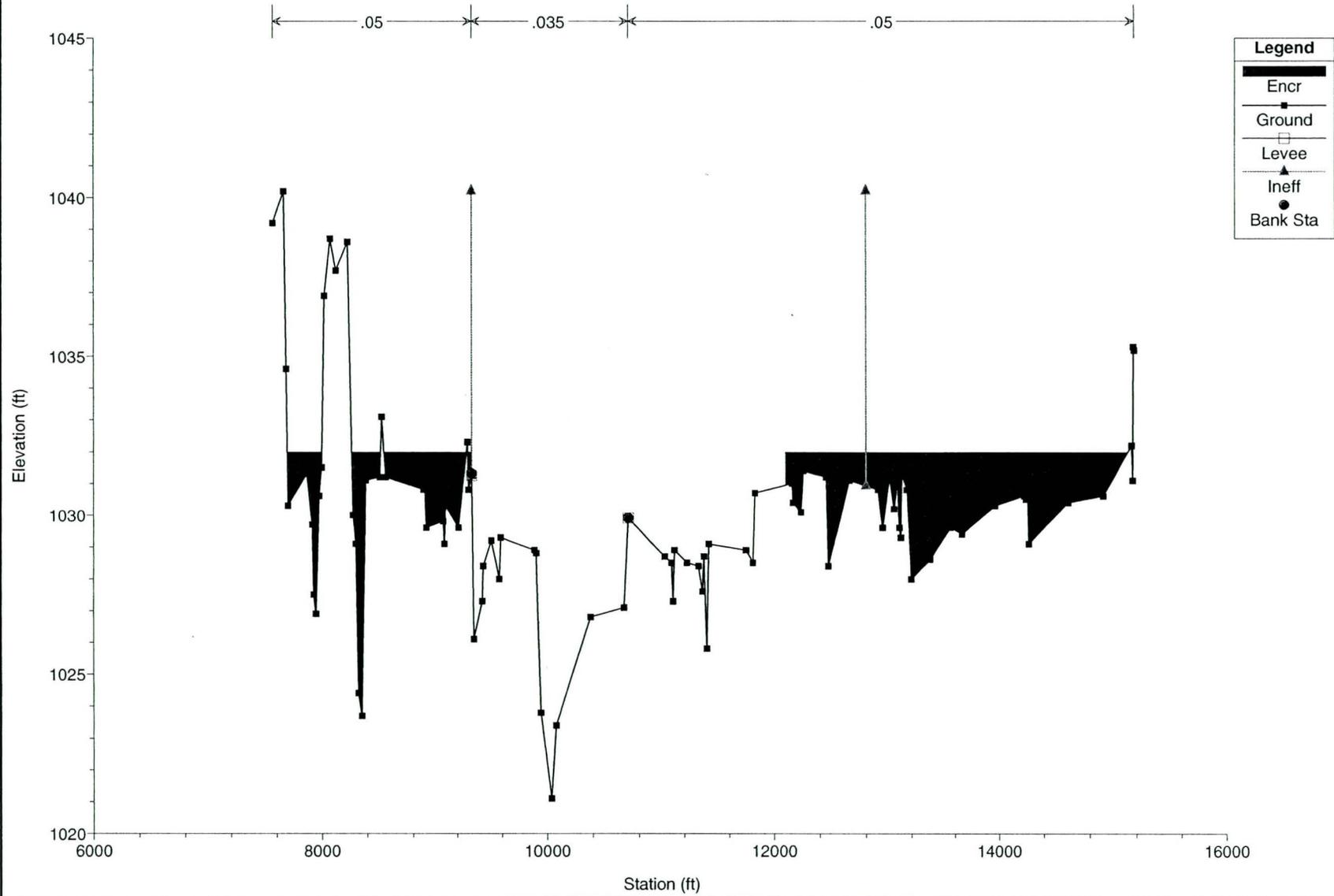
RS = 10.343



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

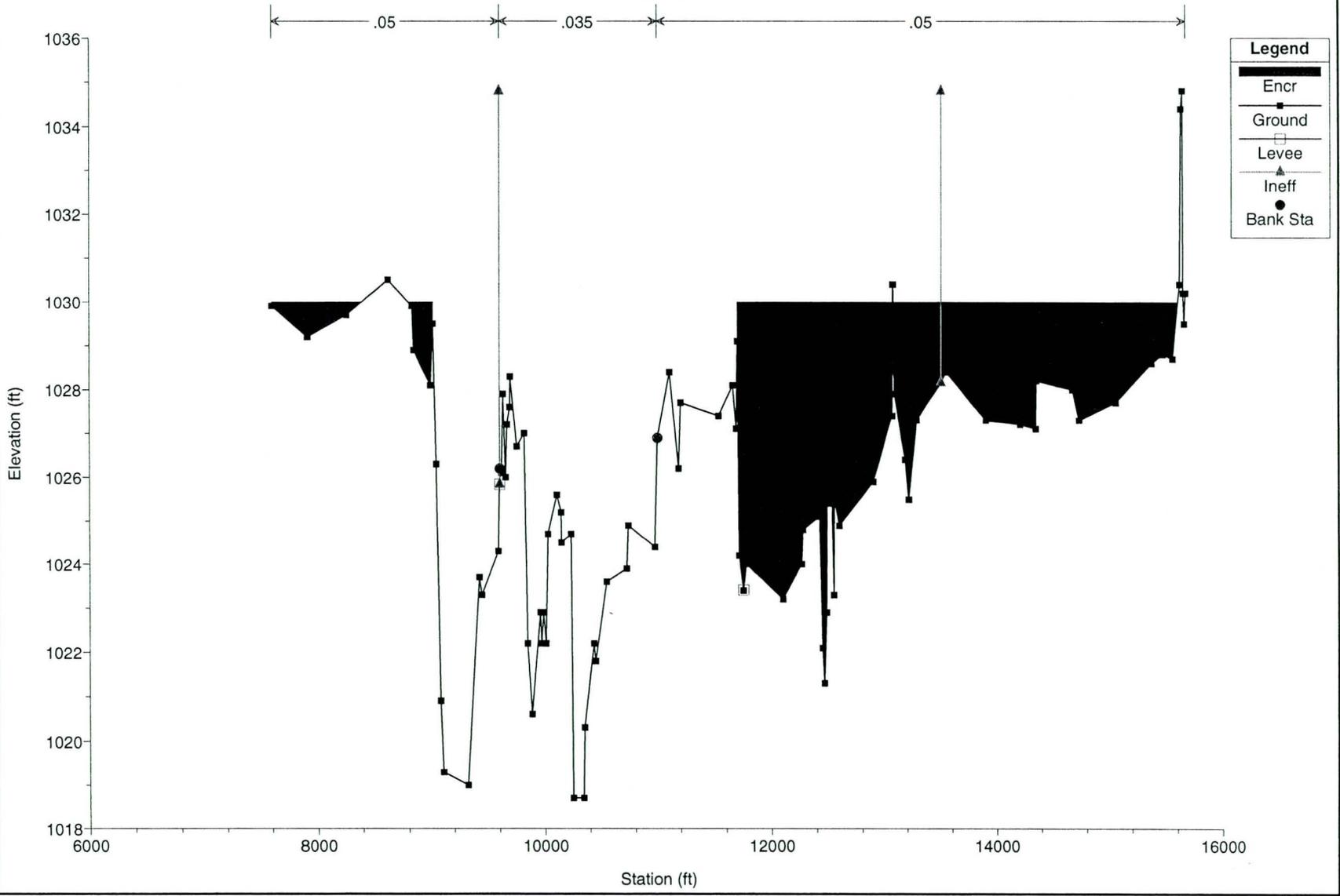
RS = 9.885



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

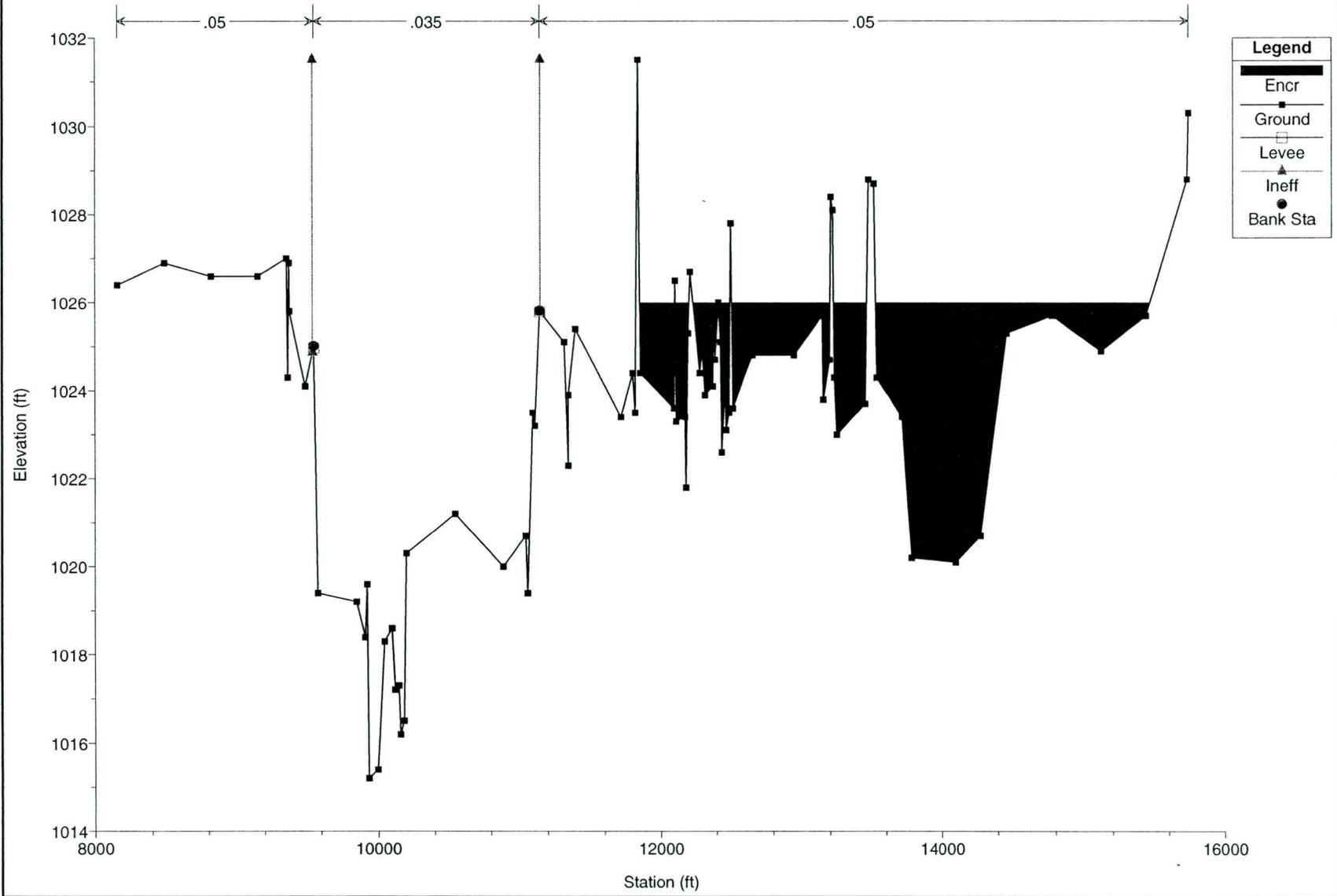
RS = 9.696



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

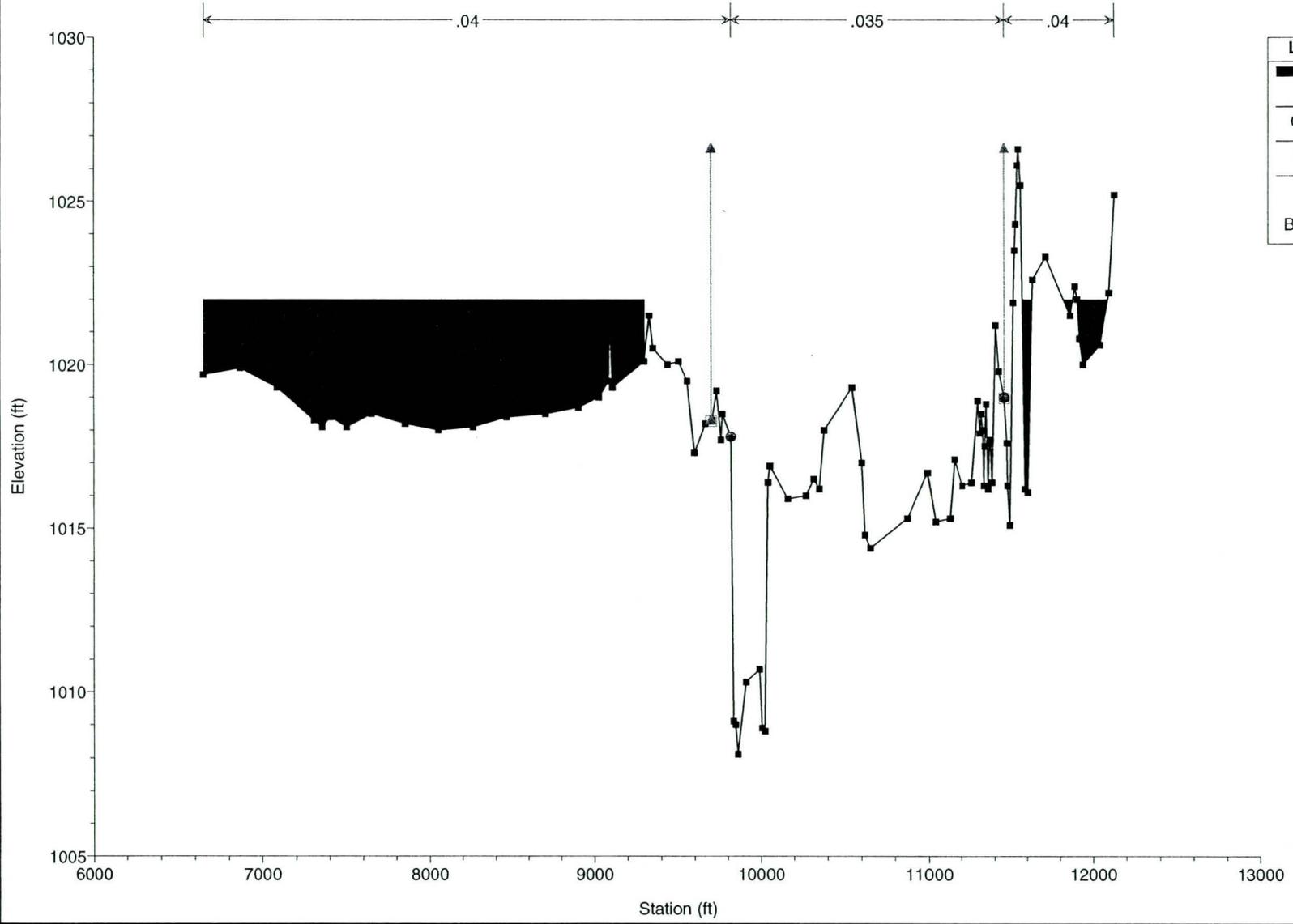
RS = 9.435



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

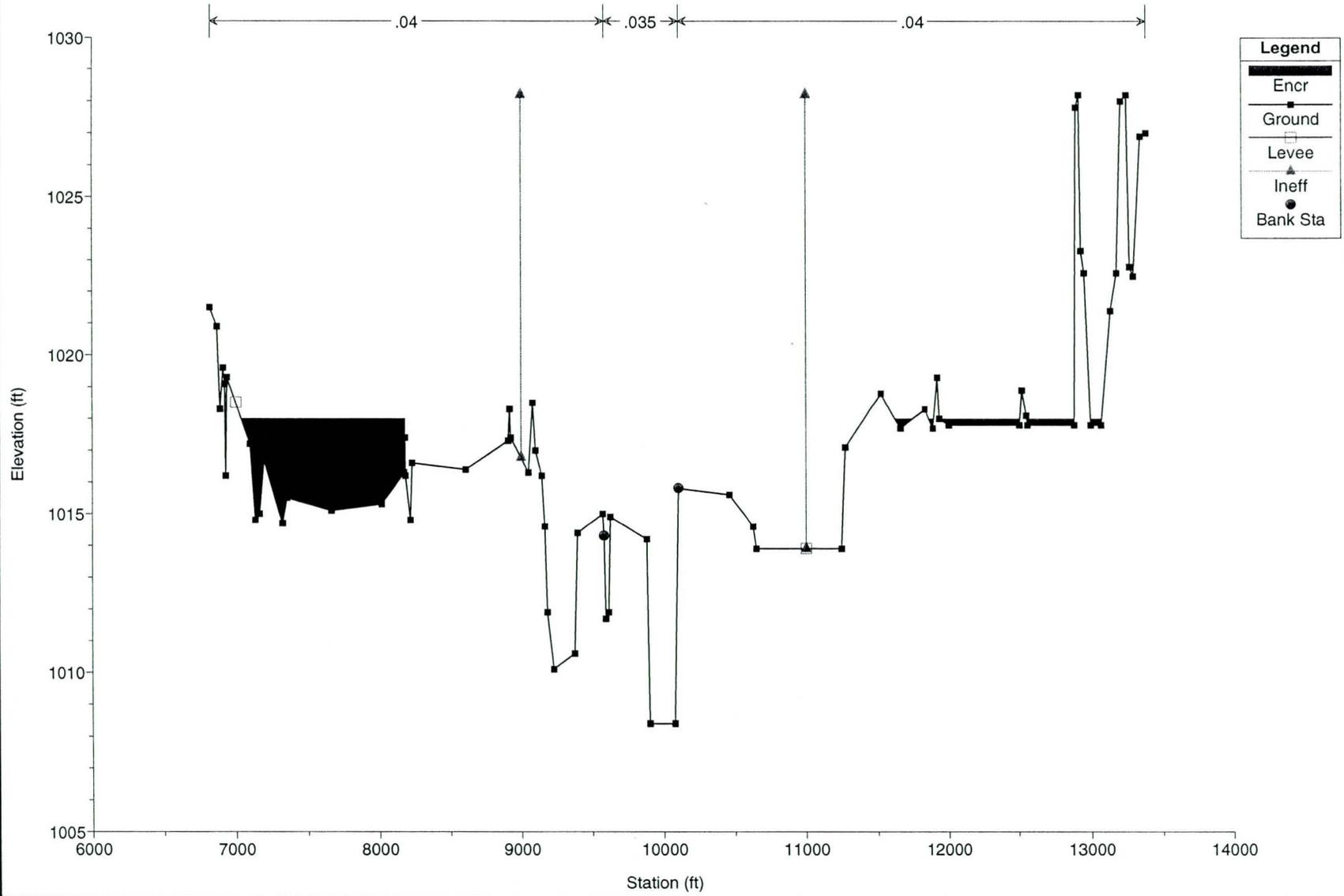
RS = 8.992



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

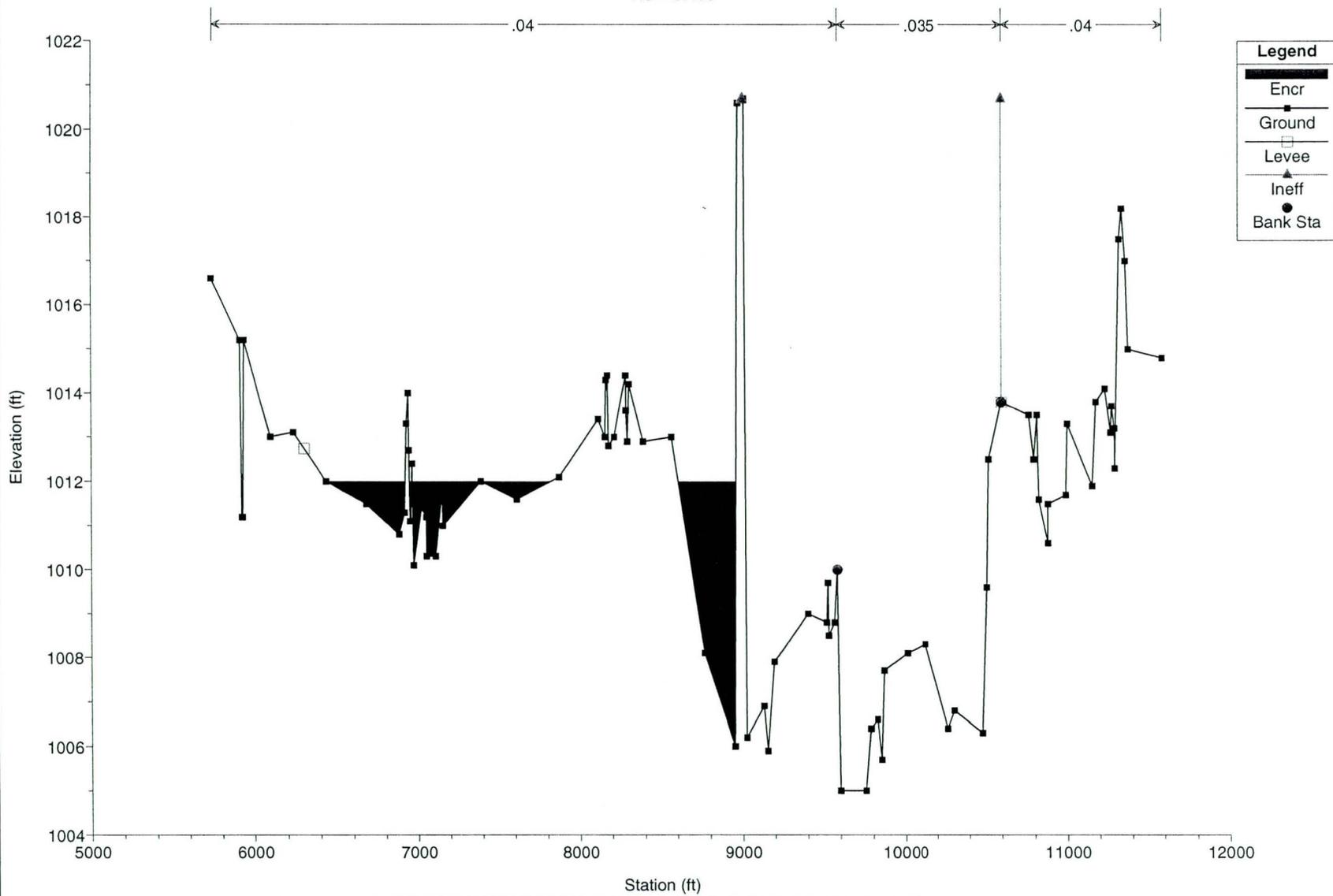
RS = 8.768



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

