

# Hohokam Area Drainage Master Study

## Phase I Study Report

Contract FCD 2009C029  
July 2012



Prepared for:



**Flood Control District  
of Maricopa County**

2801 West Durango Street  
Phoenix, AZ 85009

and



**City of Phoenix**

200 W. Washington St.  
Phoenix, AZ 85003

Prepared by:

**Stanley Consultants**

1661 East Camelback Rd, Suite 400  
Phoenix, AZ 85016



**Stanley Consultants** INC.

Contributions by:



LOGAN SIMPSON DESIGN INC.



**RG** *Civil Engineering Services*

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- Appendix A: Flood Insurance Rate Maps (FIRMs)
- Appendix B: Drainage Complaints Exhibit
- Appendix C: Seed Ideas

# 1. EXECUTIVE SUMMARY

## 1.1 PROJECT DESCRIPTION, PURPOSE & GOALS

The Hohokam Area Drainage Master Study/Plan (ADMS/ADMP) is a two-phase regional flood control planning project to determine the nature and magnitude of existing flood hazards; develop and evaluate potential flood mitigation alternatives; provide preliminary design plans for recommended improvements; and ultimately provide a comprehensive plan to address flooding within the study area and guide future development and flood control improvements.

This report documents Phase I, the Hohokam Area Drainage Master Study (ADMS). The Hohokam ADMS is a comprehensive data collection and investigation effort to identify and quantify existing and potential future flood hazards and document archeological, cultural, landscape, and recreational resources and opportunities that will serve as the basis to formulate and assess mitigation alternatives. The effort includes development of hydrologic/hydraulic models to simulate flooding conditions; data collection and site investigations; and public outreach to gather essential information on existing flooding conditions and to incorporate the issues, concerns and values of the public into the decision making process. In addition, stakeholder involvement and participation is included to keep them informed on the project, facilitate the data collection effort and to identify potential opportunities for flood control improvements.

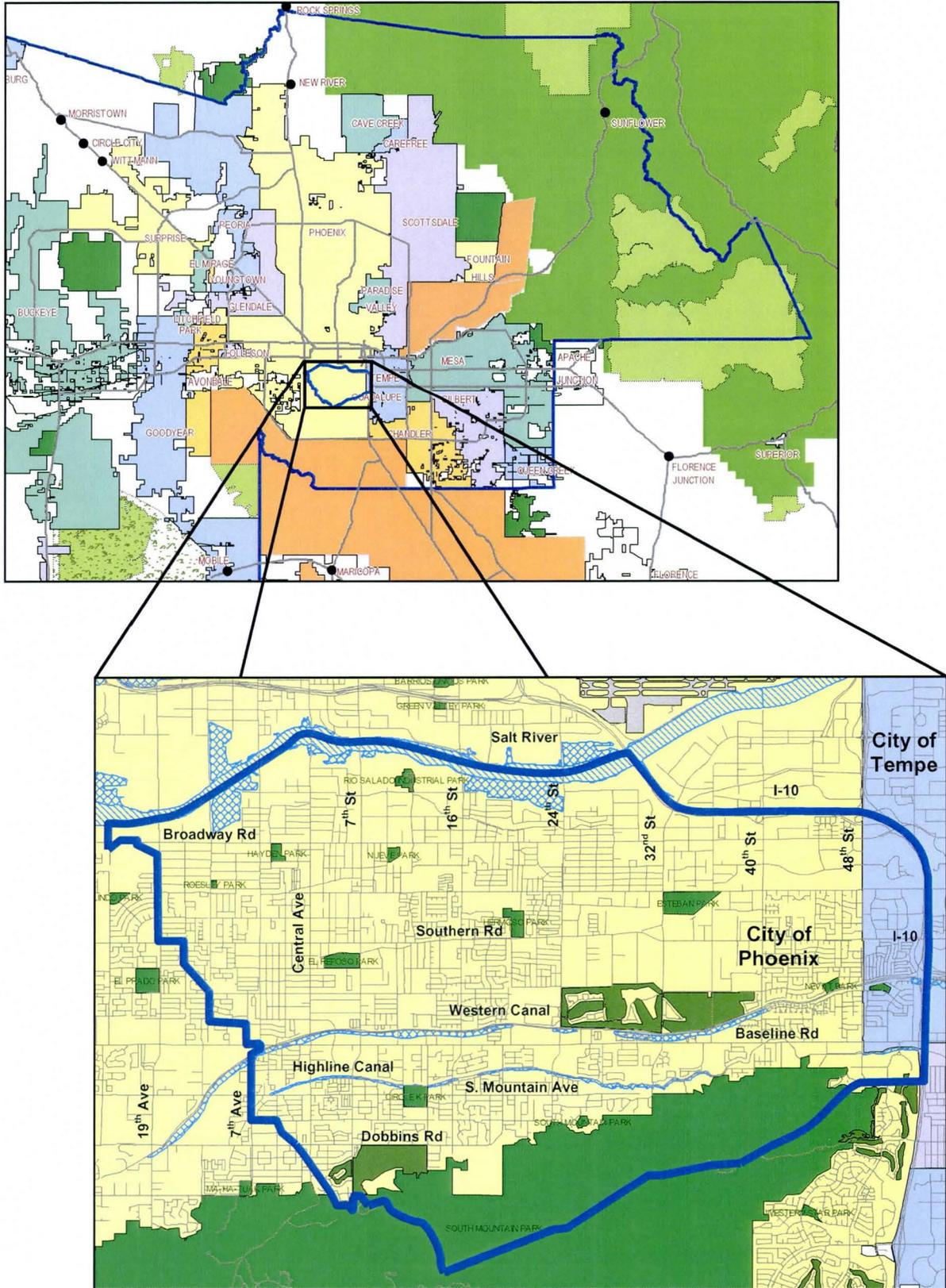
Phase II, the Hohokam Area Drainage Master Plan (ADMP), will utilize the results of the ADMS to formulate flood mitigation alternatives and through three levels of alternative formulation, analysis and evaluation, ultimately make recommendations for study area improvements. The ADMP will expand upon the public outreach and involvement efforts and develop concept plans for recommended improvements. Recommended improvements will be prioritized and a strategy for implementation prepared.

## 1.2 AUTHORIZATION

The Flood Control District of Maricopa County (FCDMC) authorized the performance of the Hohokam ADMS/ADMP under contract FCD 2009C029 with an effective Notice-to-Proceed date of May 12, 2010.

## 1.3 LOCATION

The Hohokam ADMS/ADMP study area is located within the limits of the City of Phoenix and the City of Tempe. The area is approximately 28.1 sq. miles in size and bounded by I-10 to the north and east, the Salt River to the north, South Mountain Park to the south and the limits of the Laveen ADMS to the west (see Figure 1-1).