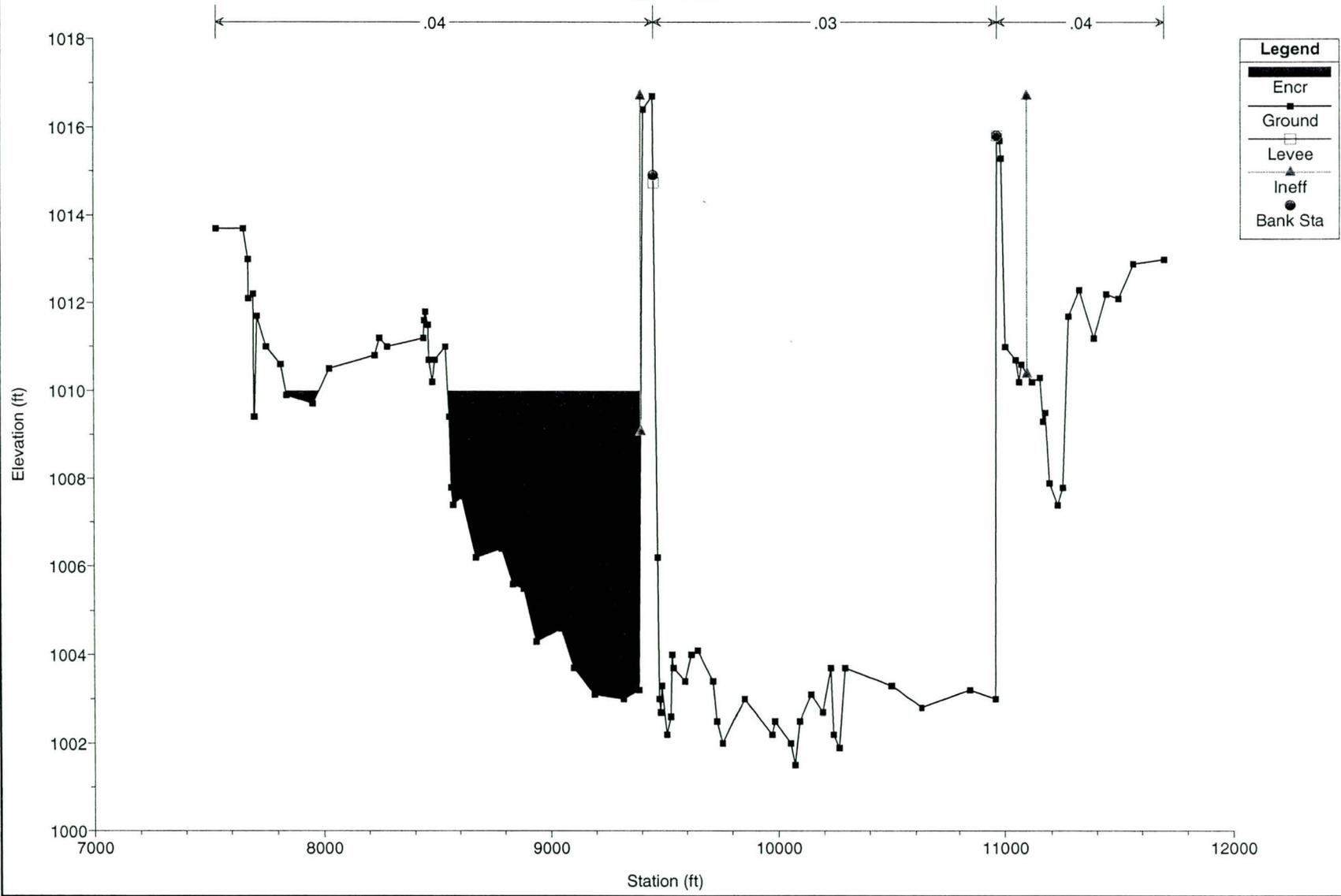
RS = 8.433



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

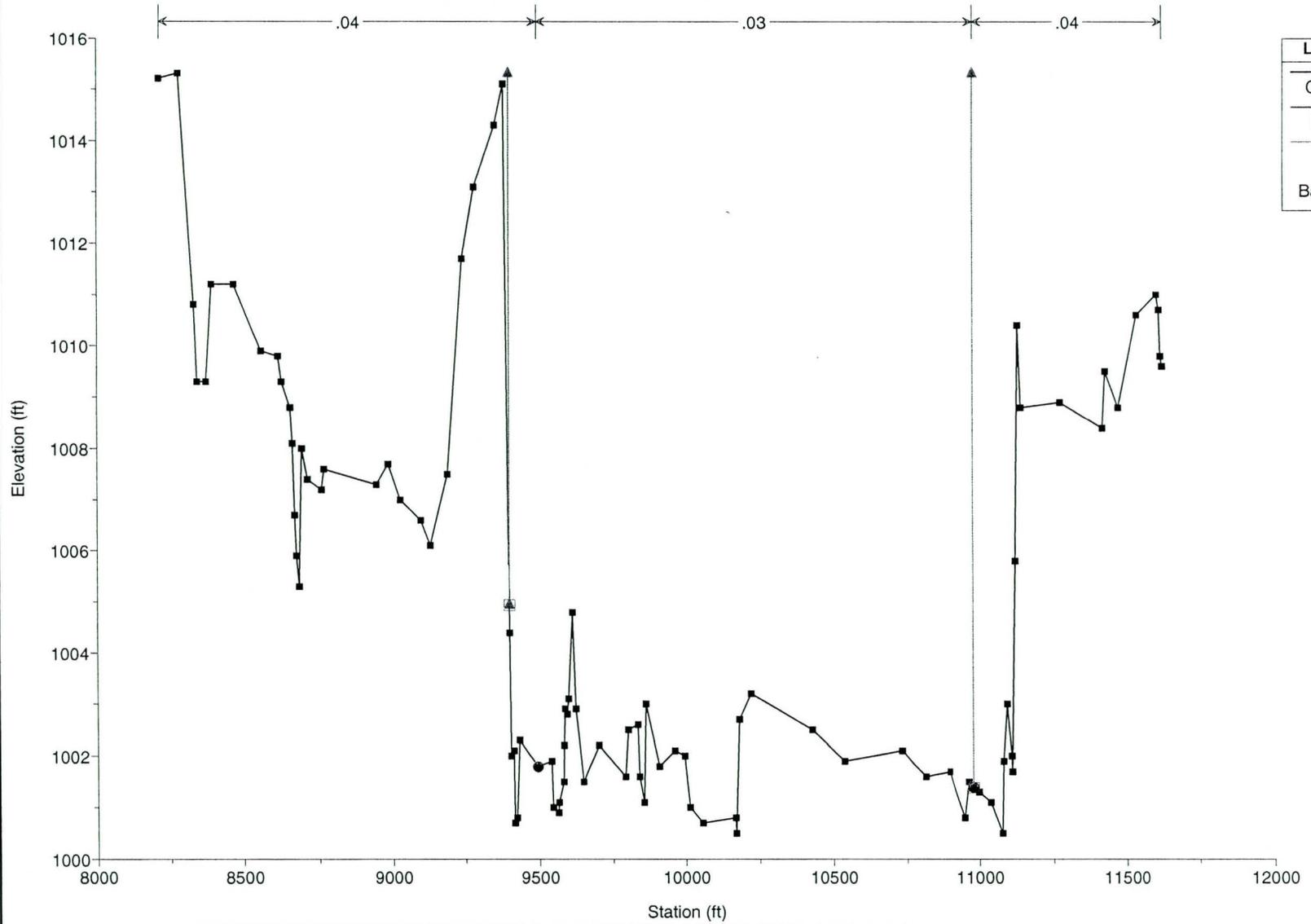
RS = 8.198



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

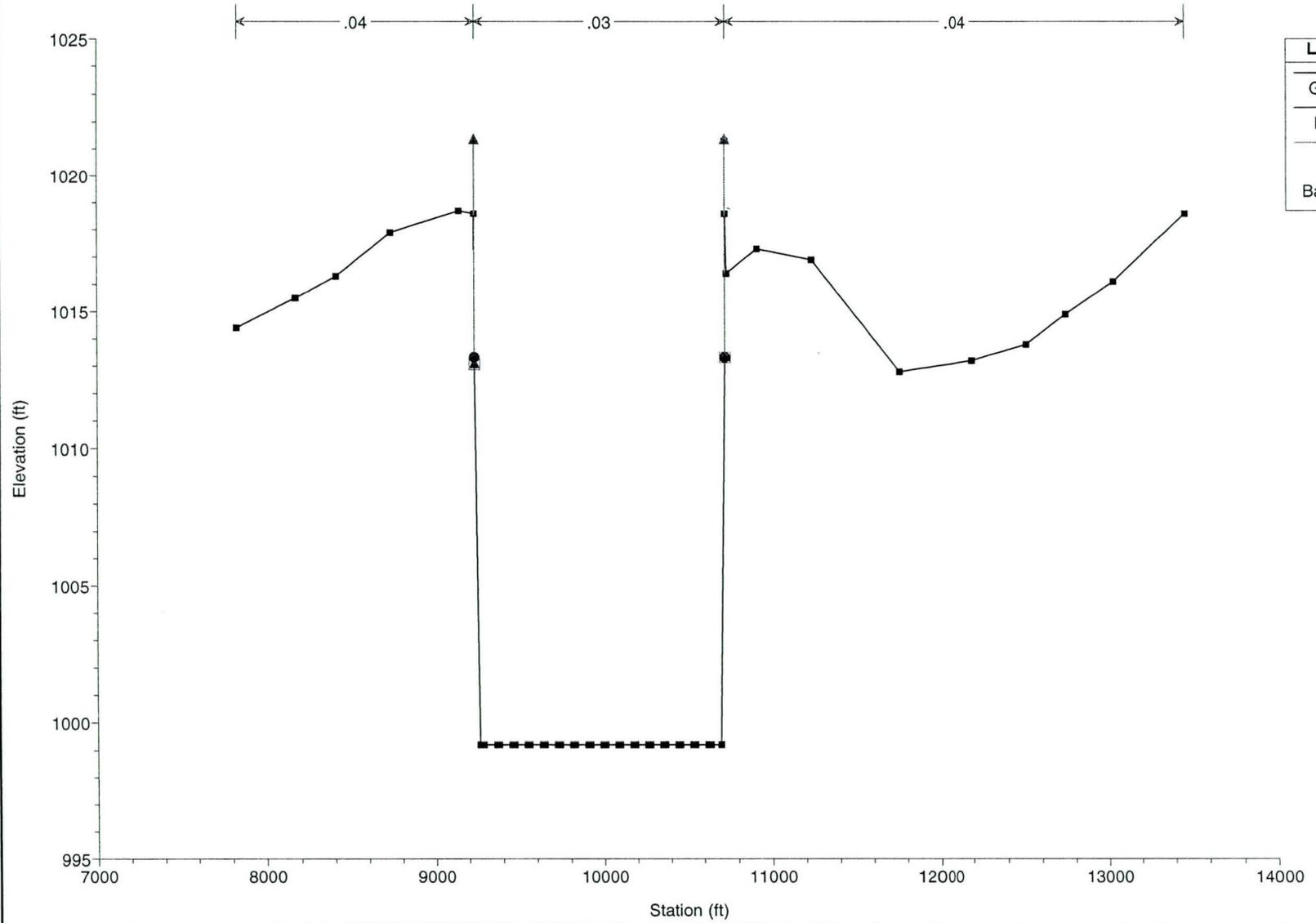
RS = 8.105



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

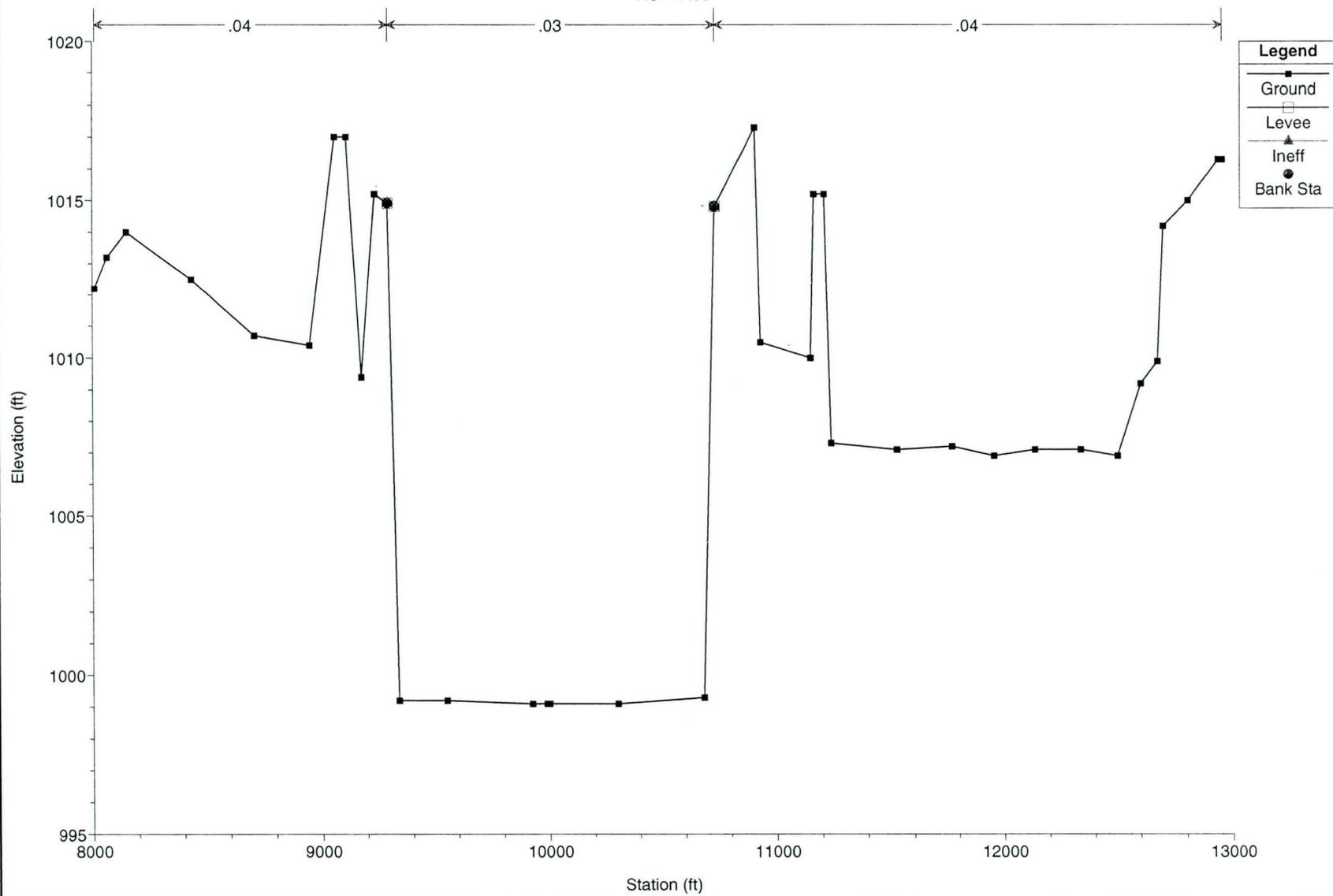
RS = 8



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

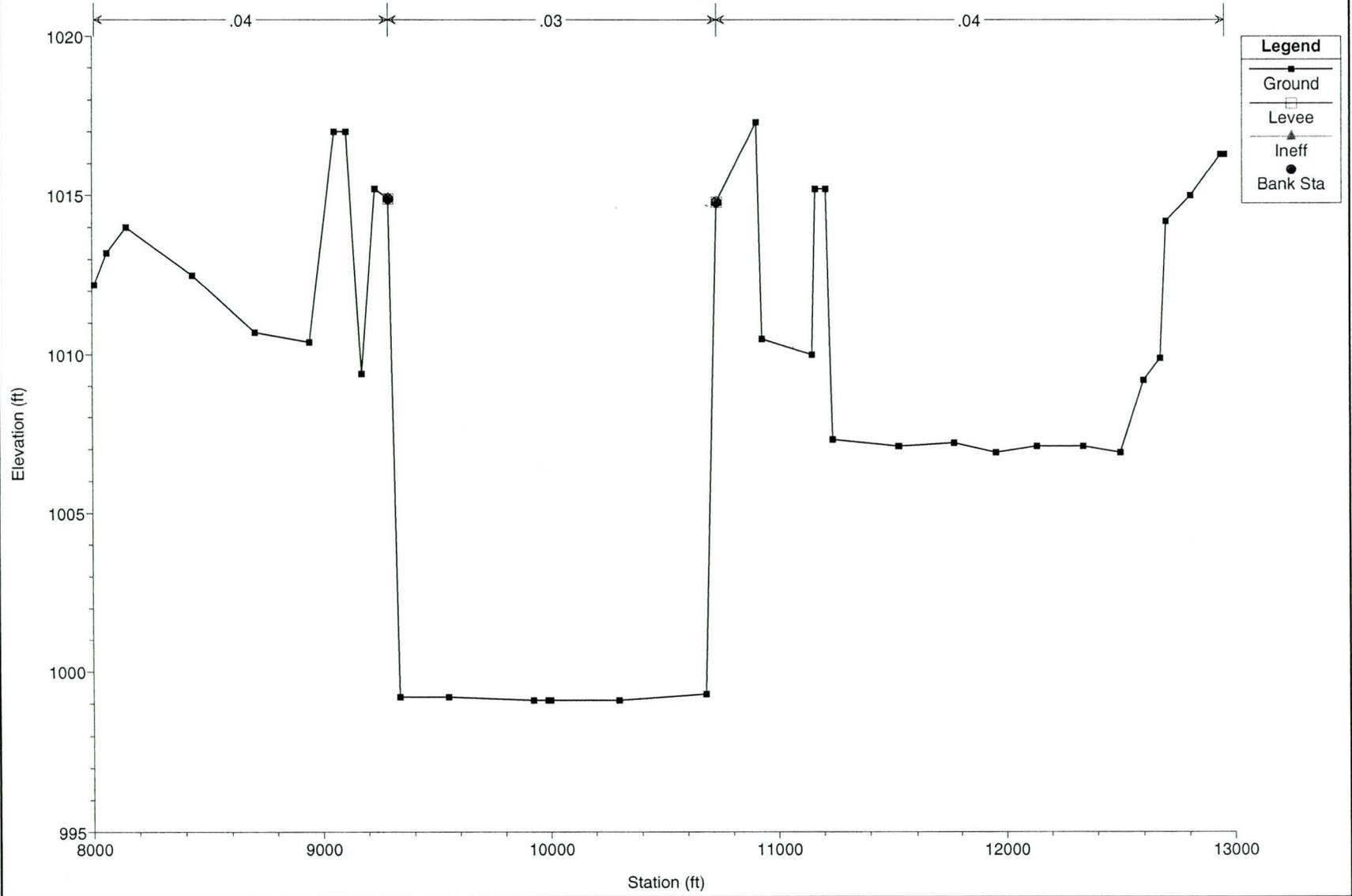
RS = 7.99



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

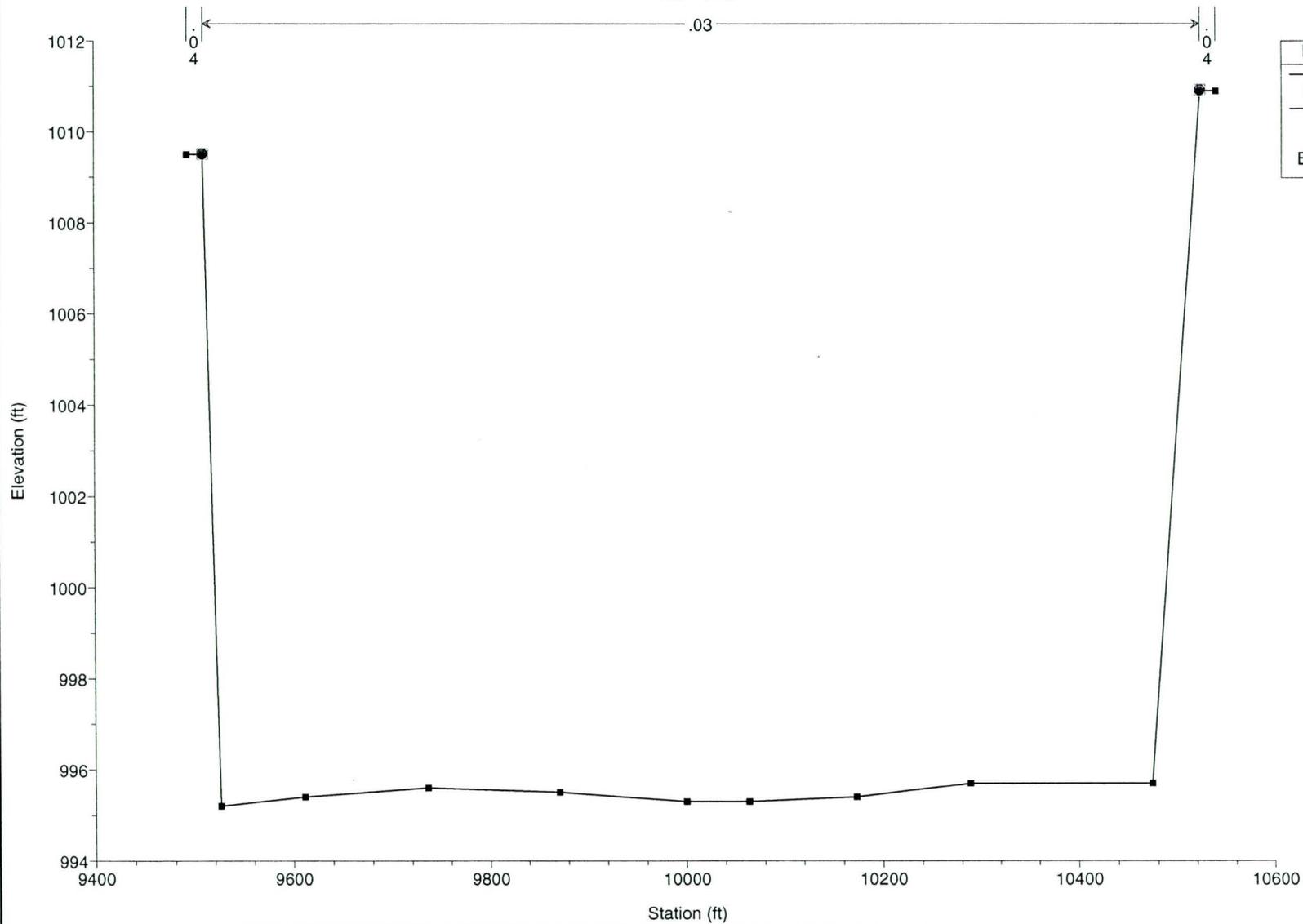
RS = 7.988



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