**Figure 1-1: Project Location and Vicinity Map**

## 1.4 OVERVIEW OF STUDY AREA

The Hohokam study area is approximately 28 square miles in size and located north of South Mountain in south Phoenix and western Tempe. Historically, the area was largely agricultural land irrigated by a system of canals, however, while a few fields still remain, much of the agricultural land has been retired and replaced with residential housing, commercial development and industrial areas. The study area watershed generally drains to the north and west; from the South Mountains to the Salt River. Possibly due to the area's extended agricultural history, no continuous natural washes remain to drain the watershed to the Salt River, nor are there any continuous manmade conveyances except as provided by the City of Phoenix's (COP) storm drain system. Storm water not captured by the storm drain system, retained in basins, or impounded behind canals, is carried overland and along surface streets.

Among the most significant features in the area are the Salt River and South Mountain. South Mountain defines the southern boundary of the study area and is home to the South Mountain Park/Preserve. The park is 16,000 acres in size and one of the largest municipal parks in the country attracting an estimated 3 million visitors a year. This desert mountain park/preserve includes riding and hiking trails, picnic areas and scenic overlooks. The Salt River defines much of the northern boundary of the study area. Historically, the river and its floodplain were the lifeblood of the area providing water and fertile land for agriculture. Just after the turn of the century, the U.S. Bureau of Reclamation placed dams along the river to provide a reliable supply of water year round. The once flowing river became a dry riverbed that was soon occupied with sand and gravel mining activities. Today, much of the south overbank of the river within the study area is still being mined; however, recent efforts have been made to restore the riparian habitat that once existed along the river. The Rio Salado Habitat Restoration Area extends 5 miles along the Salt River from 28<sup>th</sup> St. to 19<sup>th</sup> Ave. Trash, from illegal dumping, and debris have been removed from this reach of the river and the banks have been terraced to provide areas for multi-use paths and environmental educational facilities. An extensive revegetation effort using native riparian plant material has been undertaken to restore critical habitat to the area.

Also among the most significant features in the watershed are the Salt River Project's (SRP) Western and Highline Irrigation Canals. The canals are roughly parallel to each other, draining from east to west, subdividing the study area and disrupting natural drainage patterns. As a result, approximate flood hazard zones have been delineated along the canals where ponding occurs.

## 1.5 HOHOKAM ADMS REPORTS

Several reports have been prepared as part of the Hohokam ADMS. These previous reports include:

- Hohokam ADMS Data Collection Report

- Hohokam ADMS/ADMP Class I Cultural Resources Inventory Survey
- Hohokam ADMS Pilot Study & Sensitivity Analysis Drainage Memorandum
- Hohokam ADMS Hydrology & Hydraulics Report

## 1.6 PARTICIPATION

Major project participants include the FCDMC, the City of Phoenix, the consultant team comprised of members from Stanley Consultants Inc. (SCI), JE Fuller Hydrology and Geomorphology (JEF), Logan Simpson Design (LSD), Riada Engineering (RE) and RG Engineering Services (RG). Additional stakeholders include the City of Tempe, Arizona Department of Transportation (ADOT), Salt River Project (SRP), and Valley Metro.

## 1.7 PROJECT NEED

South of the Highline Canal, the study area is relatively steep and experiences intense concentrated flooding as the result of runoff from the mountains. Residential properties located along, adjacent to, or in the path of this mountain runoff are subject to recurring flooding and property damage. North of Baseline Road or the Western Canal, the study area is flatter and experiences street flooding and shallow flooding of low lying areas including some residential properties and industrial areas. Ponding along the Western and Highline Canals is also problematic particularly where the canal embankments are a barrier to the natural flow of water to the north and west. Flooding also occurs immediately downstream of the canals, particularly along the Highline Canal where flow overtops the canal. These flooding conditions are supported by historic complaints received by the City of Phoenix and many are evident from storm events as recent as July 2008 and July 2010.

The need for flood control improvement and lack of adequate regional drainage and flood control facilities has been recognized in previous studies (Southeast Phoenix Storm Drainage Study, 1972), however, with the exception of the COP's storm drain system, no significant flood mitigation facilities exist. The Hohokam ADMS/ADMP will provide a comprehensive regional plan to address existing flood hazards and serve as a guide for future development and the planning of future flood control improvements.



7/31/10: 16th St & Highline Canal (SE from Gary Way)  
From Fox News website: Flooding along 16th St and water spilling to the west down Gary Way.



7/31/10: 16th St & Highline Canal (east along canal)  
Shallow flooding across canal. NE property flooded.



7/13/08: Cortland Point Subdivision  
Cortland Point block wall failure along Highline Canal.



7/13/08: Cortland Point Subdivision  
Flooding on Francisco Drive after storm. Ponding 1-1.5 feet in street.



7/31/10: Highline Canal along Pines at S. Mountain  
Flow entering subdivision through openings in block wall provided to pass offsite flow from along the canal.



7/31/10: 14th St & Highline Canal  
Flooding of property on 13th St just south of Circle K Park and Highline Canal (between 13th St and 14th St).

## 1.8 STORM DRAIN ASSESSMENT

With the exception of the 48<sup>th</sup> St. storm drain line which outfalls to the Tempe Drain, all study area storm drains outfall to the Salt River. Storm drain lines extend down all major north-south arterial streets from the Salt River or I-10 and upstream to Baseline Road. Laterals extending down many east-west major and minor arterial streets supplement the storm drain systems. With the exception of the 7<sup>th</sup> Avenue storm drain system, no storm drain system extends south of Baseline Road.

The storm drain assessment was based upon estimated pipe-full capacities of the existing storm drain pipes and flow captured by inlets modeled in the FLO-2D model. For the purpose of storm drain assessment, hydrology for the future conditions, 6-hour events were utilized. Overall, the existing storm drain system appears to be adequate for the 2-year event. For the 10-year event, much of the storm drain system appears to be adequate with the exception of main lines on 40<sup>th</sup> St. and 16<sup>th</sup> St. For the 100-year event, the storm drain system is generally inadequate with the exception of the North 16<sup>th</sup> St. mainline (separate system from the 16<sup>th</sup> St. main line) and the 48<sup>th</sup> St. main line. The results of the storm drain capacity assessment are summarized in Table 1-1.