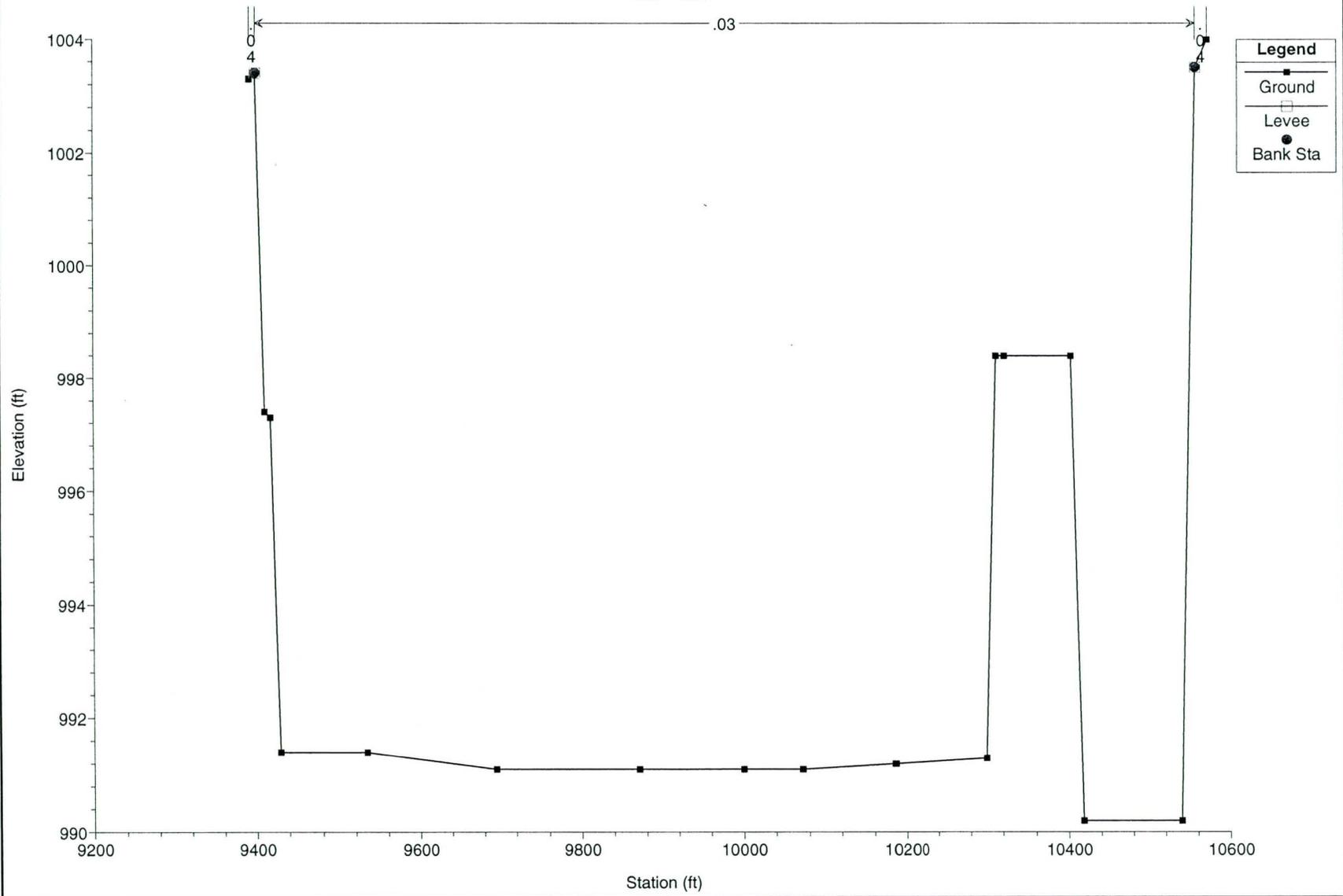
RS = 7.49



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

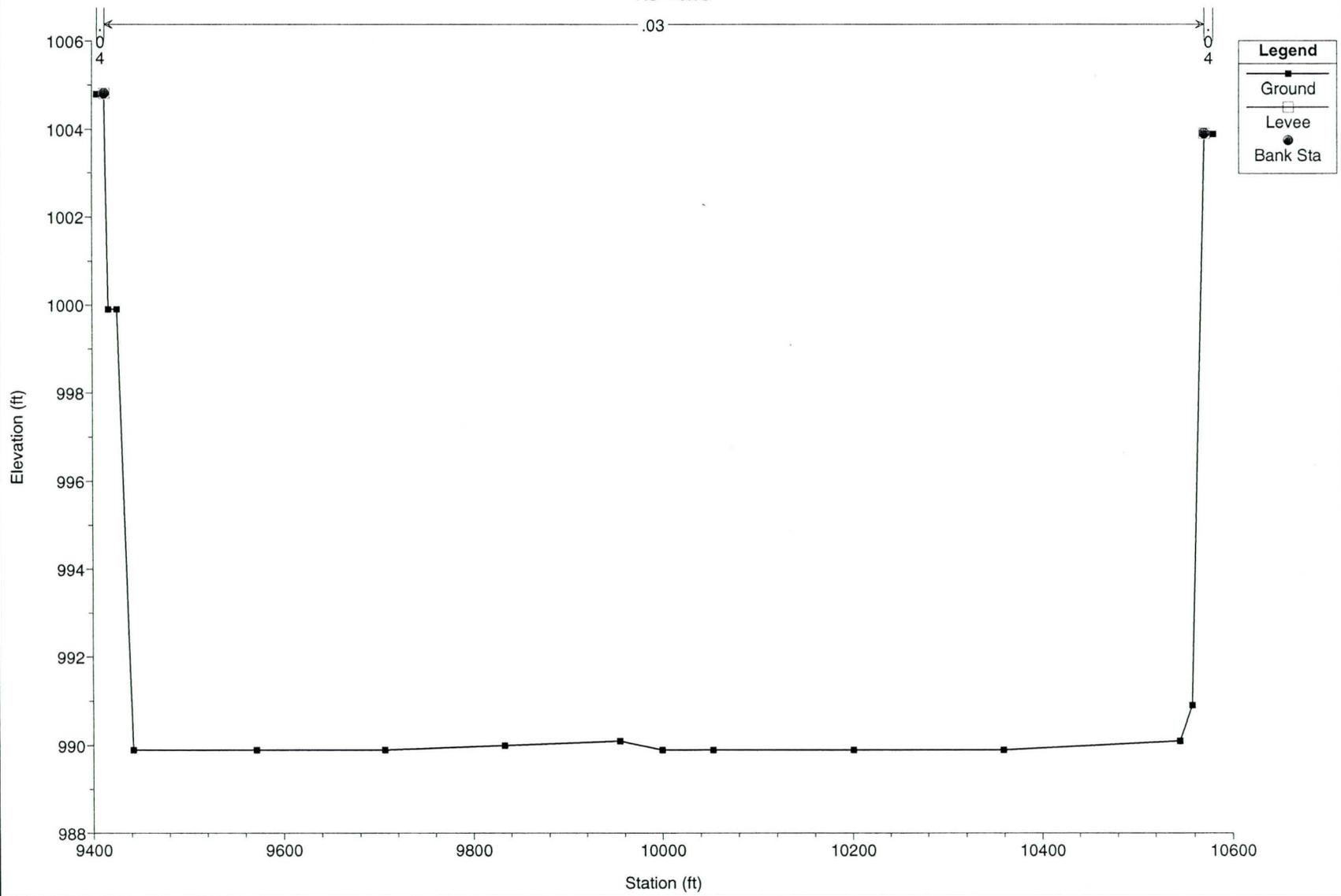
RS = 6.99



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

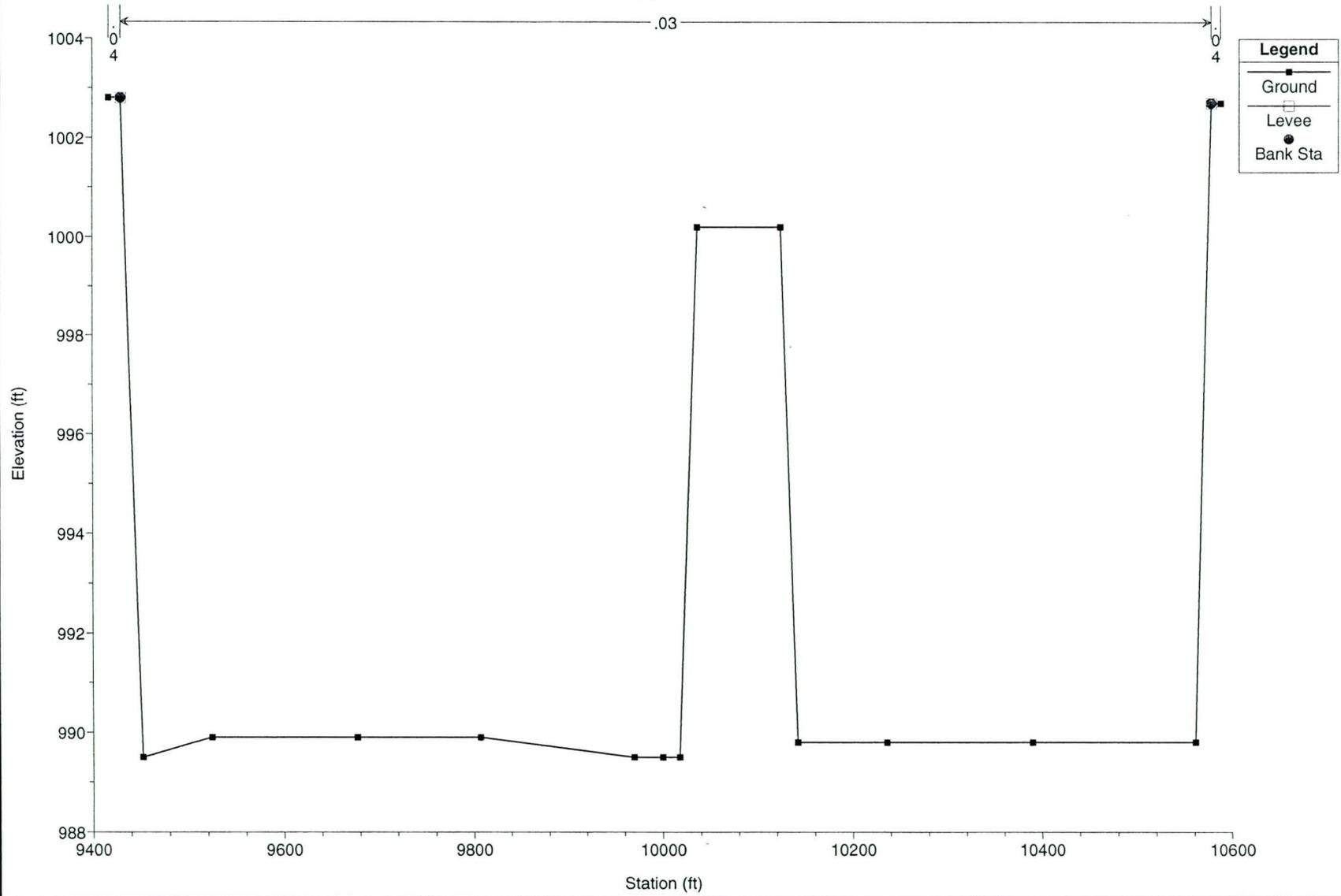
RS = 6.93



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

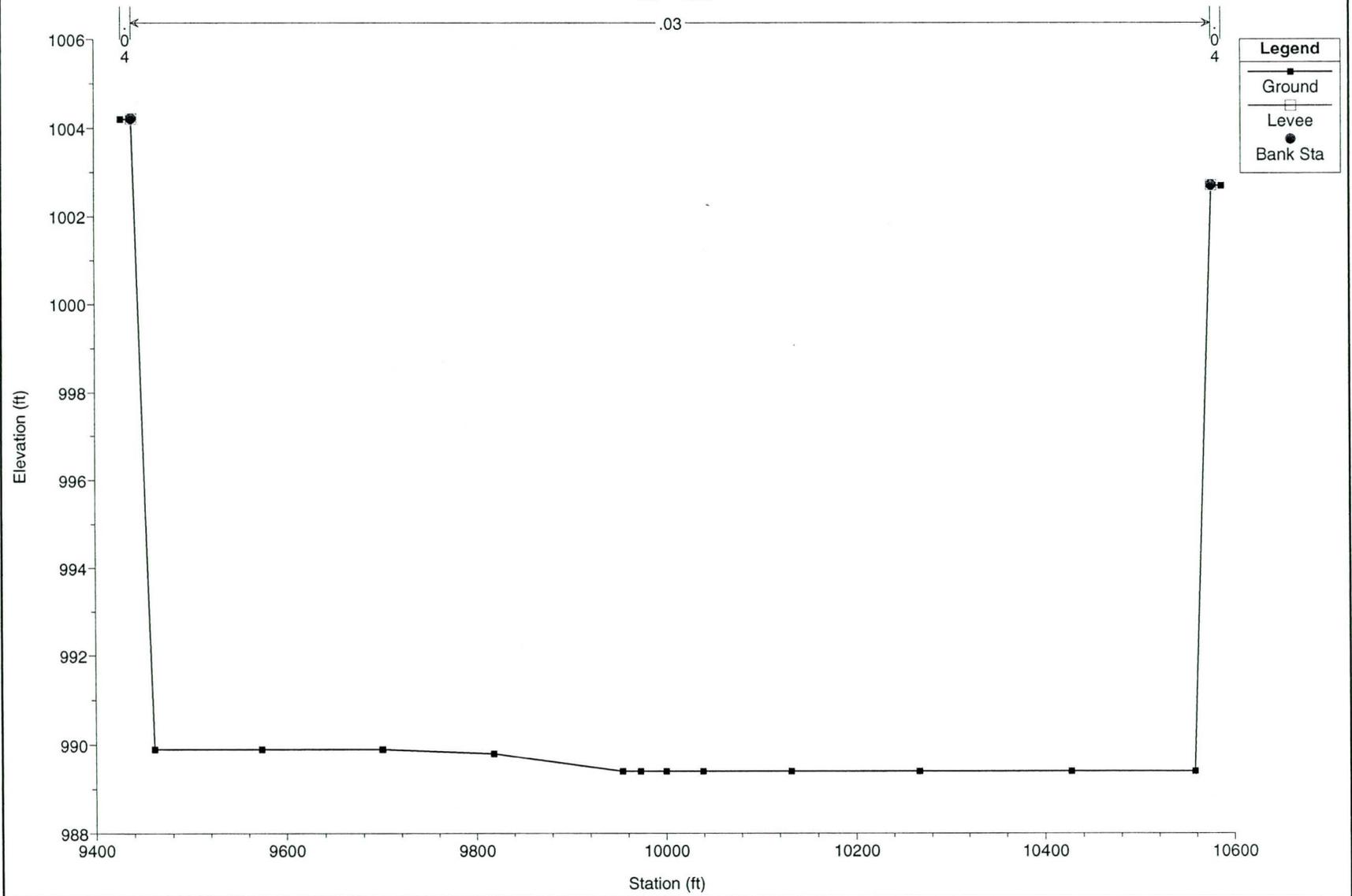
RS = 6.91



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

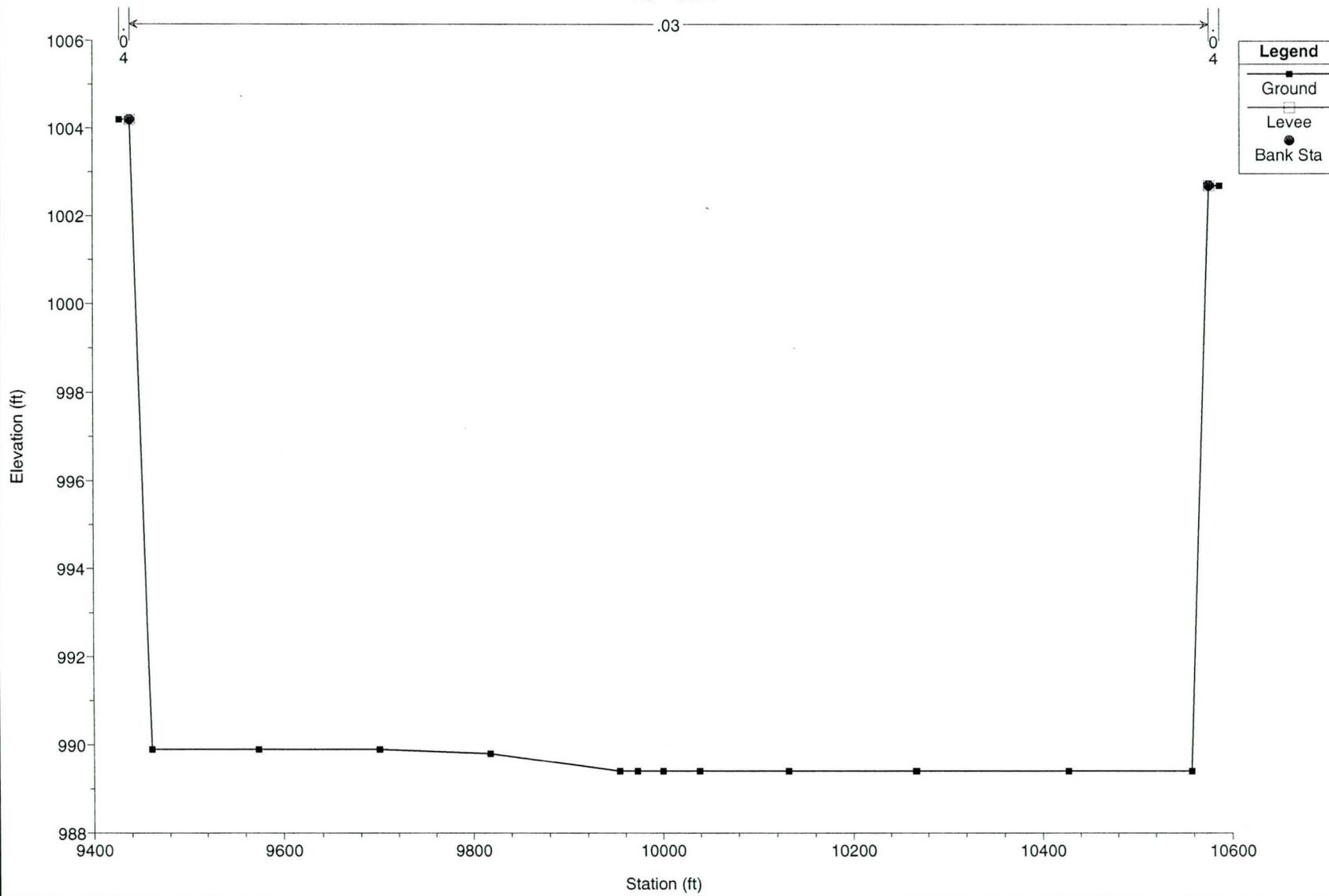
RS = 6.89



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

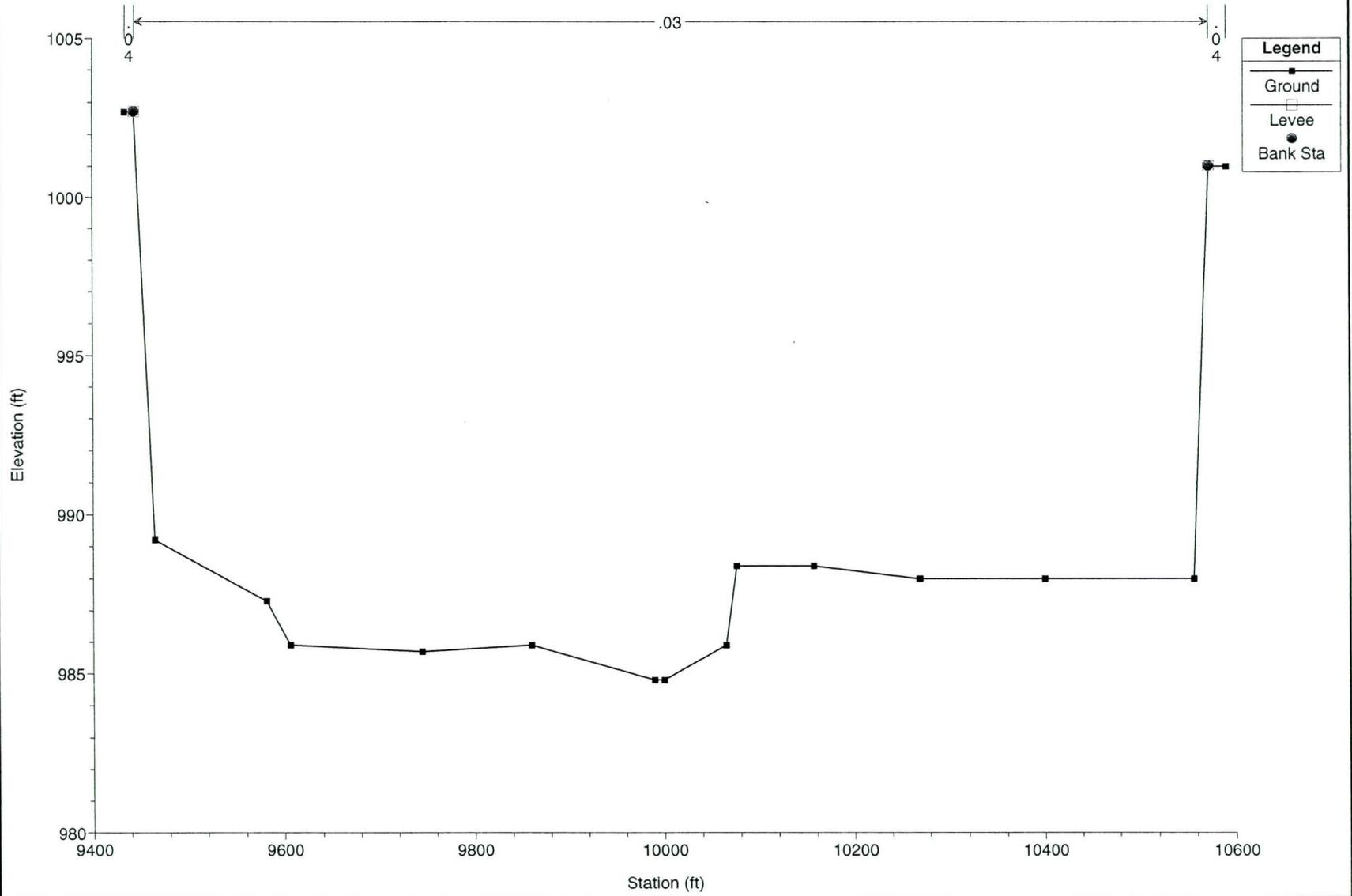
RS = 6.888



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

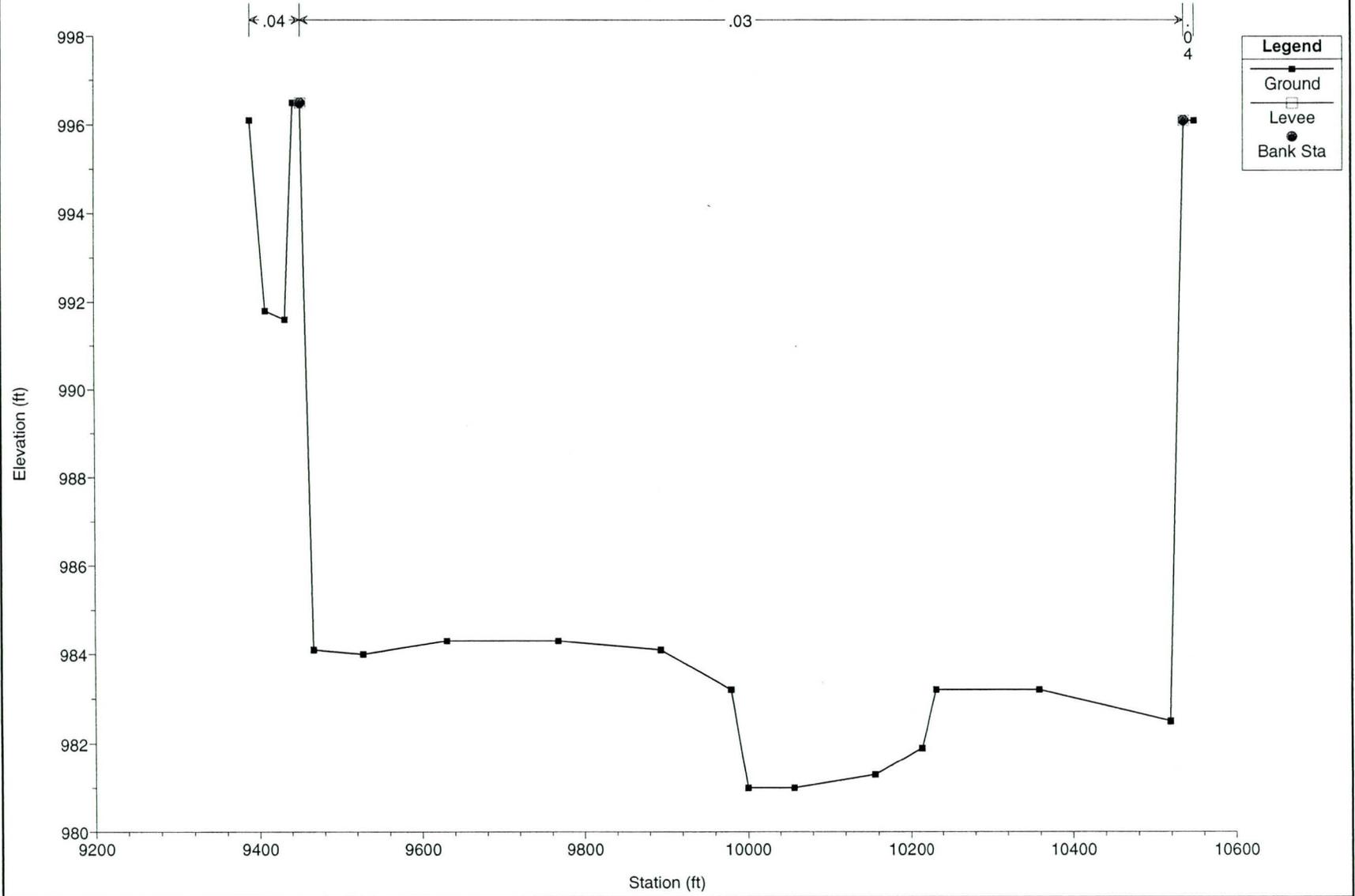