**Table 1-1: Summary of Storm Drain Assessment**

Main Line	100-Year	10-Year	2-Year
19 <sup>th</sup> Avenue	Not adequate Broadway - Atlanta	Adequate	Adequate
7 <sup>th</sup> Avenue	Not adequate	Adequate	Adequate
Central Avenue	Not adequate Limits placed on upstream pipes & laterals due to lack of downstream pipe capacity	Likely adequate. (no negative system flows, but limits are reached upstream. Capacities might be exceeded if limits removed.)	Adequate
7 <sup>th</sup> Street	Not adequate. Roeser - Southern	Generally adequate. Minor under-capacity in one pipe	Adequate
16 <sup>th</sup> Street	Not adequate	Not adequate Southern – Roeser	Adequate
N 16 <sup>th</sup> Street	Adequate	Adequate	Adequate
24 <sup>th</sup> Street	Not adequate Vineyard - Southern	Adequate	Adequate
32 <sup>nd</sup> Street	Not adequate	Generally adequate. (minor negative system flow but limits reached in upstream pipes)	Adequate (no negative system flows and no limits exceeded)
40 <sup>th</sup> Street	Not adequate	Not adequate. (no negative system flows but limits reached on nearly all pipes)	Adequate (no negative system flows and no limits exceeded)
48 <sup>th</sup> Street	Generally adequate. Minor under capacity in one pipe	Adequate	Adequate
Basis of assessment based upon future land use, 6-hour conditions.			

## 1.9 IDENTIFIED FLOOD HAZARD AREAS

The identification of flood hazards in the study area is the result of multiple sources. Existing FEMA floodplains identify existing regulatory flood hazards. Drainage complaints lodged with the municipalities or received directly from residents from public meetings helped identify more detailed and specific problem areas. These complaints were investigated as part of the Data Collection Report to identify whether the issues were attributed to local conditions or more regional in nature. In addition, FLO-2D simulations substantiate the existence of existing known problem areas and identify potential flooding sources. The model results also helped identify other potential hazard areas that may not have been identified through the drainage complaints received. Utilizing these sources, 21 flood hazard areas were identified. These areas were subsequently grouped into eight large areas for purposes of the Phase I brainstorming sessions (see Figure 1-2).

## 1.10 CLASS I SURVEY SUMMARY

The Class I Cultural Resources survey of the study areas indicated that more than 200 surveys have been completed within the study area. Collectively, these surveys encompass 616 acres, or approximately 4 percent of the study area. Archaeological surveys identified 112 prehistoric and historic cultural resources. Prehistoric sites span the length of the Hohokam occupation in the Phoenix Basin. The types of sites range from artifact scatters and petroglyph sites, to agricultural sites associated with canals and field houses, to large village sites with cemeteries. The historic period sites include canals (Roosevelt Canal, Western Canal, San Francisco, Hayden, and North Branch Highline), multiple spurs of the Southern Pacific Railroad (Welton-Phoenix-Eloy and Tempe-West Chandler), and cemeteries, as well as artifact/trash scatters with and without features, and buildings.

The majority of previously recorded cultural resources have not been evaluated for their NRHP eligibility. Three historic properties are listed in the NRHP including the Niels Peterson House, the Phoenix Carnegie Library and Park, and the Ralph H. Stoughton Estate. Eight other cultural resources and five sites have been previously determined eligible for inclusion in the NRHP. For the remainder of the identified cultural resources, 4 were recommended for testing to determine NRHP-eligibility, 45 were recommended NRHP-eligible, and 60 are unevaluated or of unknown eligibility.

If existing or newly recorded NRHP-eligible cultural resources could be affected by drainage improvements, these resources shall be treated in a manner consistent with the Secretary of the Interior's Guidelines for the Treatment of Historic Properties, applicable Arizona statutes, and City of Tempe and City of Phoenix regulations.

## 1.11 CSFHM INVENTORY & ANALYSIS

The District's Context Sensitive Flood Hazard Mitigation (CSFHM) Planning and Design Approach was implemented as a part of the Hohokam ADMS. The CSFHM Approach is designed to serve as a framework and tool for the development of flood hazard mitigation plan alternatives that integrate the three basic required functions of being acceptable to local communities, compatible with landscape resources and effective in reducing flood losses. A context sensitive solution is one that is capable of performing all three of these basic functions.

The CSFHM model examines the relationship between three contexts: Community, Land & Resources, and Flooding. The Community Context was defined in this study based upon an analysis an inventory and analysis of the direction and vision contained in local community plans (see Section 6.1) and public sensing. The Land & Resources Context was defined, by the District, through an inventory and analysis of the valued characteristics of landscape resources that included scenery, recreation, and open spaces. The Flooding Context was defined through an analysis of flood hazards and public exposure to flooding. These analyses were utilized to identify the range of flood hazard mitigation structure types, structural methods, and landscape design themes (refer to Table 1-2) that are capable of simultaneously performing all three functions of a context sensitive solution.

The range of solutions that were identified through the application of the CSFHM Approach, served as the building blocks for development of context sensitive plan alternatives during the alternatives formulation stage of the study. The CSFHM analysis of the three contexts was also utilized as a baseline for evaluating the context sensitivity of the Brainstormed and Phase II Alternatives during the alternatives evaluation stage of the study.

**Table 1-2: Components of Flood Hazard Mitigation Solutions**

Structural Types	Structure Methods	Landscape Themes
• Natural Structure	• Natural	• Natural Sonoran Desert Upland
• Underground Pipe	• Soft Structural	• Natural Sonoran Desert Upland Riparian
• Channel Levee	• Semi-Soft Structural	• Natural Lower Sonoran Desert
• Conveyance Channel	• Enhanced Hard Structural	• Natural Lower Sonoran Desert Riparian
• Storage Basin	• Semi-Hard Structural	• Natural Sonoran Desert Hydro Riparian
• Dam	• Hard Structural	• Semi-Natural Sonoran Desert
		• Enhanced Desert
		• Desert Park
		• Desert Oasis
		• Urban Plaza

### 1.11.1 Community Context

The following is a summary of the flood hazard mitigation structure types, structural methods, and landscape design themes that were identified as acceptable based upon the inventory and analysis of the Community Context:

**Structure Type Acceptability.** All structure types except for a dam were determined to be acceptable throughout most of the study area (see Figure 6-2). In open space and park areas, all structure types would be acceptable with the exception of a channel levee and a dam. All structural types would be acceptable in areas comprised of heavy commercial and industrial land uses which may be found in the eastern portion of the study area.

**Structural Methods Acceptability.** Throughout the study area, the natural, soft structural, and semi-soft structural methods were generally determined to be acceptable (see Figure 6-3). Hard structural methods would be acceptable only in limited areas.

**Landscape Themes Acceptability.** Throughout most of the study area, all landscape themes would be acceptable except Natural Sonoran Desert Hydro Riparian and the Urban Plaza (see Figure 6-4). In the South Mountain area, the acceptable themes are limited to the Natural Sonoran Desert themes while generally most themes except the Natural Sonoran Desert Upland and Natural Sonoran Desert Upland Riparian themes would be acceptable.

### 1.11.2 Land and Resource Context

The following is a summary of the flood hazard mitigation structure types, structural methods, and landscape design themes that were determined to be compatible with landscape resources based upon the analysis of the Land & Resources Context:

**Structure Types Compatibility.** Natural structures are the only flood hazard mitigation structure types that are compatible within the floodway and flood fringe of the Salt River as well as the slopes of South Mountain (Figure 6-5). All structure types would be compatible over the majority of the study area, and in a few areas, all of the structure types, except for dam structures would be compatible with the land and resource context.

**Structural Methods Compatibility.** Similar to the structural types, natural is the only structural method that is compatible within the floodways, flood fringe areas, and the slopes of South Mountain (Figure 6-6). The Semi-Soft, Soft, and Natural Methods are compatible for a majority of the study area. Hard Structural Method is compatible in areas of heavy commercial or industrial use which are mostly located in the northern and eastern portions of the study area. All of the structural methods are compatible within the industrial valley plain landscape units.

**Landscape Design Theme Compatibility.** The Natural Sonoran Desert Uplands and Riparian landscape design themes would be compatible within the South Mountain area (Figure 6-7), while Natural Lower Sonoran Desert Riparian and Hydro-riparian landscape design themes would be compatible in areas along the Salt River. Within the developed areas throughout most of the study area, landscape design themes including the Semi-natural Desert, Enhanced Desert, Desert Park, Oasis and plaza themes would be compatible.

### 1.11.3 Flooding Context

The following is a summary of the flood hazard mitigation structure types, structural methods and landscape design themes that were determined to be effective in reducing flood losses based upon the analysis of the Flooding Context:

**Effective Flood Control - Structure Types.** All structure types except a natural structure would be considered effective over the majority of the area (see Figure 6-8). In the areas of riverine flow, underground pipe, channel levee and conveyance channels would be considered the most effective flood control structure types. In the South Mountain area, all structure types would be effective.

**Effective Flood Control - Structural Methods.** The simplicity of the drainage patterns and flooding types leads to a simple solution for developing effective flood control methods (see Figure 6-9). Except within the South Mountain Park, all flood control methods except a natural method could be effective. Within South Mountain Park all methods would be considered effective.

**Effective Flood Control - Landscape Themes.** For landscape themes, only the limited riverine flow areas would have some limitations on themes for the flooding context (see Figure 6-10). In those areas the Natural Sonoran Desert Upland and Upland Riparian Themes as well as the Desert Oasis and Urban Plaza themes would not be considered effective. Throughout most of the study area, all themes except the two natural Sonoran Desert themes would be considered effective for the flooding context. In the South Mountain Area, all landscape themes would be effective.

### 1.11.4 CSFHM Analysis and Results

In the final step of the application of the CSFHM Approach, the Comparative Analysis, information from the analysis of the Community, Land & Resources and Flooding contexts were combined, using GIS, to reveal flood hazard mitigation solutions (structure types, structural methods and landscape design themes) that are capable of concurrently meeting all three functional requirements of a context sensitive solution (see Figure 6-11). The following is a summary of the Structure Types, Structural Methods, and Landscape Design Themes that are context sensitive within discreet geographic areas of the project study area:

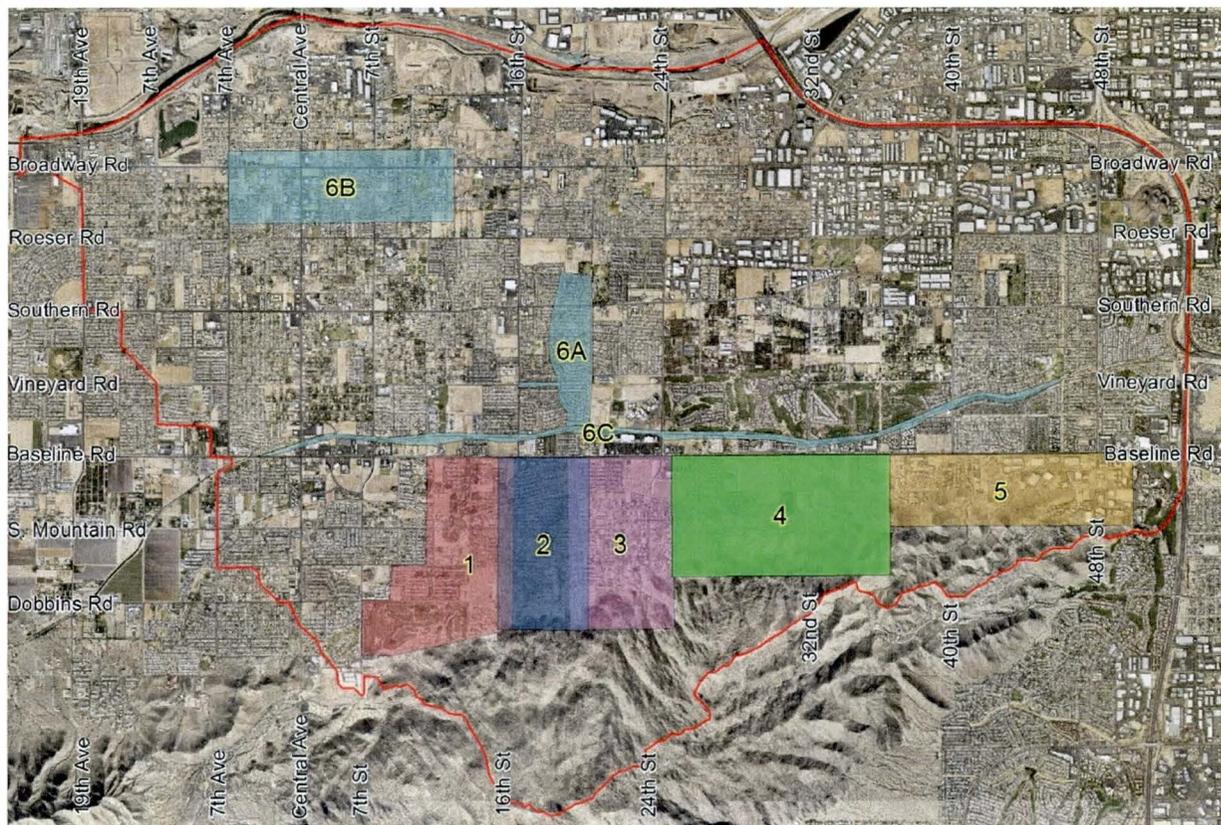
**Context Sensitive Structure Types.** The Underground Pipe, Channel Levee, Conveyance Channel and Storage Basin Structure Types will be context sensitive in most parts of the study area (see Figure 6-12). Only the Natural Structure Type will be context sensitive on the slopes of South Mountain.

**Context Sensitive Structural Methods.** The Natural, Soft, and Semi-Soft Structural Methods will be context sensitive in most parts of the study area (see Figure 6-13). The Enhanced Hard Structural Method will also be context sensitive within the North and Eastern parts of the study area and along some arterial streets.