RS = 6.82



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

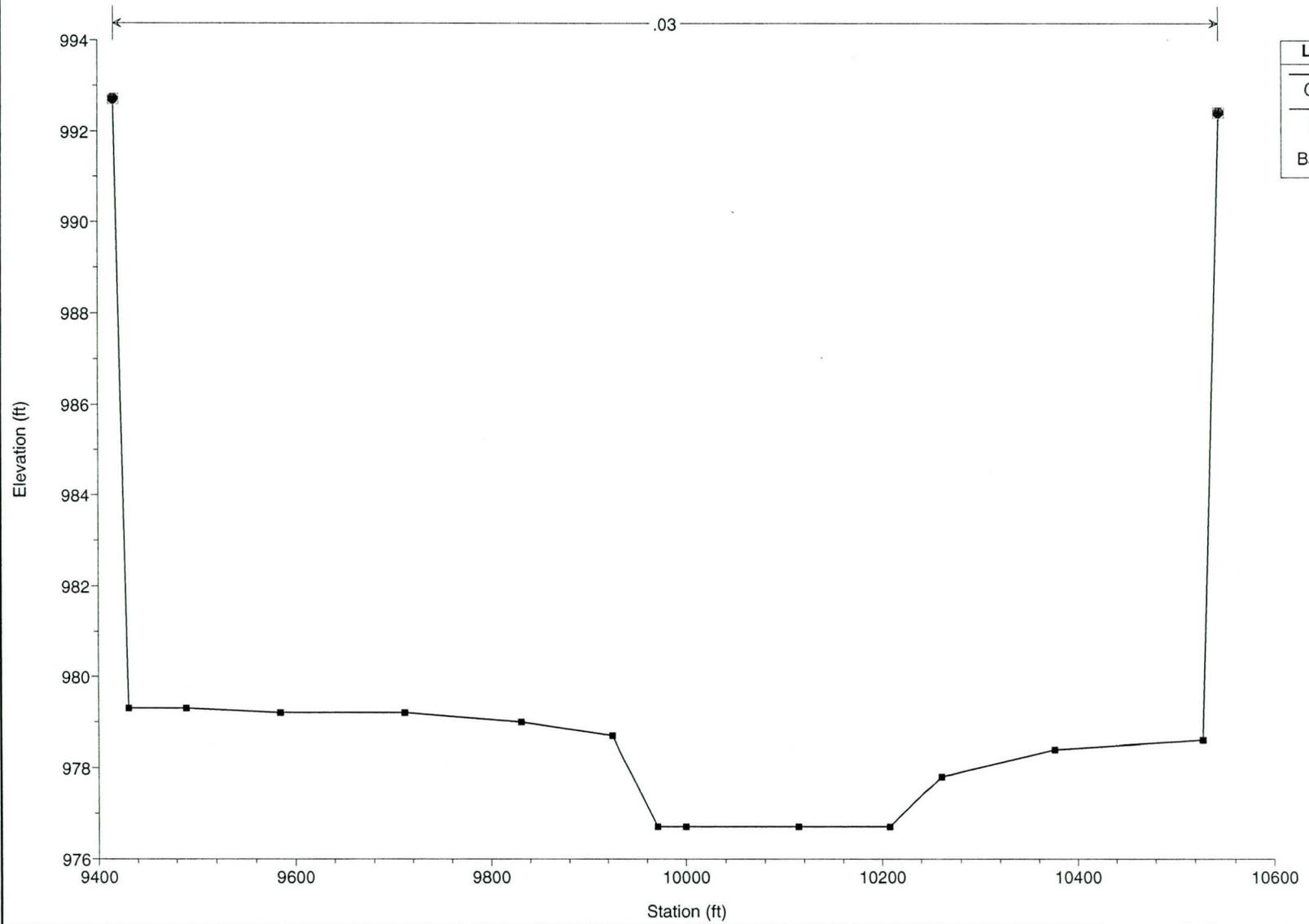
RS = 6.43



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 6.07

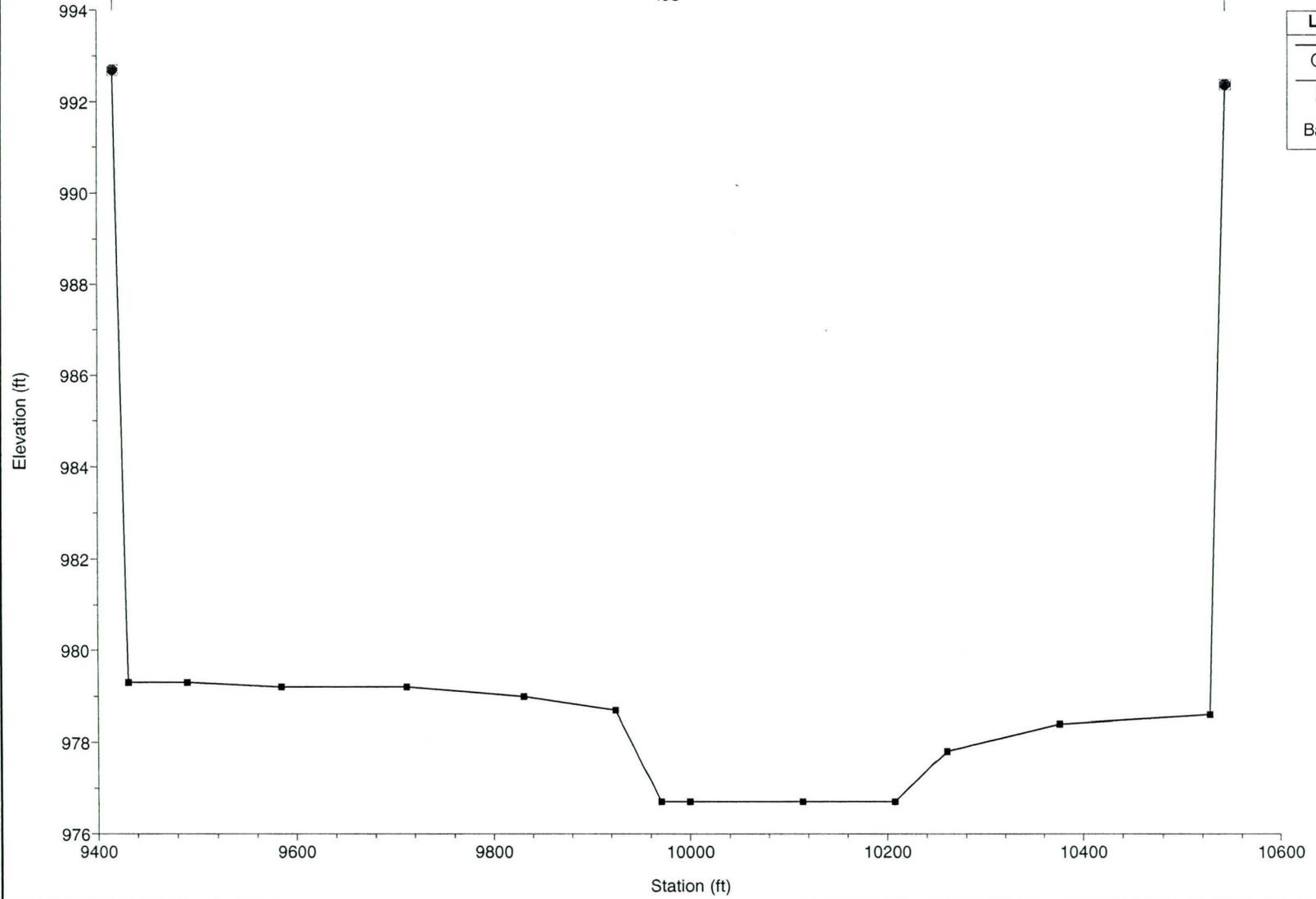


Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 6.068

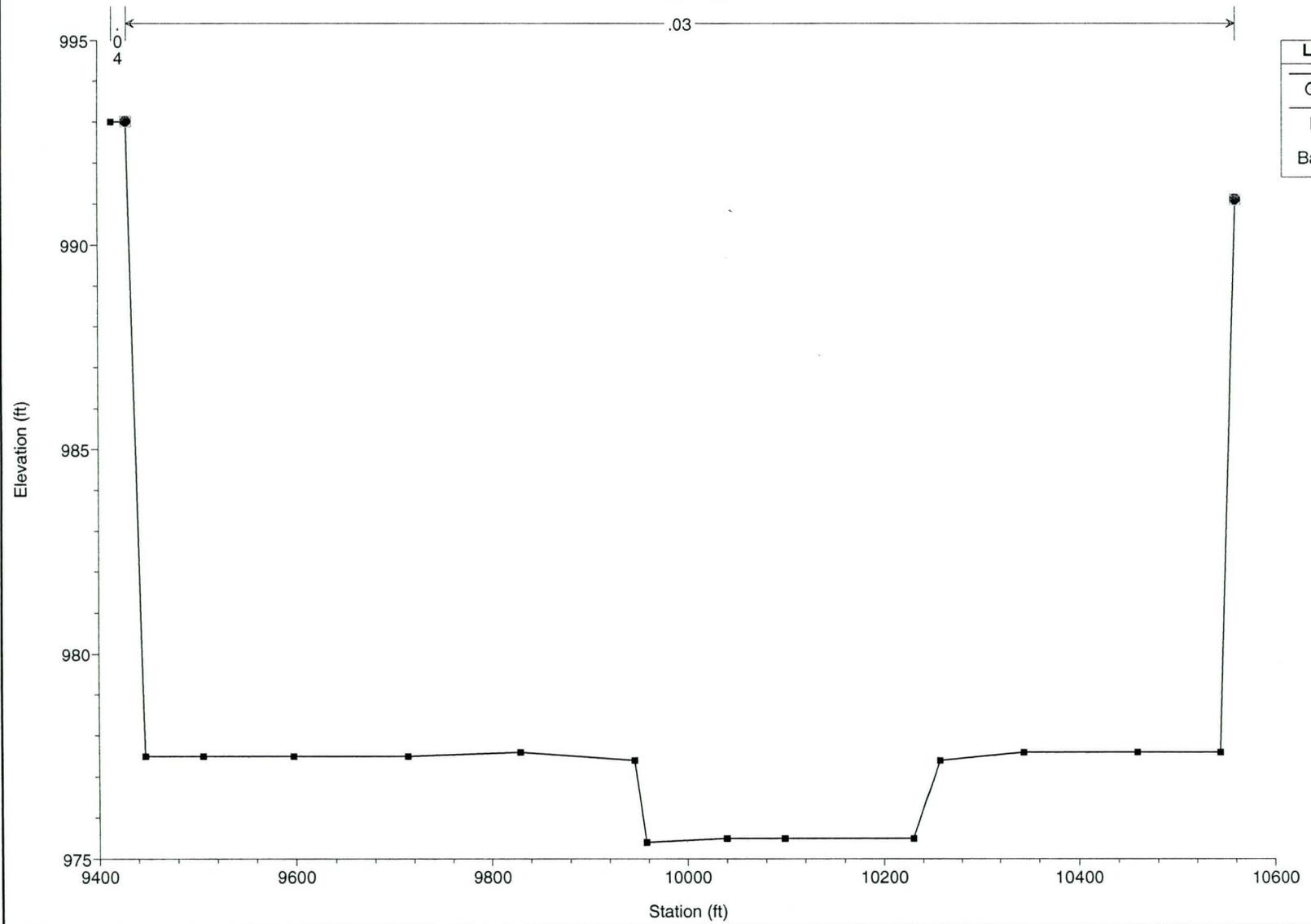
.03



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

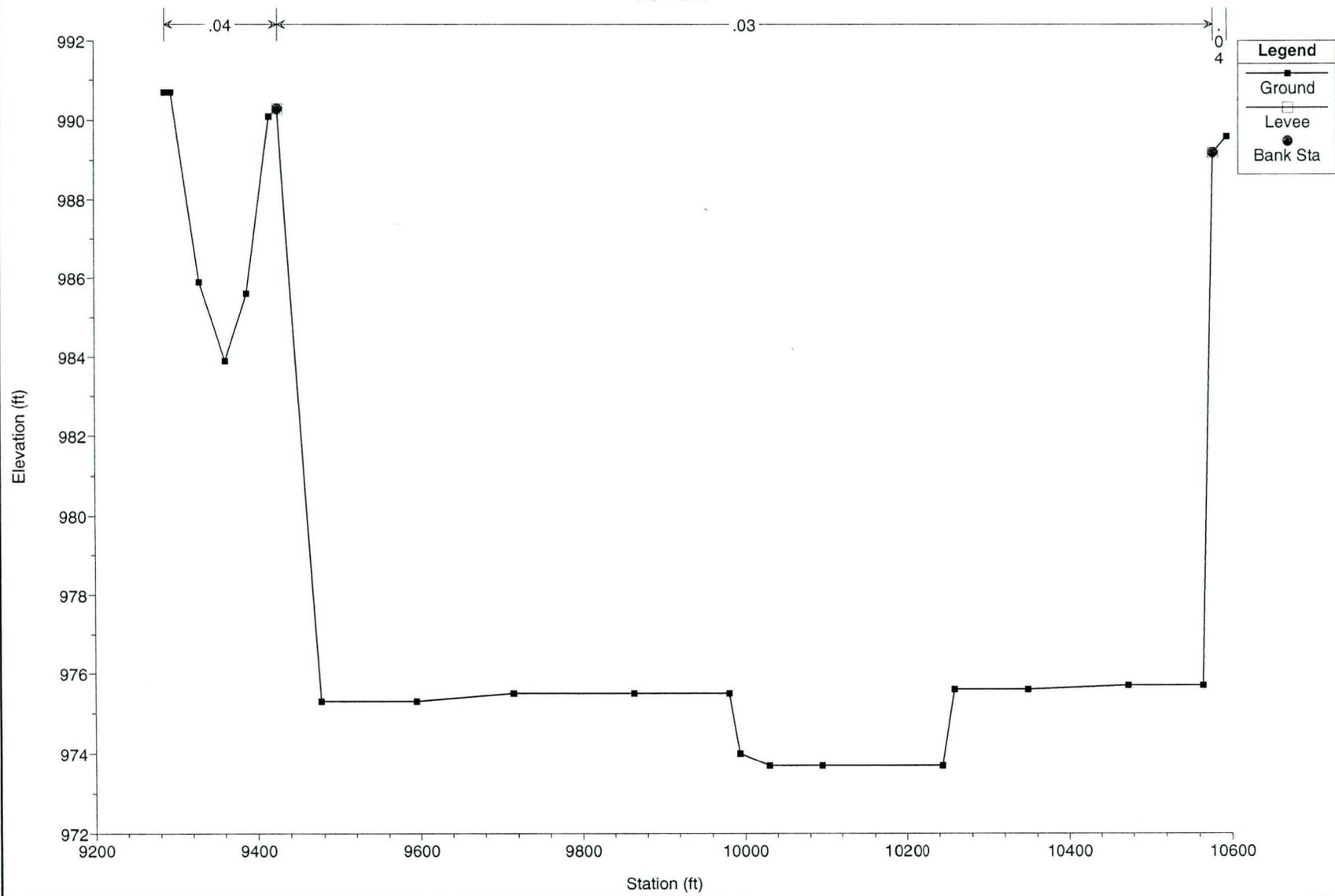
RS = 5.9



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

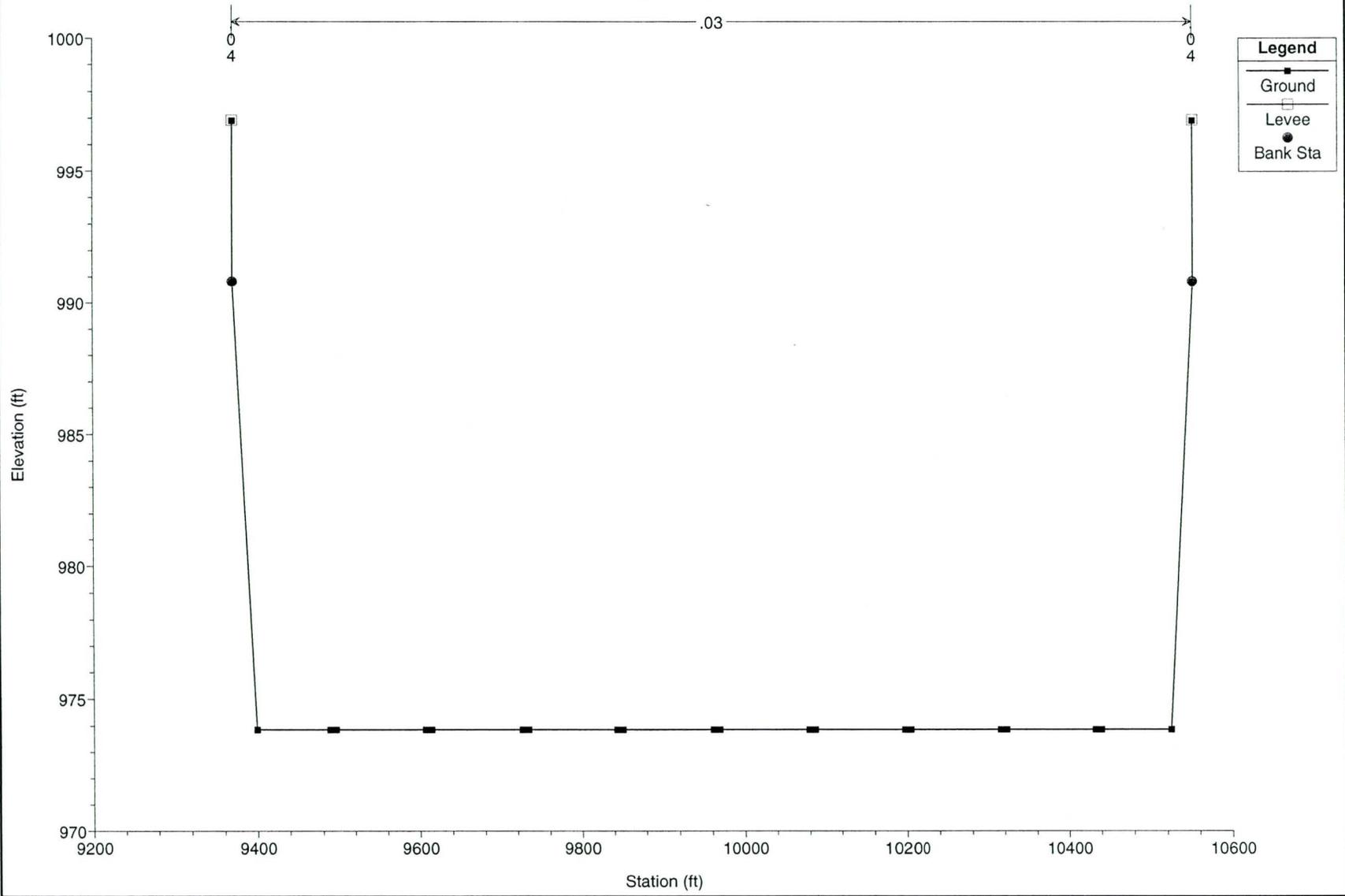
RS = 5.75



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

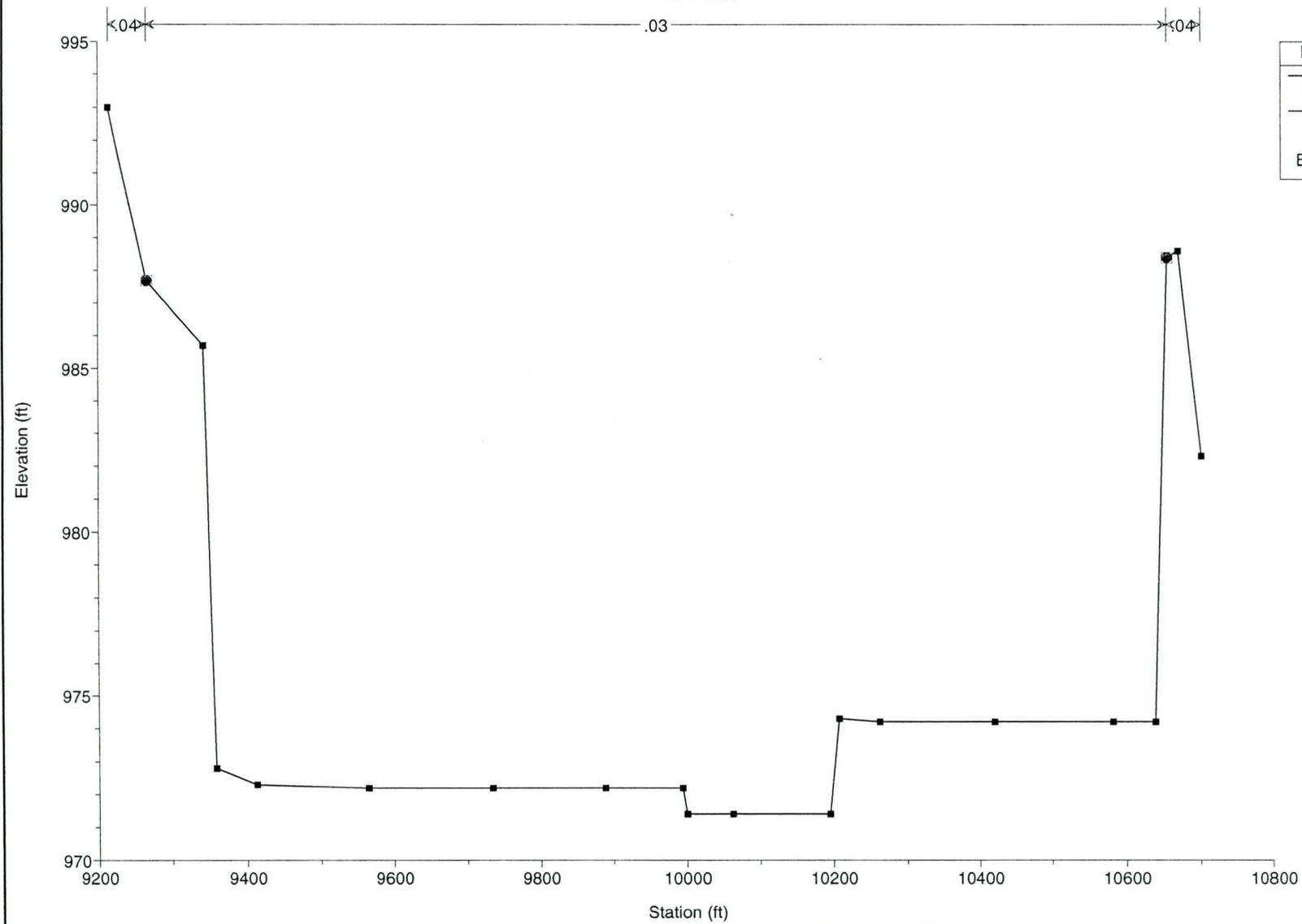
RS = 5.689