**Context Sensitive Landscape Design Themes.** The large number of landscape design themes creates a somewhat complex array of landscape themes that would be context sensitive and part of a flood control solution. However, there is generally a good selection of landscape themes that would be considered context sensitive in most areas so that the landscape design would have the flexibility to meet a variety of site specific landscape conditions. In general the Natural Sonoran Desert Upland and Upland Riparian themes would be context sensitive in the South Mountain area (see Figure 6-14). Semi Natural Sonoran Desert, Enhanced Desert and Desert Oasis would be context sensitive in the bajada areas at the base of the mountain. For most of the study area, the Natural Lower Sonoran Desert themes, the Semi-natural Sonoran Desert, enhanced Desert and Desert Park would be context sensitive.

## 1.12 BRAINSTORMING ALTERNATIVES

Project team members, additional District personnel, and project stakeholders were invited and participated in the Phase I Brainstorming session. For purposes of the brainstorming session, eight areas were delineated based upon the identified flood hazards areas and potential source of the flooding issues (see Figure 1-2). At the session, the results of the data collection effort were provided including the general plans, cultural and land resources, study area opportunities and constraints and the results of the Landscape Inventory Analysis performed by the District. The results of the FLO-2D analyses were also presented including a FLO-2D animation to help define the problem areas and show potential flooding sources and contributing drainage areas. Participants were then divided into six working groups with each group assigned a brainstorming area with the exception of one group (Group 6) that was assigned three (Areas 6A, 6B and 6C). The groups then brainstormed ideas to address the flooding issues and presented them to the entire project team.



**Figure 1-2: Brainstorming Areas**

Alternatives developed during the brainstorming session are summarized below. Alone, the specific concept summarized may not achieve the desired flood mitigation but may be proposed as a component of an alternative comprised of multiple concepts.

### **1.12.1 Area 1: 7<sup>th</sup> St – 16<sup>th</sup> St (Circle K Park)**

- 1) Dobbins Rd Channel: Channel east-west along Dobbins Road as a means to capture and convey mountain runoff (9<sup>th</sup> St to 20<sup>th</sup> St)
- 2) 16<sup>th</sup> St/Dobbins Basin: Basin in vacant parcel northwest of 16<sup>th</sup> St and Dobbins
- 3) Basin south of Circle K Park: Purchase low density residential parcels south of the park for a basin
- 4) Circle K Park Basin: Rebuild/regrade Circle K Park to provide retention
- 5) Basin Northwest of Circle K Park: Basin in vacant parcel north of Highline Canal and west of park

- 6) Siphon under Highline Canal: Construct siphon under Highline canal (to convey floodwater to above basin north of Highline Canal)
- 7) Improvements to be included as part of golf course development: Require/work with future development of Thunderbird Country Club Golf Course/Vistal Development to assure and possibly to provide additional improvements for area flood mitigation (retention, channel)
- 8) Dobbins/20<sup>th</sup> St Basins: Basin in vacant area southeast of Dobbins/20<sup>th</sup> St (south of Siesta Foothills)

### **1.12.2 Area 2: 16<sup>th</sup> St – 20<sup>th</sup> St (Siesta Foothills/Boy Scout Camp)**

- 1) Boy Scout Camp Basin System (Eastern System):
  - a) Capture Boy Scout Camp Wash flows into a basin southeast of Dobbins
  - b) Convey to a larger basin
  - c) Convey from basins via new storm drain in 20<sup>th</sup> St.
  - d) Scalp peak with basins on either side of 20<sup>th</sup> St (or Highline Canal) subject to Baseline storm drain capacity
- 2) 16<sup>th</sup> St Basins and Storm Drain (Western System):
  - a) Basin upstream of Dobbins at 16<sup>th</sup> St
  - b) Route flows from basin under Dobbins via pipe to a basin northwest of 16<sup>th</sup> St and Dobbins (southeast of Euclid & 14<sup>th</sup> St).
  - c) Discharge basin into storm drain along 16<sup>th</sup> St
  - d) Discharge storm drain to Baseline Rd storm drain
- 3) Boy Scout Camp Offline Basin System (Eastern System):
  - a) Offline basin to boy Scout Camp Wash flows
  - b) Convey westerly via channel to Western System Alternative (above)
- 4) Boy Scout Camp Offline Basin System 2 (Eastern System):
  - a) Offline basin to boy Scout Camp Wash flows
  - b) Convey flow by storm drain to along Euclid and west to storm drain system in 16<sup>th</sup> St.

### **1.12.3 Area 3: 20<sup>th</sup> St – 24<sup>th</sup> St (Pines at South Mountain)**

- 1) String of Pearls System: Series of channel and basins along the existing flow path west of 23<sup>rd</sup> Pl. Take flow to basins at northwest corner of 24<sup>th</sup> St / Highline Canal.

- 2) Highline Canal Channel: Channel to deliver flow to a basin(s) at northwest corner of 20<sup>th</sup> St / Highline Canal and/or at northwest corner of 24<sup>th</sup> St / Highline Canal
- 3) Dikes along base of mountains: Dike system along base of mountains
- 4) Channel along base of mountains: Channel system along base of mountains
- 5) Mountain Dam: Dam across major flow west of 24<sup>th</sup> St in mountains.
- 6) Dam at 24<sup>th</sup> St: Dam at 24<sup>th</sup> St and Euclid
- 7) Canal Storm Drain System: Storm drain system parallel to Highline Canal. Discharge to extensions of the existing storm drain system south of Baseline Rd.
- 8) East String of Pearls: Series of basins that delivers flow to a big basin at Baseline and 24<sup>th</sup> St.
- 9) Increase park retention: Enlarge existing retention in park
- 10) Move/Remove Houses: Move all houses.
- 11) Spillways from Highline Canal: Provide spillways in Highline Canal to detention basins.

#### **1.12.4 Area 4: 24<sup>th</sup> St – 36<sup>th</sup> St (Cortland Point)**

- 1) 28<sup>th</sup> St & South Mountain System:
  - a) Basins south of Winston
  - b) Outfall storm drain 27<sup>th</sup> St to South Mountain Ave
  - c) Collector storm drains in South Mountain Ave
  - d) Outfall basins at Puerto Park
- 2) 32<sup>nd</sup> St & Highline Canal System:
  - a) Bleed-off storm drain to 24<sup>th</sup> St storm drain
  - b) Collector storm drain at South Mountain Ave from 30<sup>th</sup> St to 32<sup>nd</sup> St
  - c) Collector storm drain on south side of Highline Canal from 34<sup>th</sup> St-32<sup>nd</sup> St
  - d) Outfall storm drain and basin at 32<sup>nd</sup> St (basin west side of 32<sup>nd</sup> St north of Highline Canal.)
  - e) Bleed off storm drain on 32<sup>nd</sup> St north to Baseline

- 3) 36<sup>th</sup> St & Highline Canal System:
  - a) Small basins along the Highline Canal to either the north or south side from 34<sup>th</sup> PI to 36<sup>th</sup> St
  - b) Channel/floodwall on north side of Highline Canal from 34<sup>th</sup> PI to 36<sup>th</sup> St outfalling to basin(s)
  - c) Outfall storm drain north in 36<sup>th</sup> St to Baseline then west in Baseline to 32<sup>nd</sup> St storm drain.

#### **1.12.5 Area 5: 36<sup>th</sup> St – 48<sup>th</sup> St (South Mountain Industrial Area)**

- 1) 42<sup>nd</sup> St or 42<sup>nd</sup> PI / Baseline Basin: Basin at end of wash south of Baseline. Bleed off to existing storm drain in Baseline. Potential multi-use opportunity as a trailhead for park
- 2) Basin at Beverly Road South of Highline Canal: Basin southwest of 14<sup>th</sup> St and Vineyard

#### **1.12.6 Area 6A: 16<sup>th</sup> St – 20<sup>th</sup> St (North of Western Canal)**

- 1) Storm Drain: Provide a storm drain system along 18<sup>th</sup> St, 20<sup>th</sup> St and/or Vineyard to capture flows.
- 2) 16<sup>th</sup> St / Vineyard Basin: Basin in undeveloped area west of 20<sup>th</sup> St & south of Vineyard (undeveloped property belonging to South Mountain Community College)
- 3) 14<sup>th</sup> St / Vineyard Basin: Basin southwest of 14<sup>th</sup> St and Vineyard
- 4) 16<sup>th</sup> St / Vineyard Basin: Basin northwest of 16<sup>th</sup> St and Vineyard

#### **1.12.7 Area 6B: 3<sup>rd</sup> St & Broadway (Low Laying Area)**

- 1) Multiple Small Basins: Provide retention in available vacant parcels in general areas that bleed to the storm drain on Broadway or Central
- 2) Extend/expand storm drain into/in the area to alleviate flooding (and bleed basins): Extend the storm drain systems from Central and/or Broadway into area to capture flows and provide a means to bleed off multiple small retention basins.

#### **1.12.8 Area 6C: Ponding along the south side of Western Canal**

- 1) Basins along south side of Canal: Provide a storm drain system along 18<sup>th</sup> St, 20<sup>th</sup> St and/or Vineyard to capture flows.

- 2) No Action: Expectation that improvements upstream would address and/or alleviate ponding along the canal.

### 1.13 SEED IDEAS

Prior to the brainstorming session, 48 seed ideas were conceived for the twenty one flood hazard locations and also to address more systemic problems such as storm drain deficiencies, flooding along the Highline Canal and future development. These seed ideas were not presented to the brainstorming participants as part of the brainstorming session, so as not to influence or bias potential ideas generated by the attendees. Many of the seed ideas were similarly envisioned by the brainstorming groups, however, some were not, and consequently the seed ideas are included for further assessment during Phase II. A table is provided in Appendix C that briefly describes the seed idea and then shows which problem areas the idea would address and the nature of the impact on the flooding location.

### 1.14 LEVEL 1: ALTERNATIVE FORMULATION

For Level 1, the developed brainstorming and seed ideas generated in Phase I will be utilized, in part or in their entirety, to develop more comprehensive alternatives for each problem area to be evaluated for further consideration in Level 2. The preliminary analysis and evaluation of alternatives will be based upon information available from the Phase I hydrologic & hydraulic analyses and other data collection efforts. A preliminary evaluation matrix will be developed and used to help assess and identify alternatives for further development and assessment in Level 2.

## **2. INTRODUCTION**

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### **2.1 PROJECT DESCRIPTION, PURPOSE & GOALS**

The Hohokam Area Drainage Master Study/Plan (ADMS/ADMP) is a two-phase regional flood control planning project to determine the nature and magnitude of existing flood hazards; develop and evaluate potential flood mitigation alternatives; provide preliminary design plans for recommended improvements; and ultimately provide a comprehensive plan to address flooding within the study area and guide future development and flood control improvements.

This report documents Phase I, the Hohokam Area Drainage Master Study (ADMS). The Hohokam ADMS is a comprehensive data collection and investigation effort to identify and quantify existing and potential future flood hazards and document archeological, cultural, landscape, and recreational resources and opportunities that will serve as the basis to formulate and assess mitigation alternatives. The effort includes development of hydrologic/hydraulic models to simulate flooding conditions; data collection and site investigations; and public outreach to gather essential information on existing flooding conditions and to incorporate the issues, concerns and values of the public into the decision making process. In addition, stakeholder involvement and participation is included to inform significant area stakeholders, facilitate the data collection effort and to identify potential opportunities for flood control improvements.