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

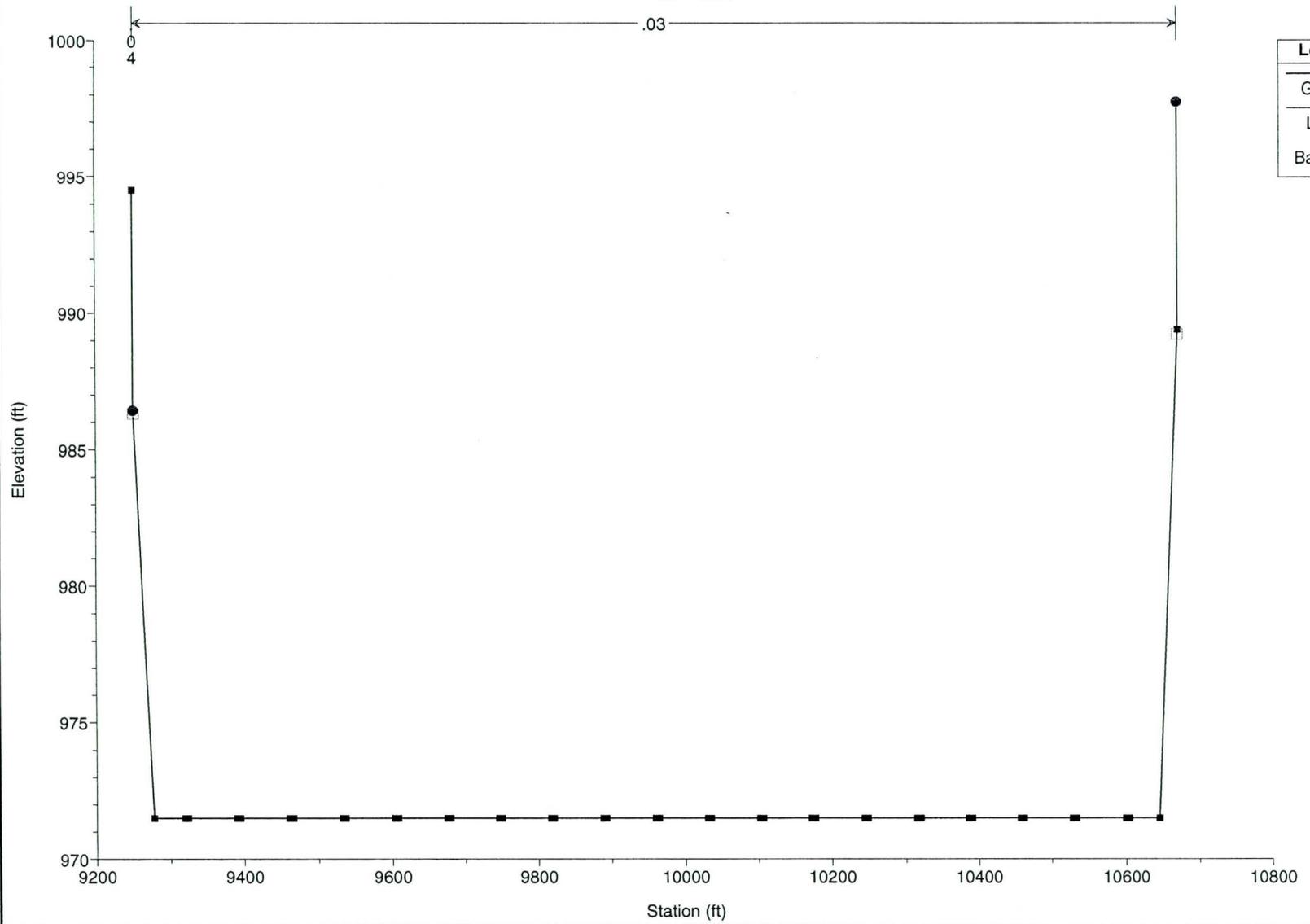
RS = 5.38



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

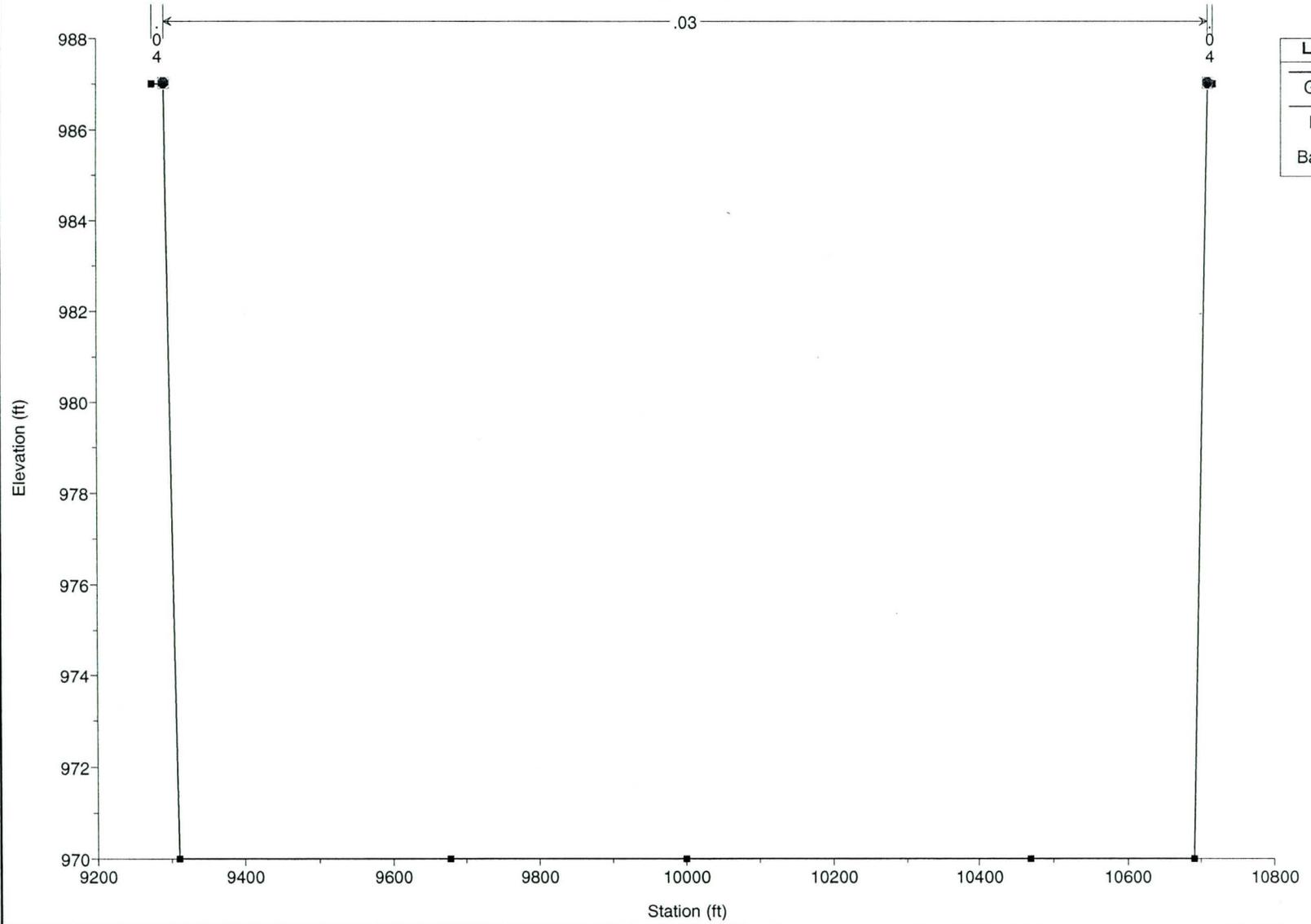
RS = 5.29



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

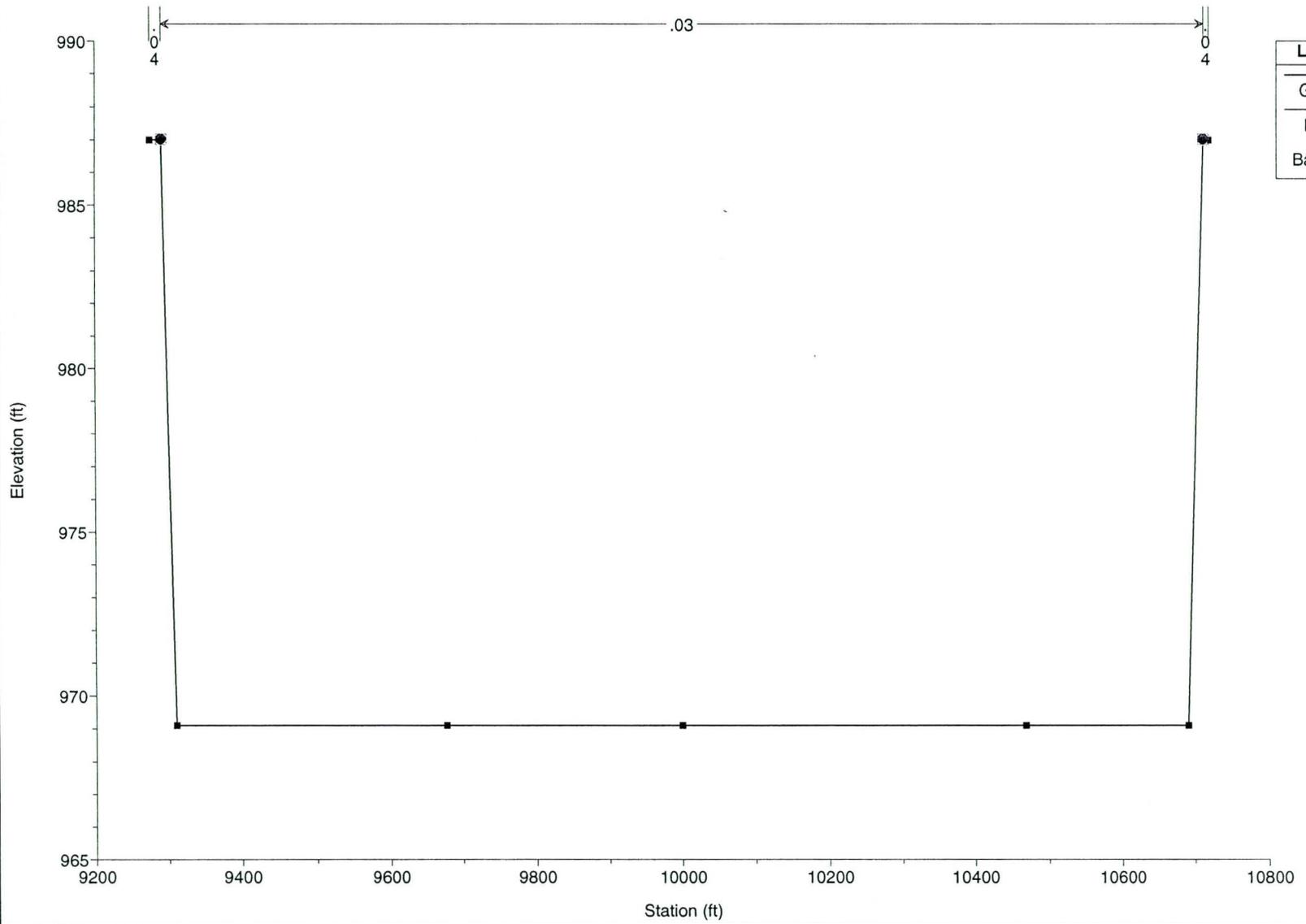
RS = 5.203



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

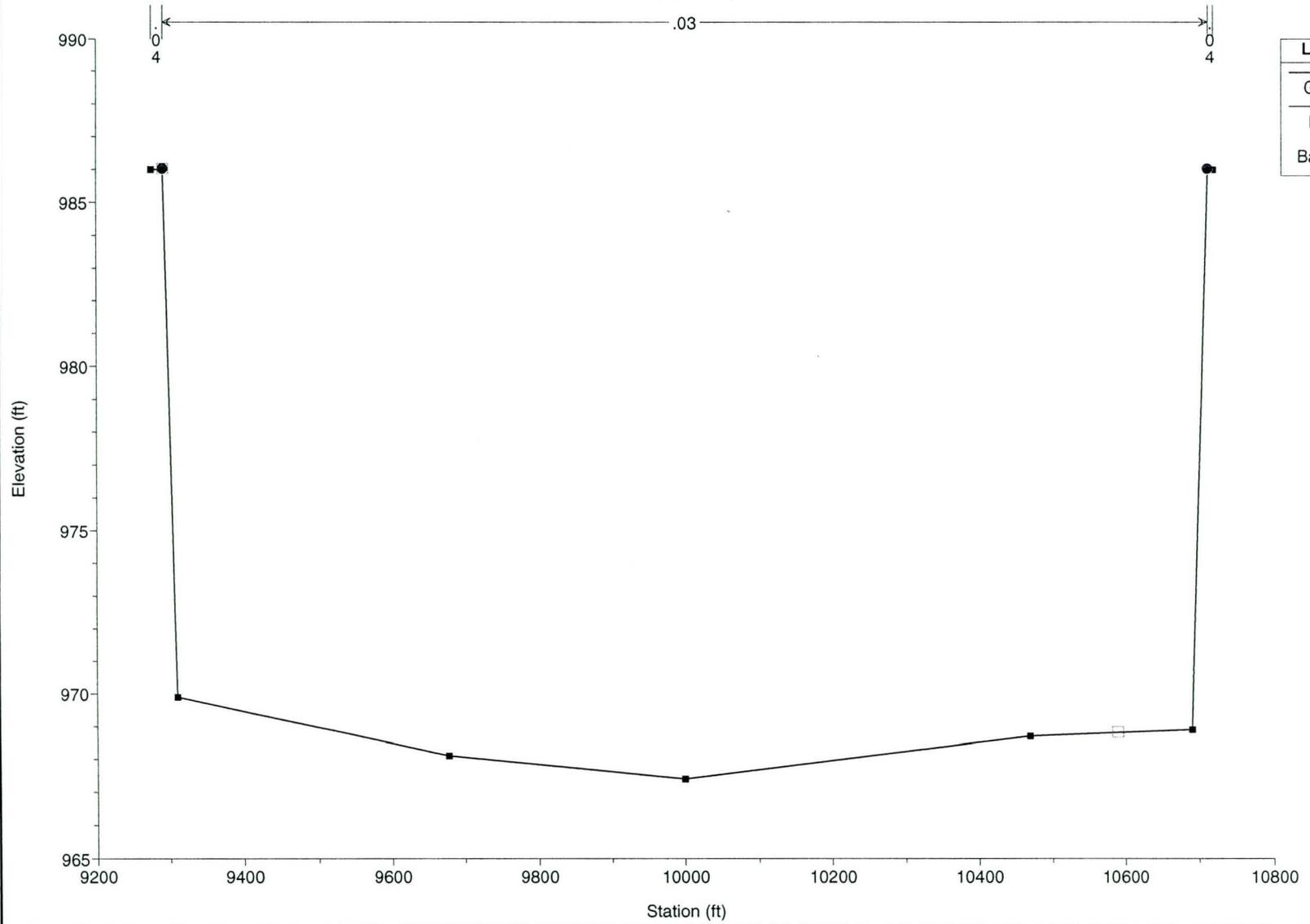
RS = 5.201



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

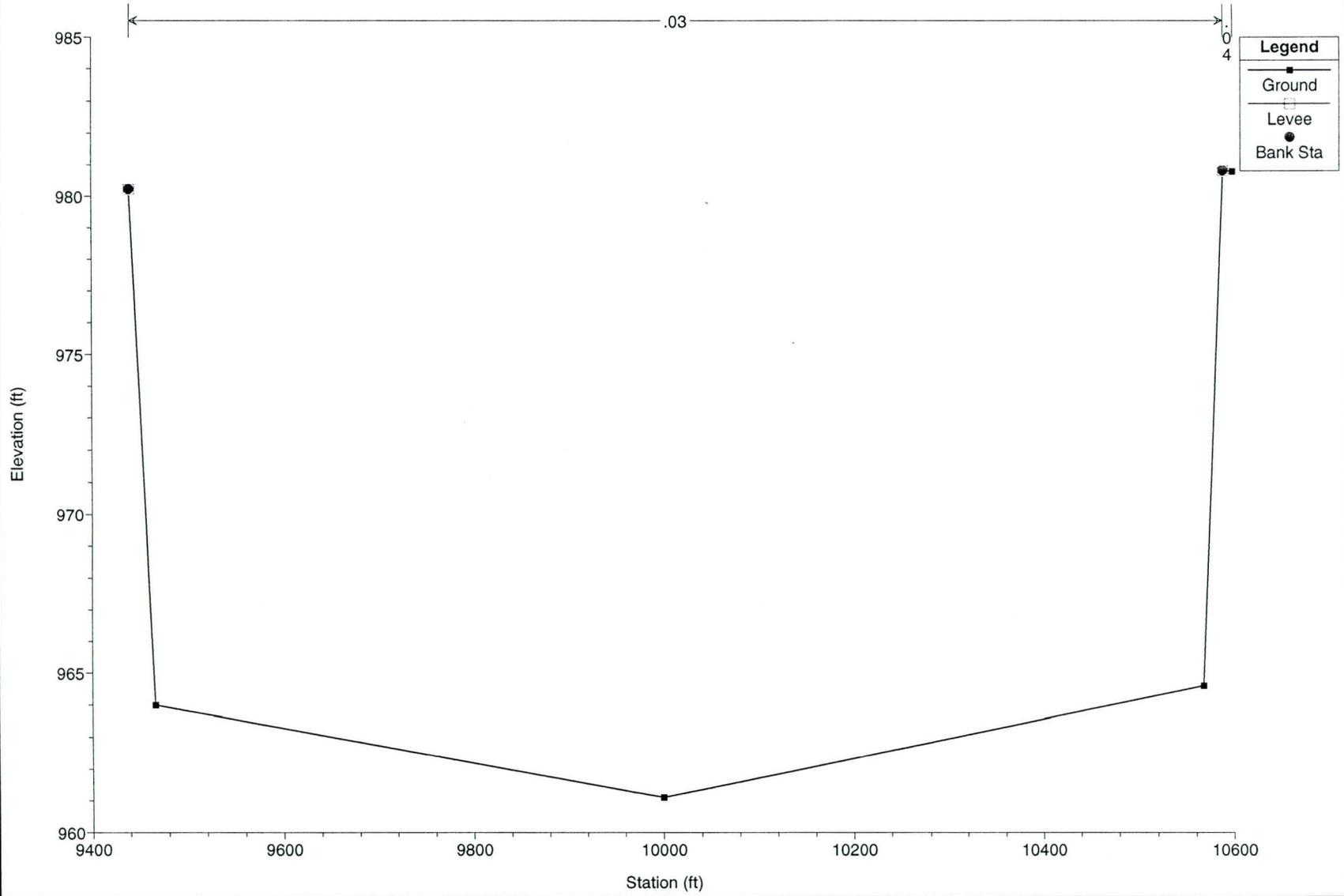
RS = 5.15



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

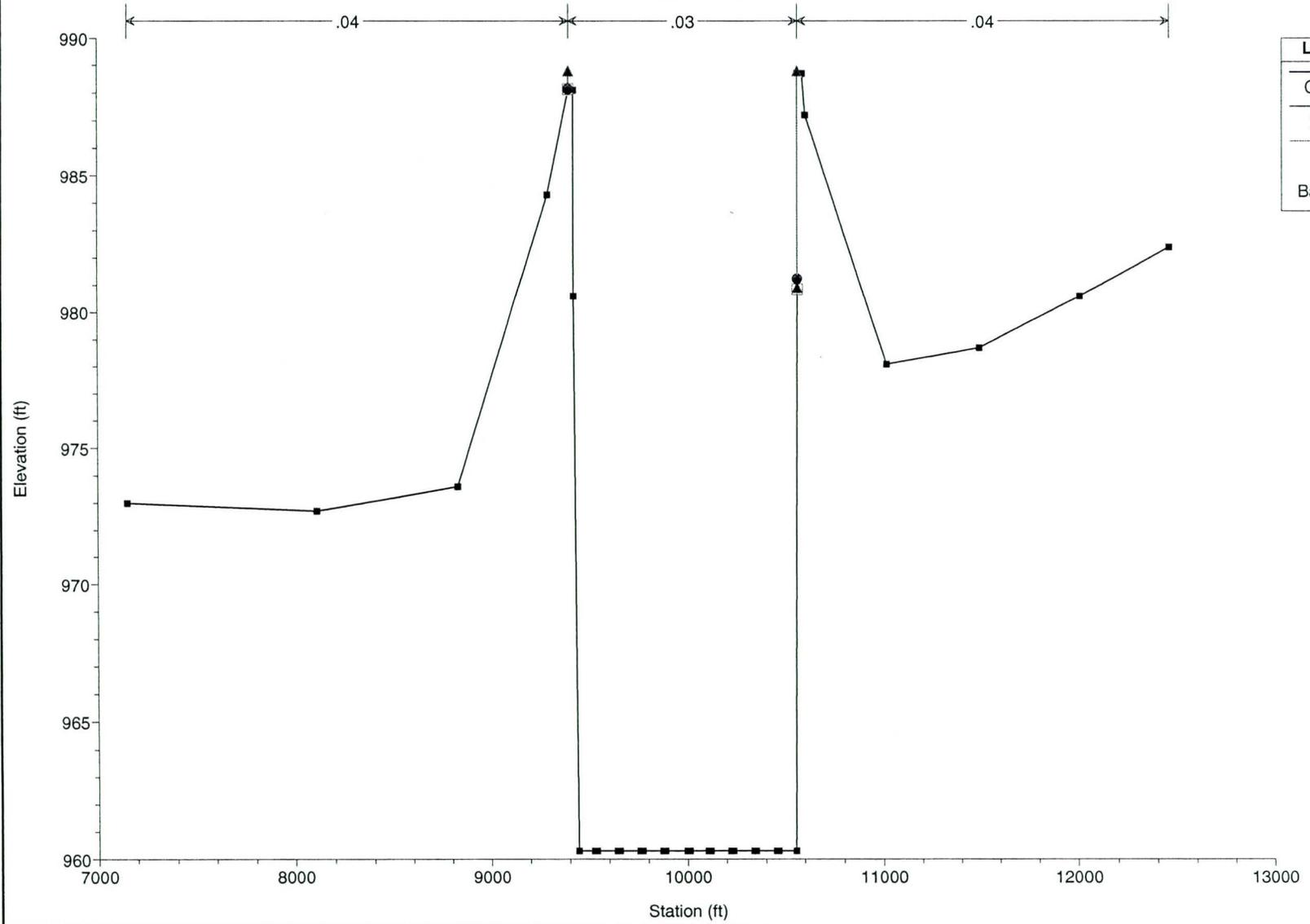
RS = 4.79



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

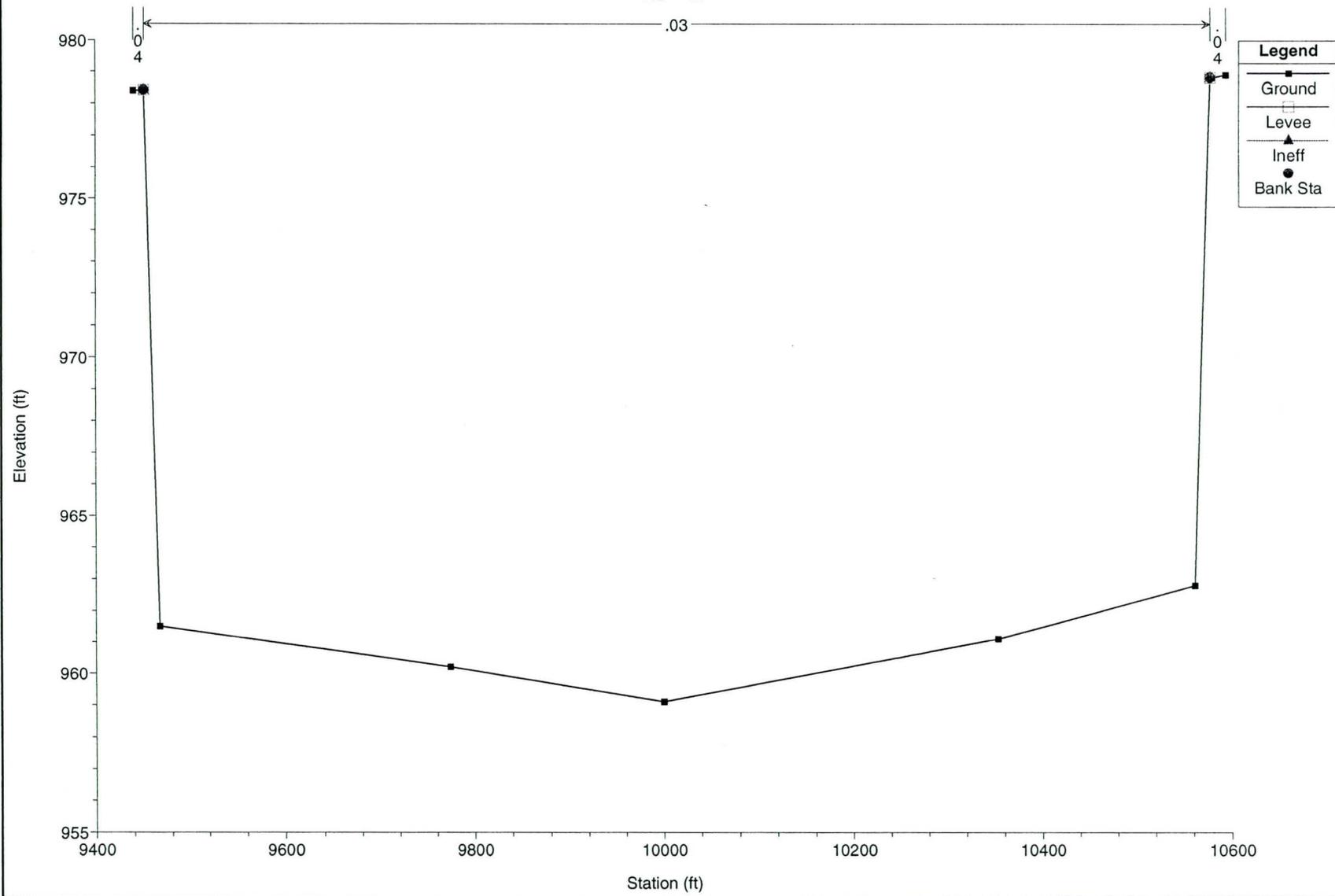
RS = 4.754



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

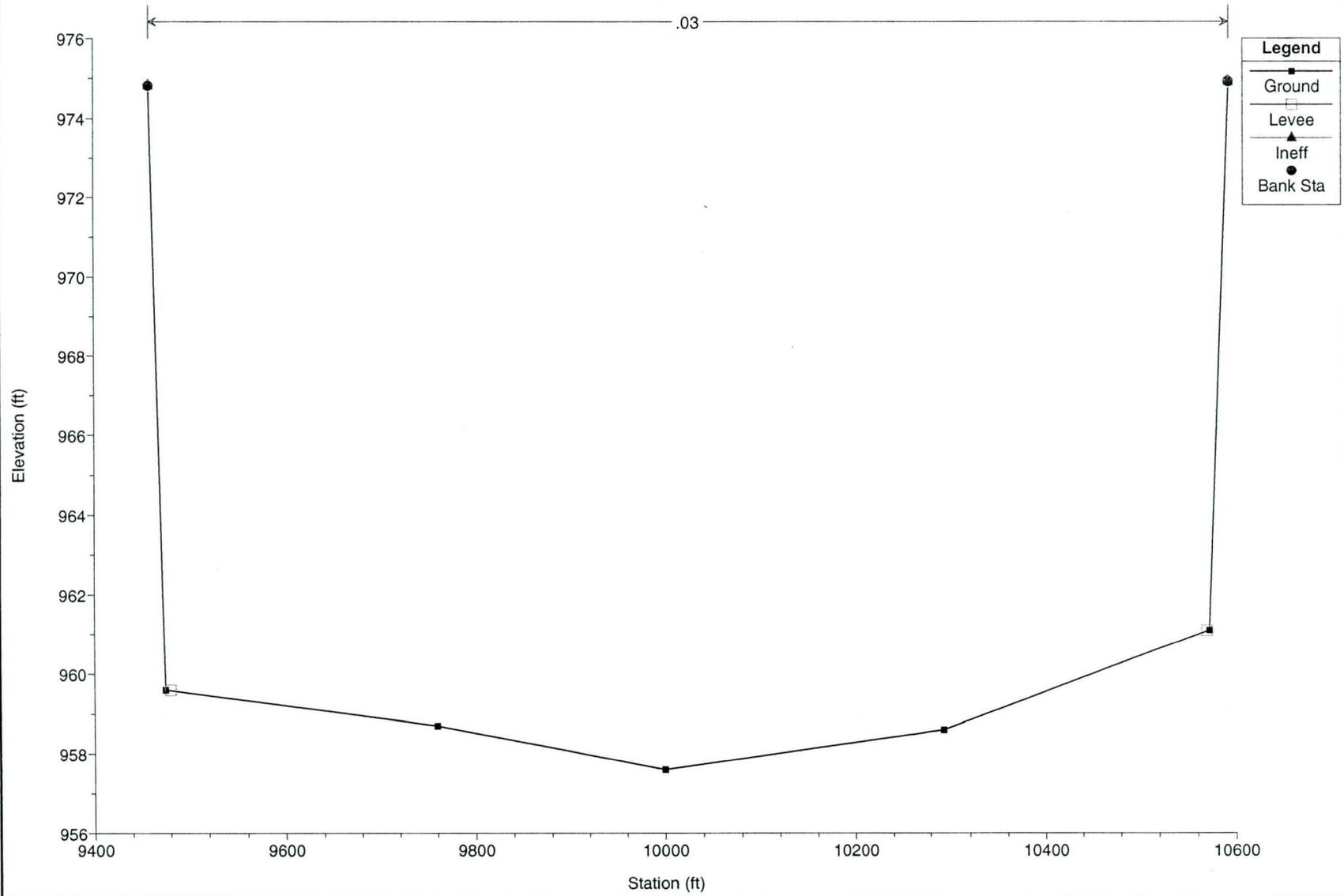
RS = 4.7



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

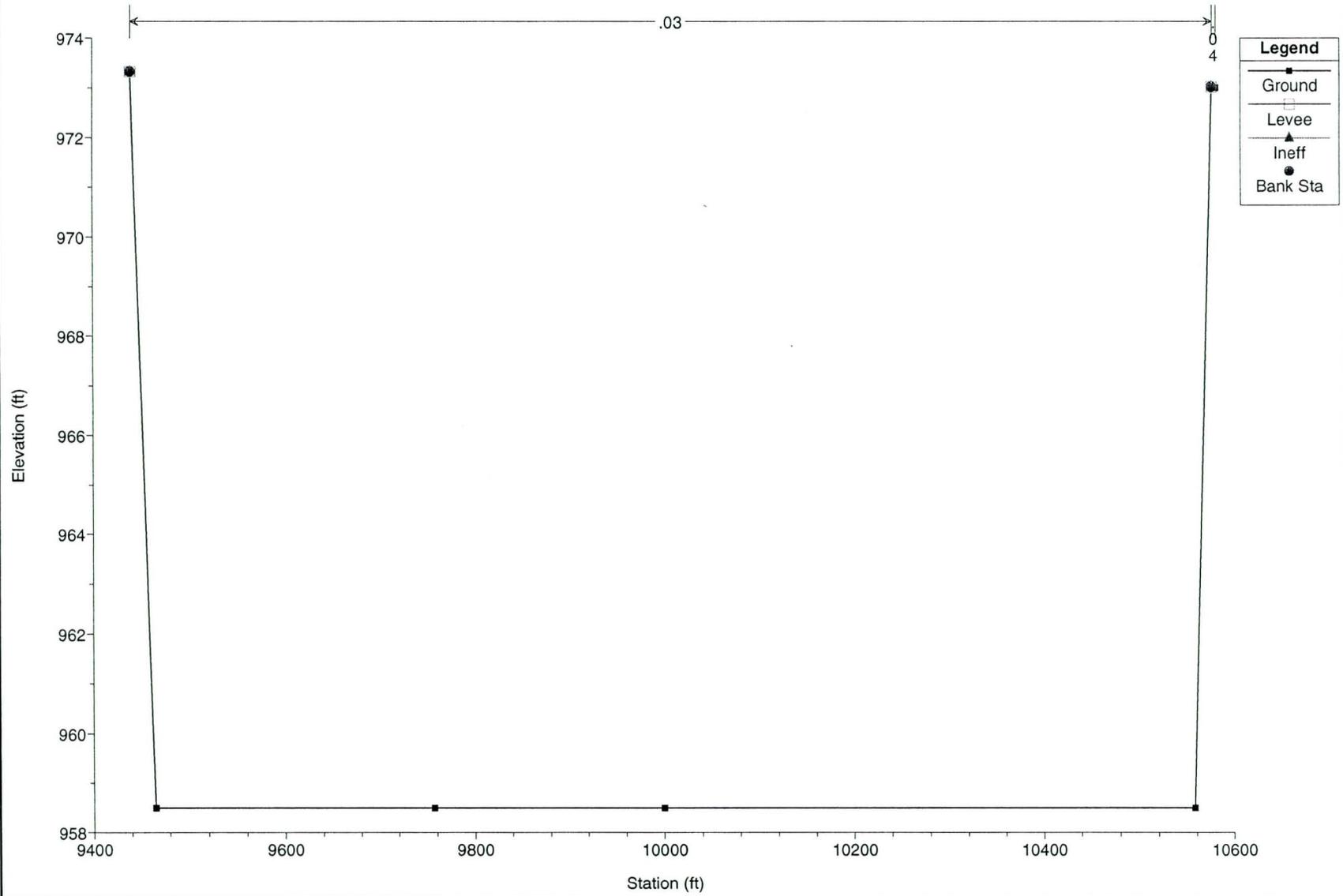
RS = 4.3



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

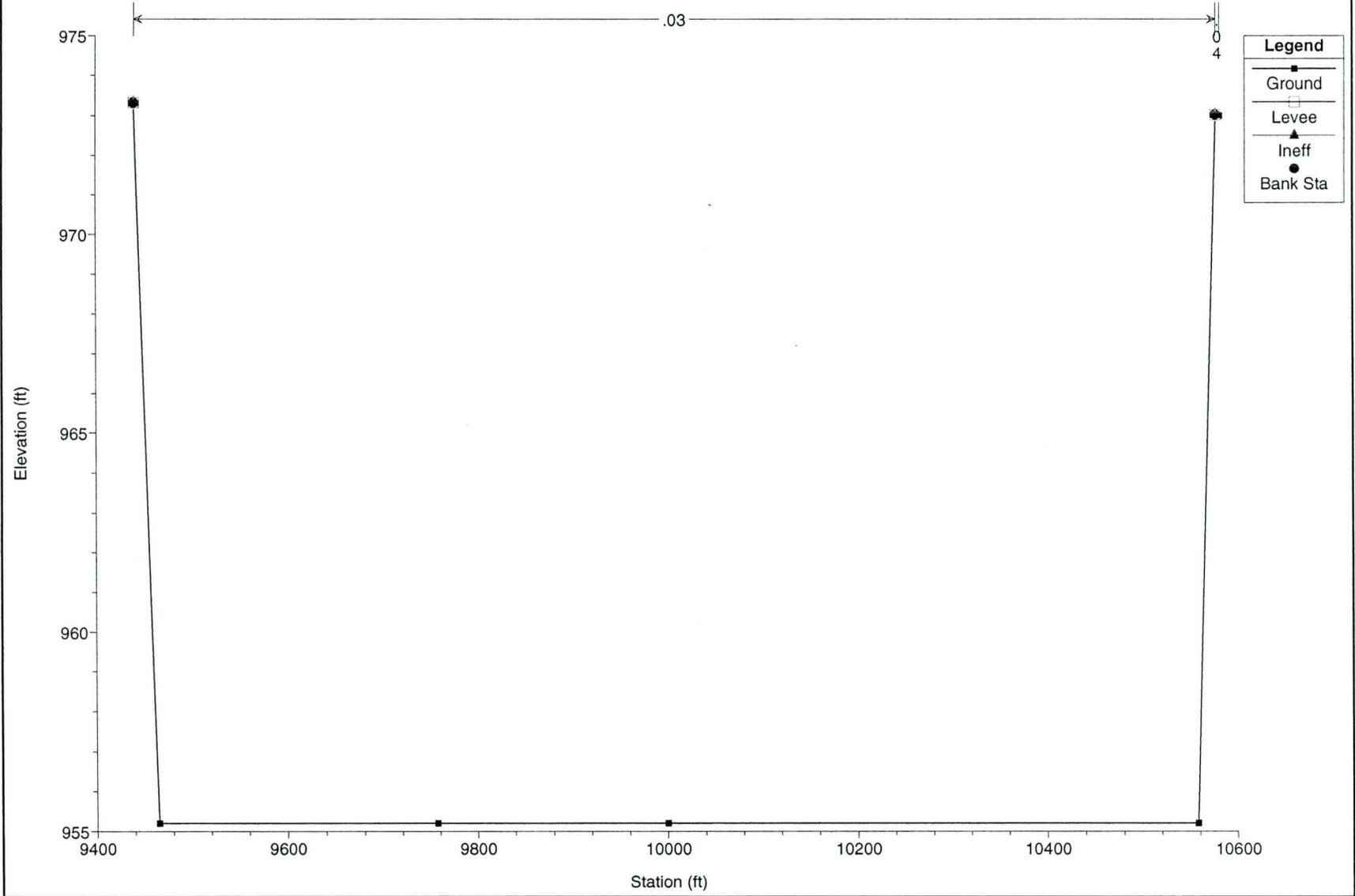
RS = 4.094



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

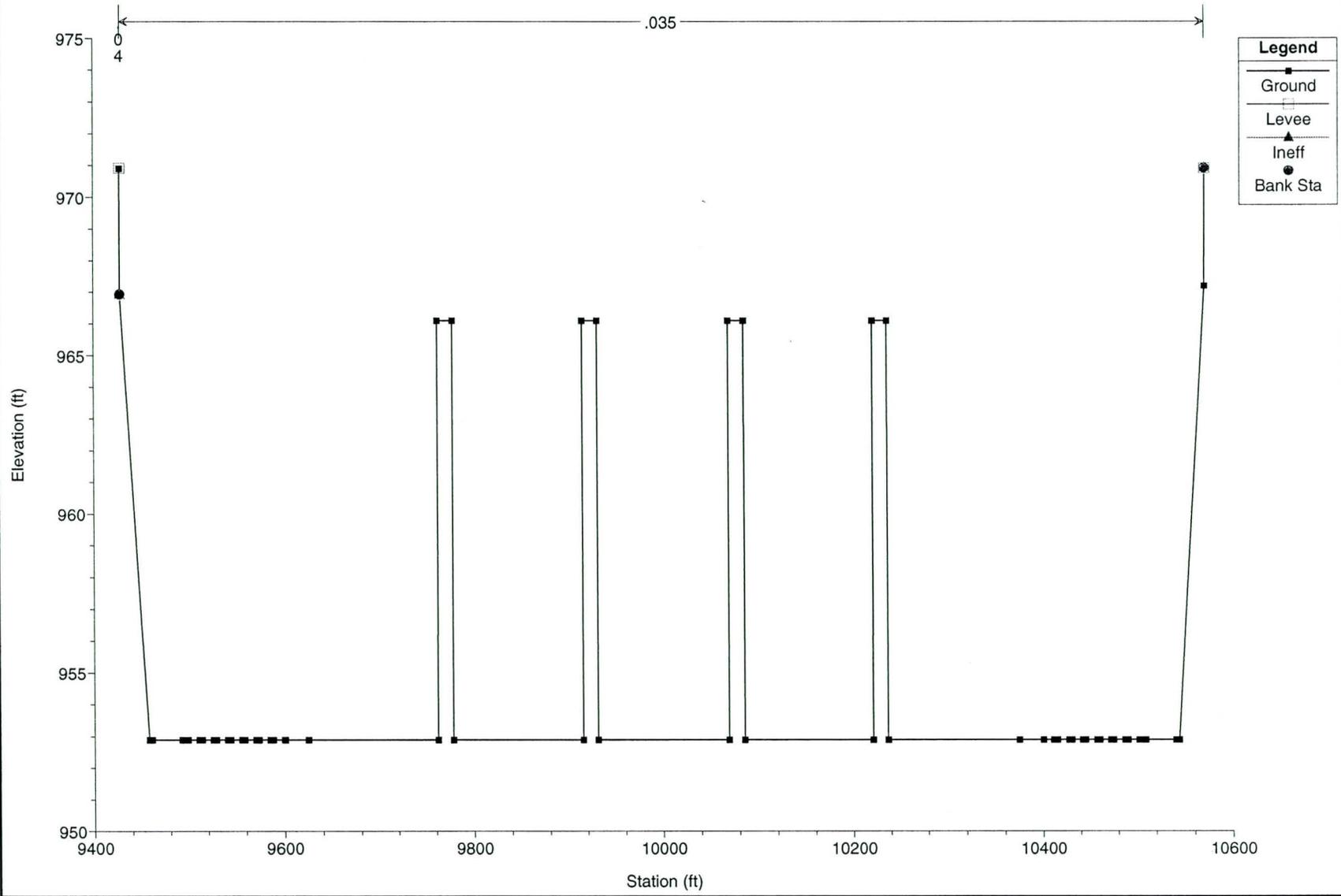
RS = 4.092



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

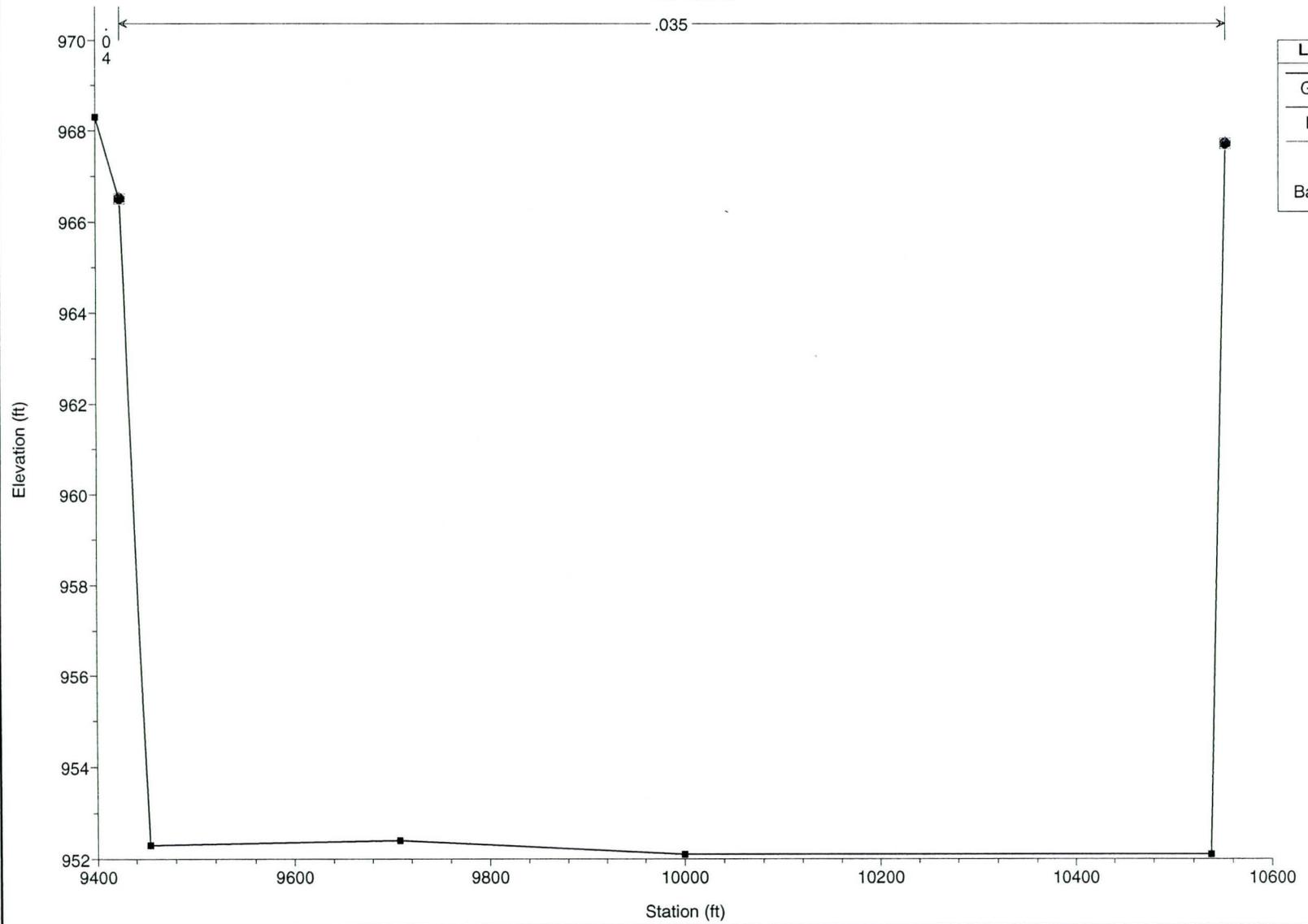
RS = 3.767