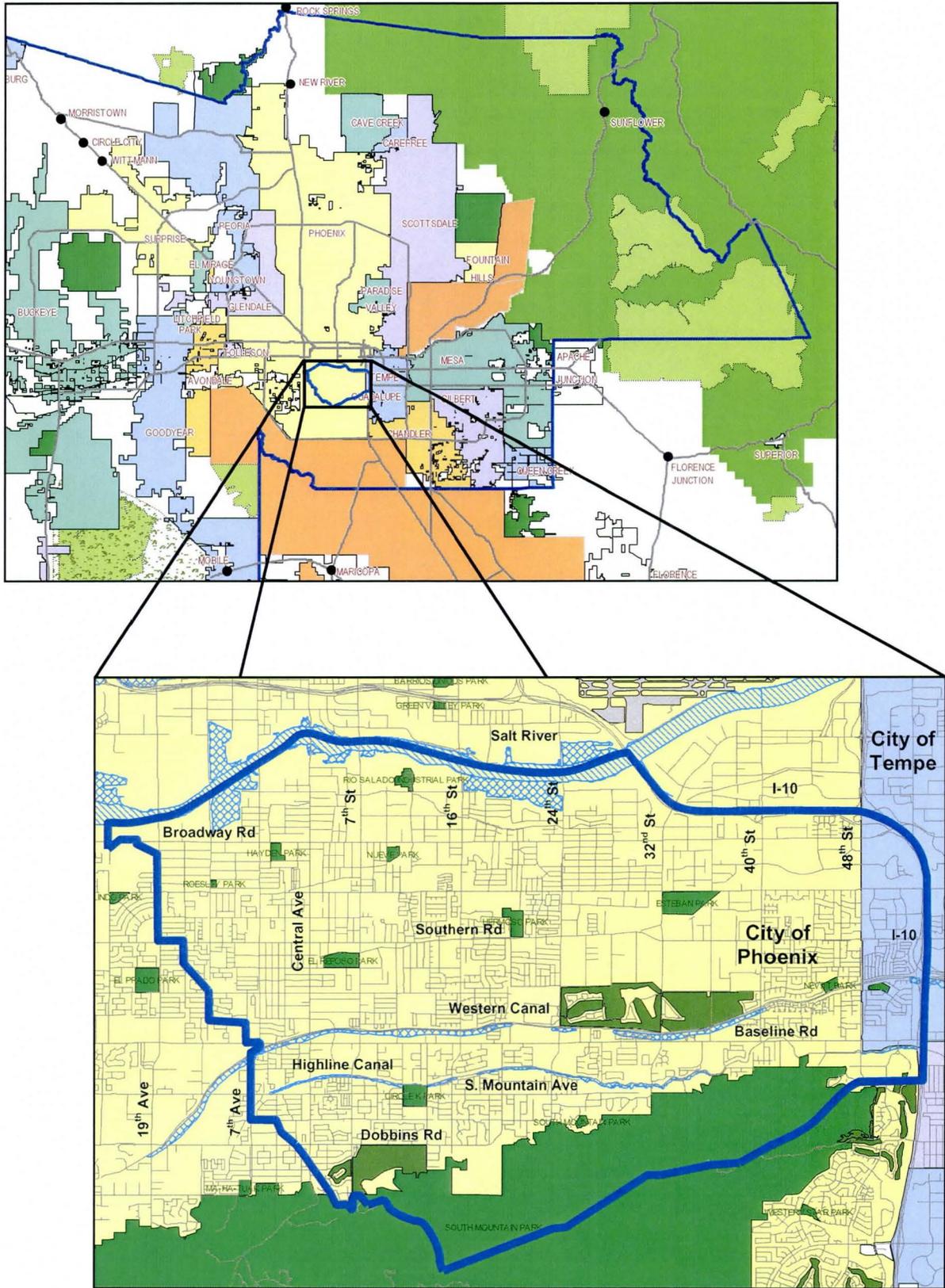
Phase II, the Hohokam Area Drainage Master Plan (ADMP), will utilize the results of the ADMS to formulate flood mitigation alternatives and through three levels of alternative development, analysis and evaluation, ultimately make recommendations for study area improvements. The ADMP will expand upon the public outreach and involvement efforts and develop concept plans for recommended improvements. Recommended improvements will be prioritized and a strategy for implementation prepared.

### **2.2 PROJECT AUTHORIZATION**

The Flood Control District of Maricopa County (FCDMC) authorized the performance of the Hohokam ADMS/ADMP under contract FCD 2009C029 with an effective Notice-to-Proceed date of May 12, 2010.

### **2.3 PROJECT LOCATION**

The Hohokam ADMS/ADMP study area is located within the corporate limits of the City of Phoenix and the City of Tempe. The area is approximately 28.1 sq. miles in size and bounded by the I-10 to the north and east, the Salt River to the north, South Mountain Park to the south and the Laveen ADMS to the west (see Figure 2-1).



**Figure 2-1: Project Location and Vicinity Map**

## 2.4 PROJECT OVERVIEW AND HISTORY

The Hohokam study area is located north of South Mountain in south Phoenix and western Tempe. Being located along the Salt River, the area has a long history of agricultural cultivation dating back to the Hohokam culture over 500 years ago. Possibly due the area's extended history of agriculture and its gradual urban conversion, no continuous natural washes remain to drain the watershed to the Salt River, nor are there any continuous manmade conveyances except as provided by the City's storm drain system. Among the most significant features in the watershed are the SRP's Western and Highline Irrigation Canals. The canals are roughly parallel to each other, draining from east to west across the study area and approximate flood hazards have been delineated along the canals due to the disruption of the natural pattern of flow to the north and west from South Mountain to the Salt River and ponding behind the canals.

During intense storm events, the flatter lower portion of the study area (roughly north of the Western Canal) experiences more widespread shallow flooding in the streets and low laying areas. The steeper upper portion of the watershed located along the foothills of South Mountain is directly impacted by runoff from the South Mountain and experiences more intense concentrated flooding along streets, remnant washes and historic flow paths. These flooding conditions are documented in historic complaints received by the City of Phoenix and evident from recent storm events including July 2008 and July 2010.

Lack of adequate regional drainage and flood control facilities has previously been recognized. The most comprehensive study of the area was the Southeast Phoenix Storm Drainage Study conducted in 1972. This study provided conceptual design of large diameter storm drain trunk lines down the major north/south arterial streets and also recommended the construction of several regional detention basins located upstream of the Highline Canal. While storm drain lines have been constructed along the major arterial streets from the Salt River to Baseline Road, with the exception of a large basin location on Central Ave, at the entrance to South Mountain Park, the recommended detention basins were never constructed.

In 1997, a similar study, the South Phoenix-Laveen Drainage Improvement Project, was conducted that included the portion of the Hohokam study area west of Central Avenue. Similar recommendations were made (storm drain and regional detention), however, the focus was primarily on drainage issues in the Laveen area and few improvements were proposed within the current Hohokam study area.

## 2.5 HOHOKAM ADMS REPORTS

Several reports have previously been prepared as part of the Hohokam ADMS. These previous reports include:

### **2.5.1 Hohokam ADMS Data Collection Report**

The Data Collection Report documents and summarizes the data collection effort and presents pertinent data collected for the Hohokam ADMS. The report includes:

- Landscape Inventory Analysis
- Context Sensitive Flood Hazard Mitigation Planning and Approach
- Study Area General Plans and Specific Plans (summarized)
- Study Area Drainage Complaints and Site Investigations
- Study Area FIRMs
- Summary of Previous Study Area Reports
- Public Meeting Comments.

### **2.5.2 Class I Cultural Resources Inventory Survey**

The Class I Cultural Resources Inventory Survey of 16,000 acres for the Hohokam Area Drainage Master Plan documents the results of Logan Simpson Designs (LSD) investigative effort into the cultural resources within the study area and beyond its limits for 1 mile.

### **2.5.3 Pilot Study & Sensitivity Analysis Drainage Memorandum**

The Drainage Memorandum documents and summarizes the results and conclusions of the FLO-2D pilot study. The pilot study consisted of hydrologic modeling of a smaller representative portion of the Hohokam ADMS study area for the purpose of determining the methodologies and approaches to be used for the development of the final study models for the entire study area. As part of the pilot study, sensitivity analyses were performed to assess the impact of various hydrologic parameters, model variables and hydraulic features. The recommendations of the pilot study were not necessarily adopted in the final study models due to further refinement of the modeling process and FLO-2D software.