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

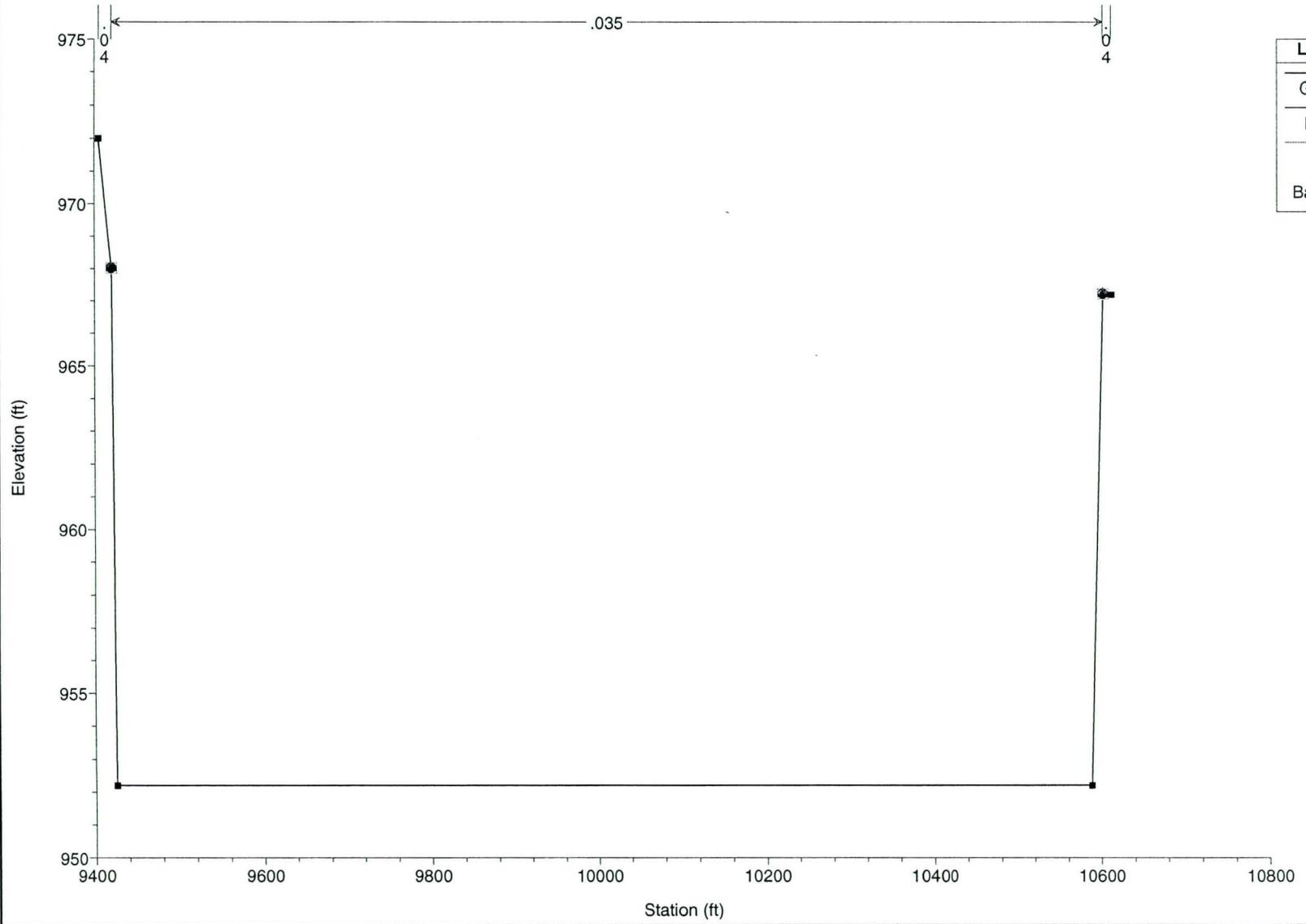
RS = 3.757



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

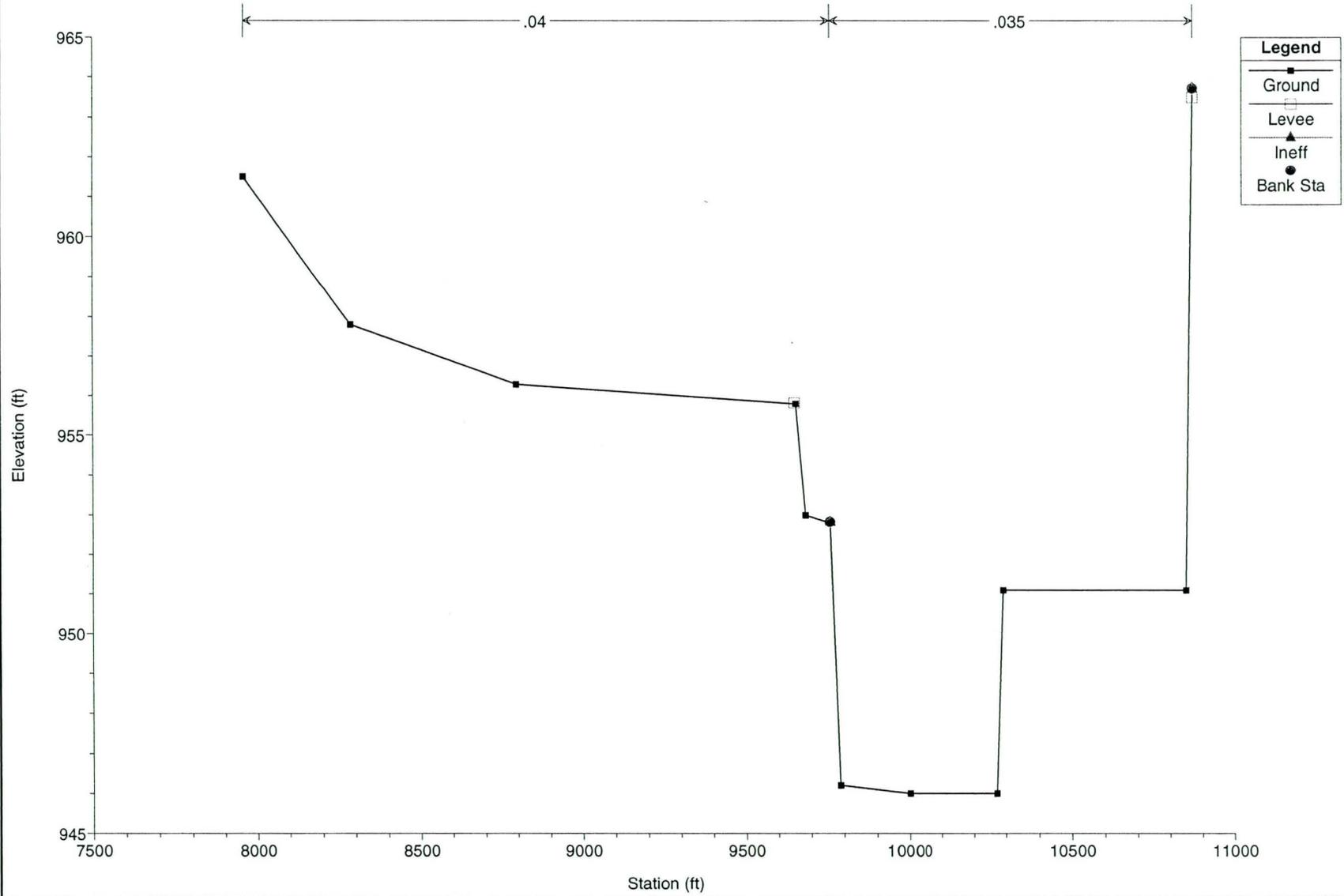
RS = 3.729



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

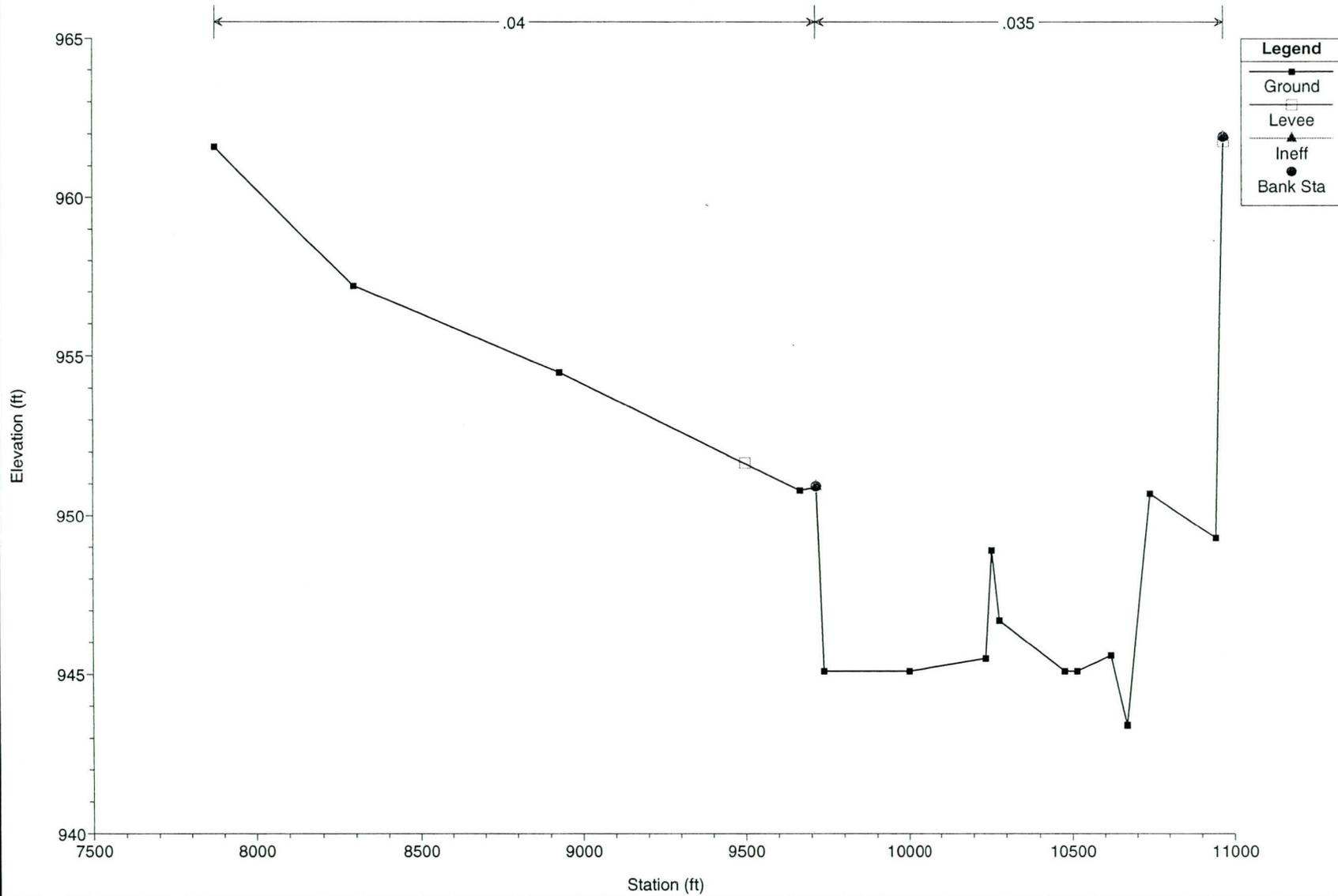
RS = 3.43



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

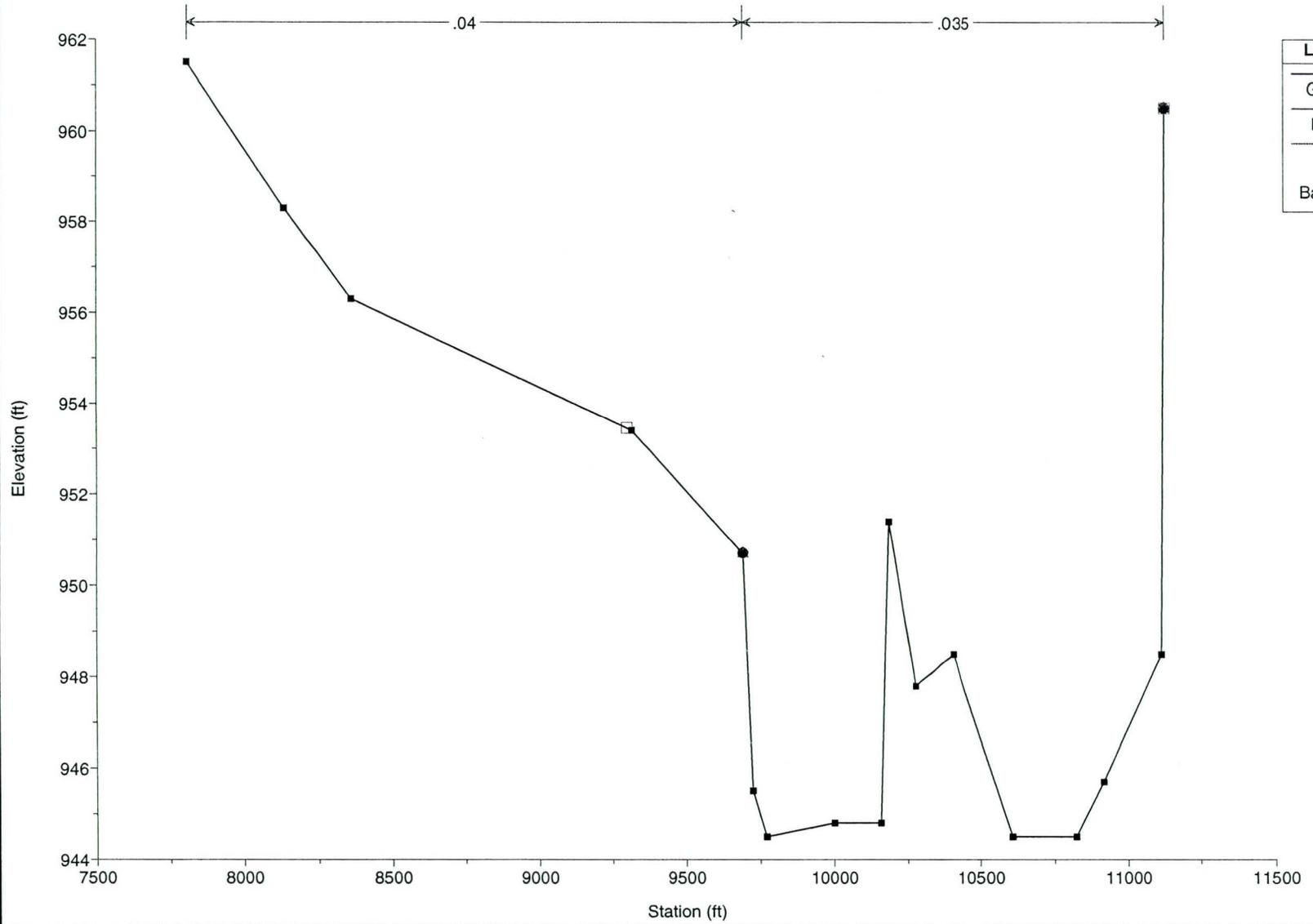
RS = 3.37



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

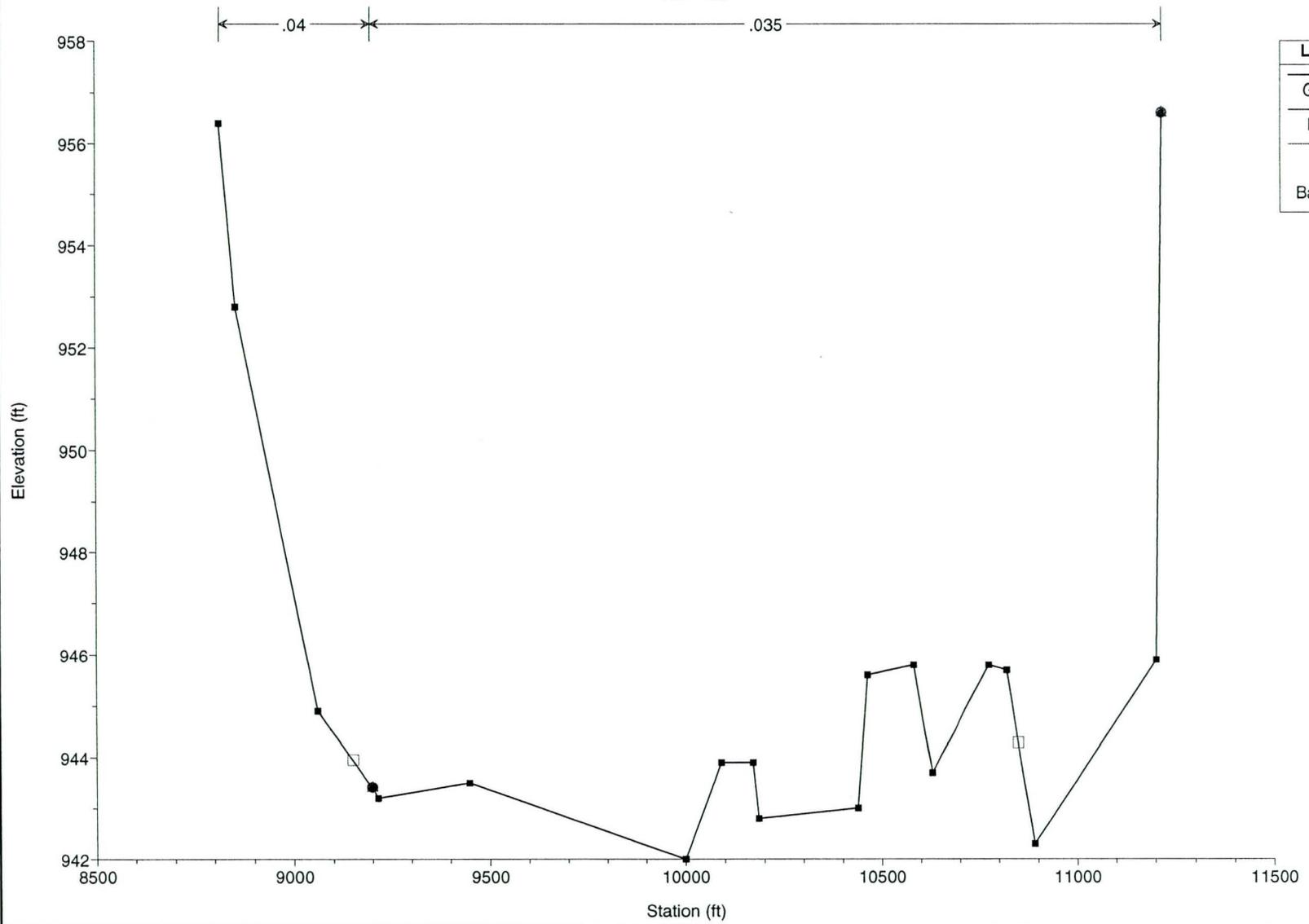
RS = 3.27



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

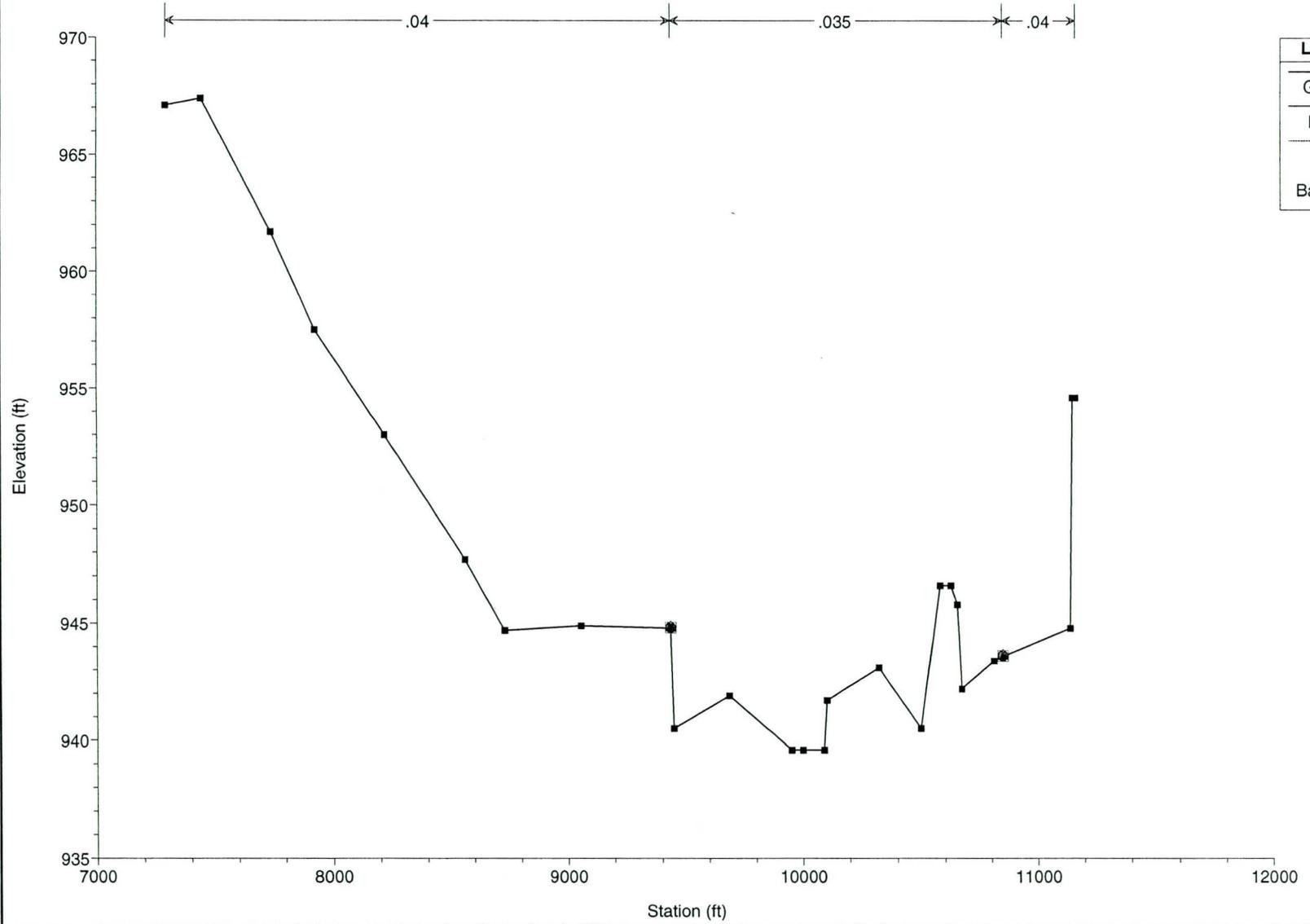
RS = 2.8



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

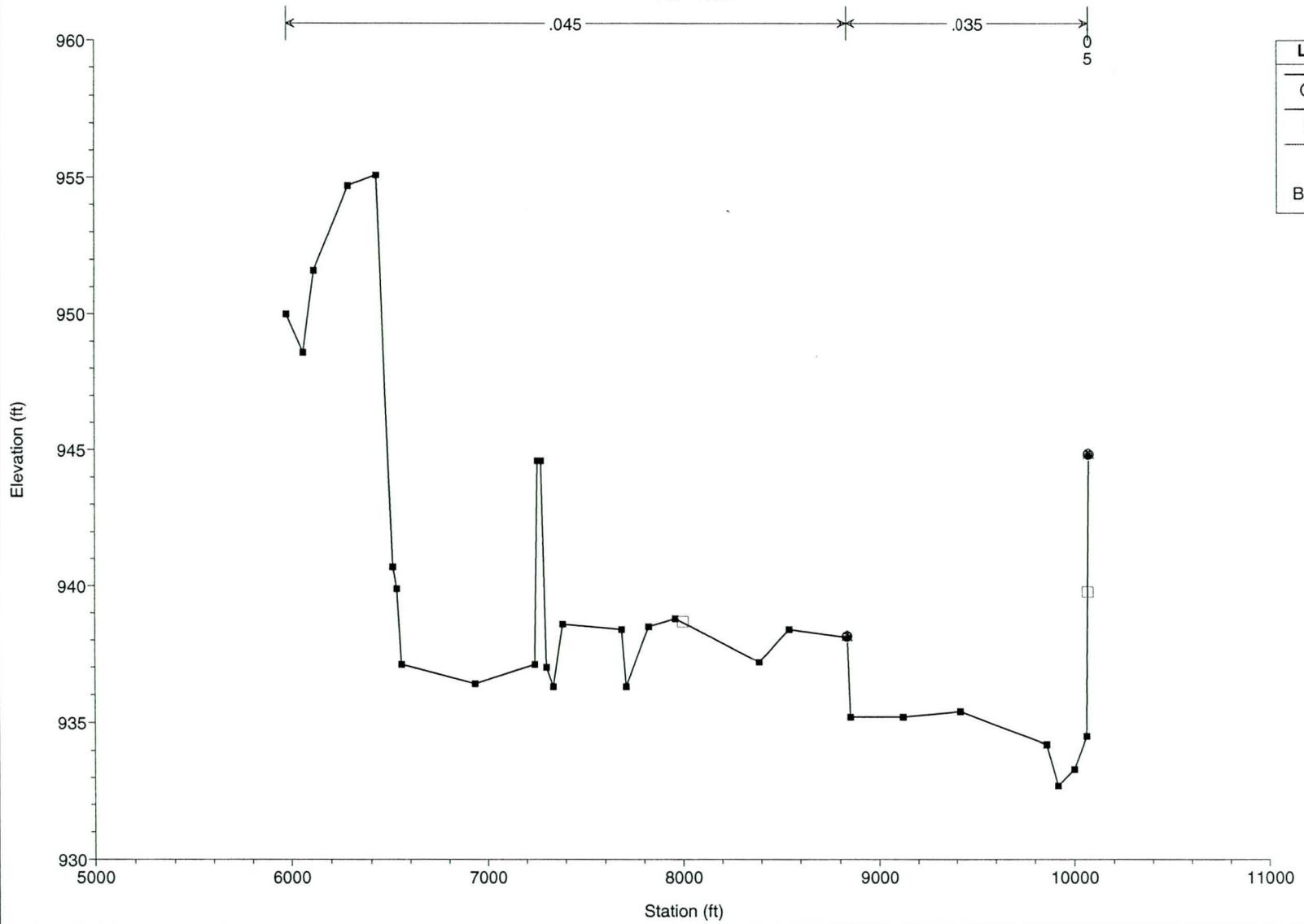
RS = 2.6



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

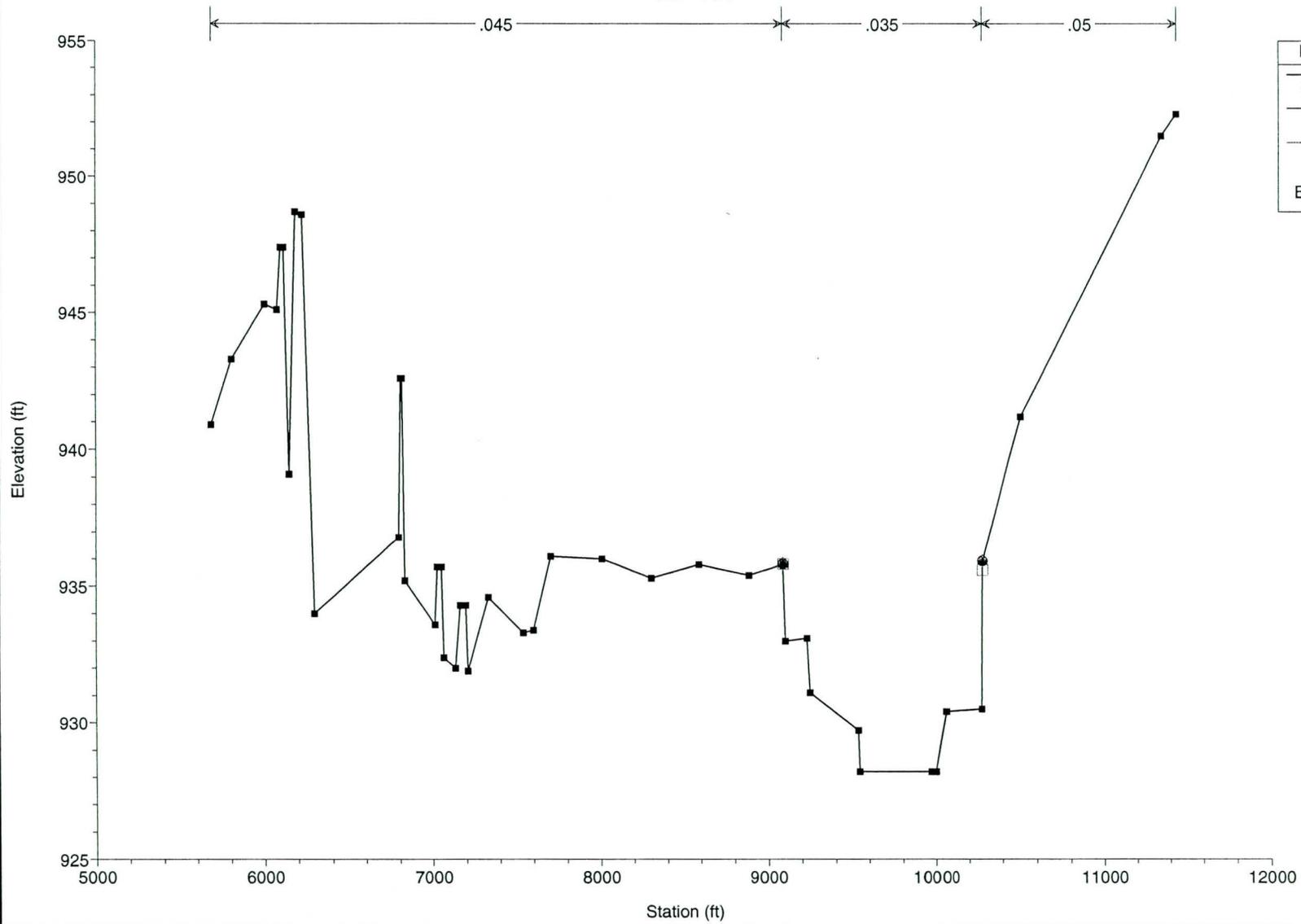
RS = 2.02



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

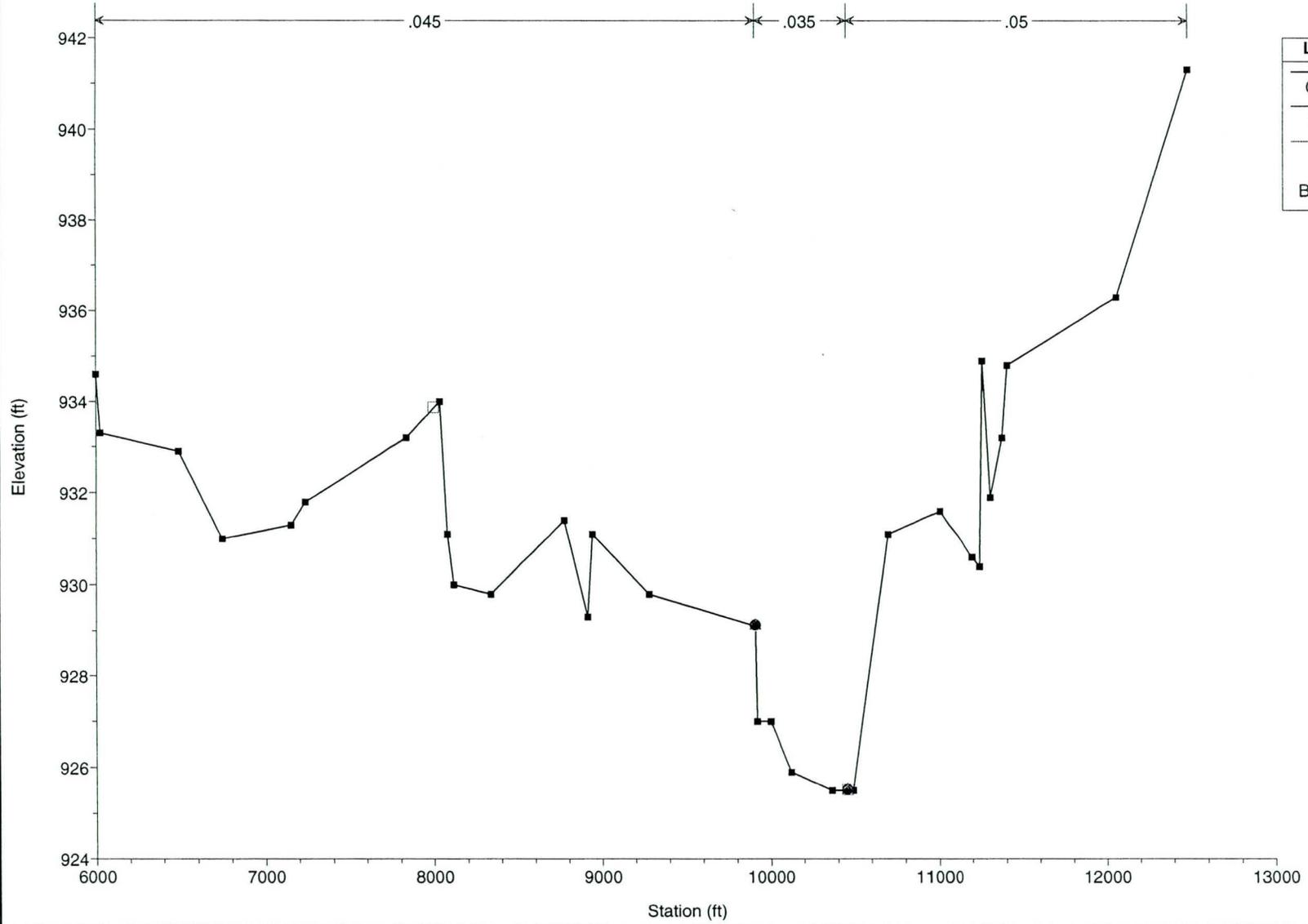
RS = 1.71



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

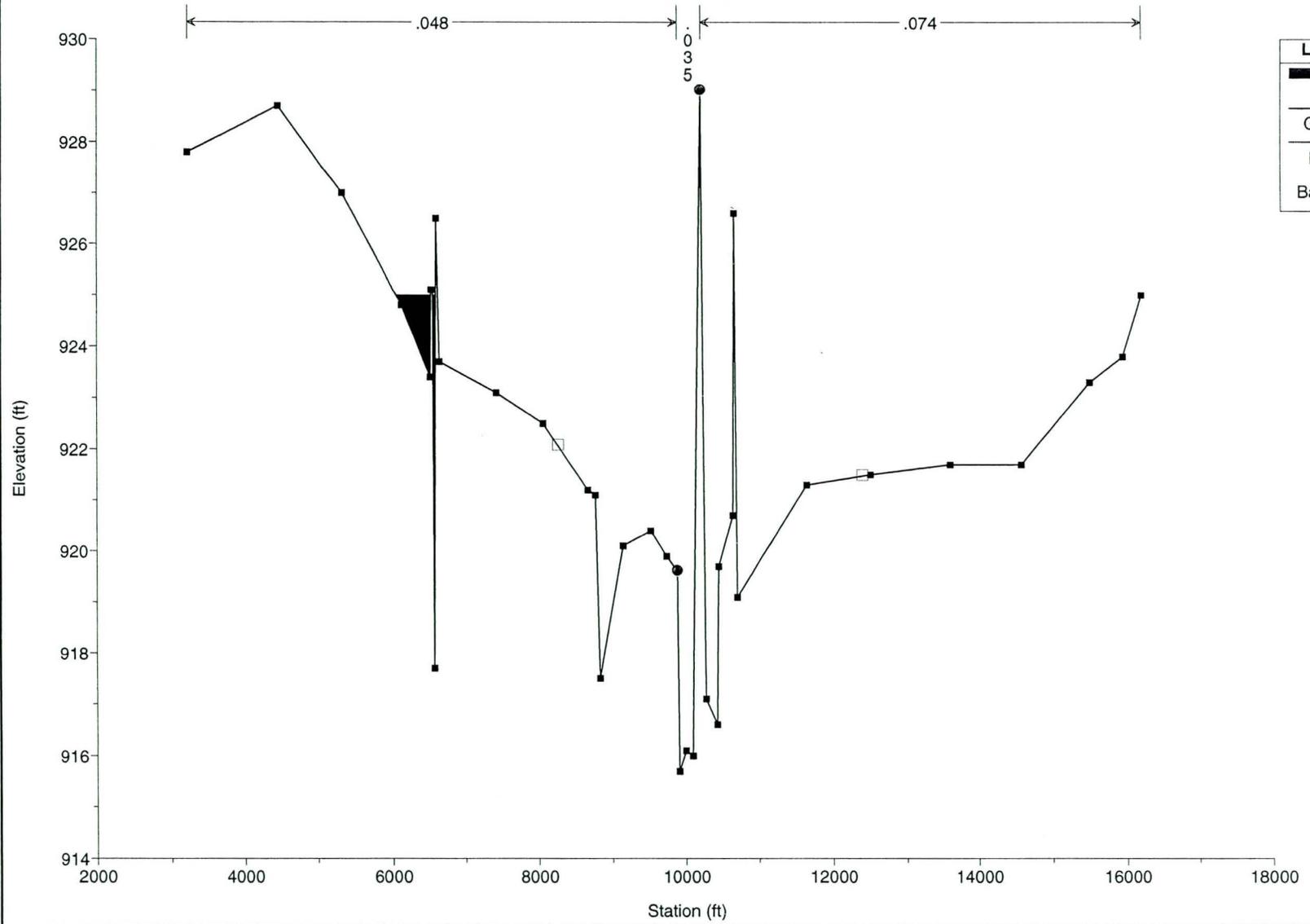
RS = 1.33



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

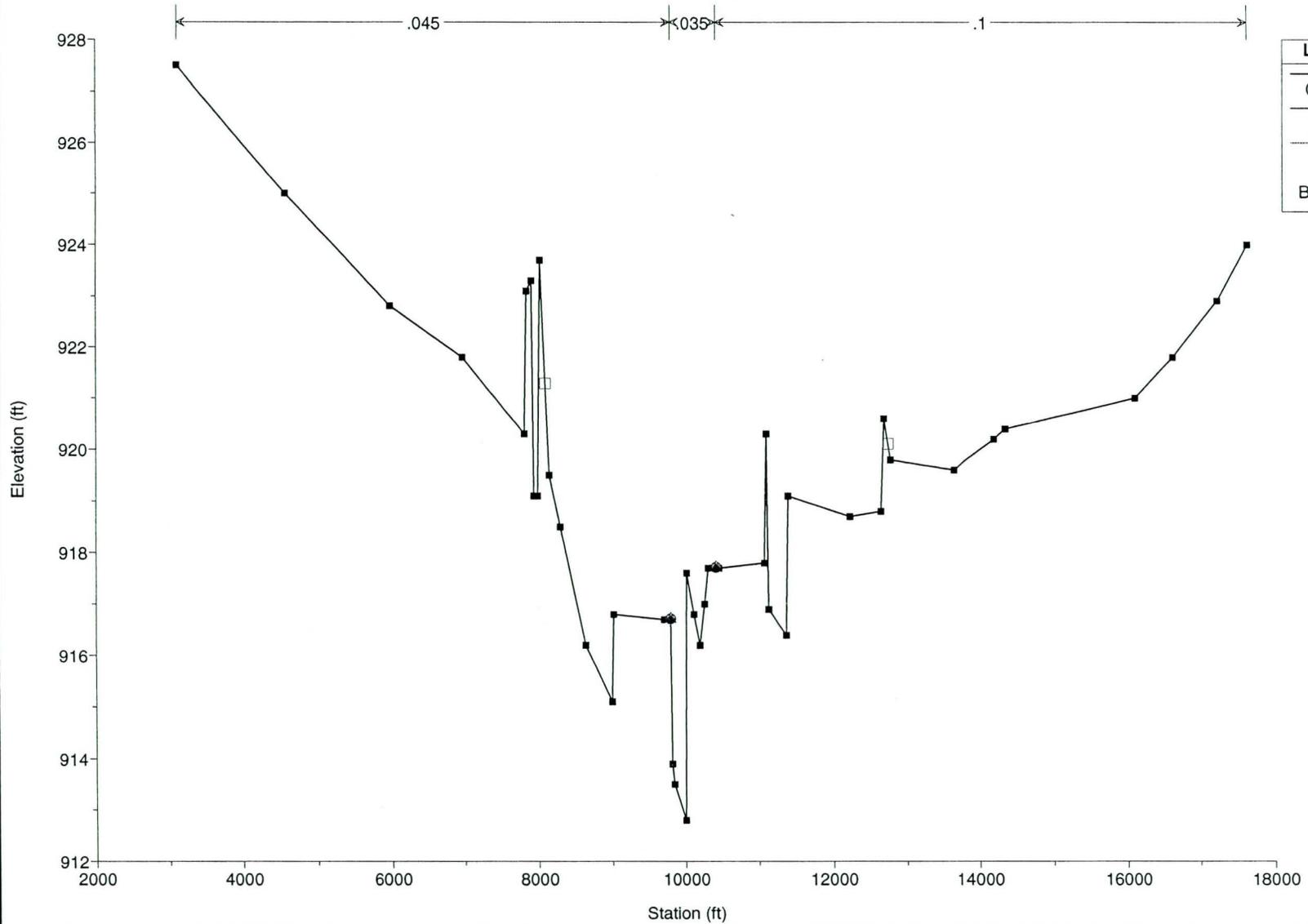
RS = 0.63



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 0.44



Agua Fria HEC-6 Review

Geom: Existing conditions with movable bed limits as levees

RS = 0.16