### **2.5.4 Hohokam ADMS Hydrology & Hydraulics Report**

The Hydrology and Hydraulics Report documents the assumptions, approaches and results of the study area FLO-2D hydrologic models and hydraulic analyses. It includes a discussion of the model verification efforts, identification of potential problem areas, and provides an assessment of the study area storm drain system based upon the FLO-2D analyses and full-pipe capacity approach used for the study.

## **2.6 PROJECT PARTICIPATION**

The FCDMC and the City of Phoenix (COP) are the primary agencies intimately involved in project activities. The consultant team included staff members from Stanley

Consultants Inc. (SCI), JE Fuller Hydrology and Geomorphology (JEF), Logan Simpson Design (LSD), Riada Engineering (RE) and RG Engineering Services (RG).

## **2.6.1 Study Contacts**

In addition to the primary agencies and project team, the following list of study contacts and stakeholders were instrumental in the collection of project data and the conduction of project activities.

### **City of Phoenix**

Engineering Records  
200 W Washington, Phoenix, AZ 85003

Neighborhood Services  
200 W Washington, Phoenix, AZ 85003  
Contact: Ray Buchanan (602) 534-2274

Parks & Recreation  
200 W Washington, Phoenix, AZ 85003  
Contact: Mike Bornhoeft (602) 262-4925

Street Transportation Department – Floodplain Management  
200 W Washington, Phoenix, AZ 85003  
Contact: Hasan Mustaq (602) 262-4026

Street Transportation Department – GIS  
200 W Washington, Phoenix, AZ 85003  
Contact: Robert Marsh (602) 534-1552

Village Planning – South Mountain  
200 W Washington, Phoenix, AZ 85003  
Contacts: Joshua Bednarek (602) 262-6823

### **City of Tempe**

Public Works Department - Engineering Division  
31 East Fifth St, Tempe AZ 85281  
Contact: Donna Sullivan-Hancock (480) 350-8341

Public Work Department – Neighborhood Services Division  
31 East Fifth St, Tempe AZ 85281  
Contact: Elizabeth Thomas (480) 350-8223

**Flood Control District of Maricopa County (FCDMC)**

GIS Branch  
2801 W Durango St Phoenix AZ 85009  
Contact: Eric Feldman (602) 506-8736

Hydrology/Hydraulics Branch  
2801 W Durango St Phoenix AZ 85009  
Contact: Julie Cox (602) 506-8401

Engineering Special Projects Branch  
2801 W Durango St Phoenix AZ 85009  
Contact: Tom Loomis (602) 506-4767

**Maricopa Association of Governments (MAG)**

302 N 1<sup>st</sup> Ave, Phoenix AZ 85003  
Contact: Tim Strow (602) 254-6300

**Salt River Project**

Water Engineering  
P.O. Box 52025, Phoenix AZ 85072-2025  
Contact: Bob Gooch (602) 236-5227

Northside Water O & M  
P.O. Box 52025, Phoenix AZ 85072-2025  
Contact: Dave Kieffer (602) 236-4954

**Southwest Gas**

10851 N Black Canyon Highway, Phoenix AZ 85029-4755  
Contact: Greg Cooper (602) 484-5276

**Valley Metro/Metro Light Rail**

101 N 1<sup>st</sup> Ave, Suite 1100, Phoenix AZ 85003  
Contact: Robert Forest (602) 322-4514

**2.6.2 Stakeholders**

In addition to assisting in the data collection effort, stakeholders provided input regarding their concerns and shared information on existing, ongoing and future projects in the study area.

### **City of Phoenix (COP)**

The COP has jurisdictional authority over most of the study area including South Mountain Park. In addition to the Street Transportations – Floodplain Management Department, other city departments are considered area stakeholders including neighborhood services, parks & recreation, and planning.

The Street Transportation – Floodplain Management Department provided 32 flooding complaints received by the city dating back to before 2000. In addition, the city provided an Emergency Storm Report for a July 31 to August 1, 2010 storm event. This report identified 19 additional areas in which city personnel were deployed to address various storm related events ranging from vactoring/cleaning storm sewers/catch basins to debris removal from streets.

Among the COP's concerns are to remove, if possible, residents from identified flood hazard zones along the Western and Highline Canals by removing or reducing the floodplain limits. The COP is also interested in the capacity of the existing storm drain system to determine their effectiveness, capacities and identify potential deficiencies.

### **City of Tempe (COT)**

The study area east of 48<sup>th</sup> St. is located within the COT. The city has provided information on the city's storm drain and assistance in identifying neighborhood contacts for purposes of public involvement. The city has no database that documents flooding complaints within the study area.

### **Salt River Project (SRP)**

SRP operates and maintains both the irrigation facilities as well as the power utilities in the study area. SRP has provided operational information and access on its irrigation facilities. SRP expressed interest in the approaches being used to model their irrigation facilities, primarily the Western and Highline Canals, and have recommended that any models consider the canals to be at bank full capacity.

### **Arizona Department of Transportation (ADOT)**

ADOT holds jurisdiction within the ADOT ROW along the I-10 corridor. ADOT expressed no significant concerns with the Hohokam ADMS. ADOT currently is conducting a Design Concept Study and Environmental Impact Study (EIS) for improvements along the I-10 corridor from SR51 to SR202. Concept designs are due in the spring of 2012 and proposed for construction in 2013. Proposed improvements will not have a significant impact on the Hohokam ADMS.

ADOT, the COP and the COT have an intergovernmental agreement (IGA) for discharges to the Tempe Drain, the outfall for the 48<sup>th</sup> St storm drain line and the only

outfall that is not to the Salt River. The IGA stipulates the maximum amount of flow each agency can discharge into the Tempe Drain.

### **Valley Metro**

Metro Light Rail will be conducting a feasibility study to extend a Metro Light Rail line into the South Phoenix area in the future. It is not foreseen that the study or any potential feasible alignments will be adequately completed or defined to consider in the Hohokam ADMS/ADMP. The most likely alignment for any extension would likely be along a Central Avenue corridor.

## **2.7 INTERAGENCY COORDINATION**

### **2.7.1 City of Phoenix (COP)**

The City of Phoenix primary concerns are to identify potential inadequacies in the COP storm drain system, define existing flood hazards and to address known flooding issues in the study area. In particular, those documented historically in flooding complaints and flooding issues in the upper portion of the study area related to mountain runoff and flooding along the Highline and Western Canals.

### **2.7.2 City of Tempe (COT)**

The City of Tempe has no existing drainage facilities in the study area and has not data base or record of drainage complaints lodged within the study area. The COT, however, is interested in identifying potential flood hazards and drainage inadequacies.

### **2.7.3 Arizona Department of Transportation (ADOT)**

ADOT has not expressed any specific concerns about flooding and/or drainage from the Hohokam study area. Interstate-10 which bounds the study area to the north and east is generally hydrologically separated from study area by raised earthen embankments and the freeway drainage infrastructure is generally isolated with the exception drainage to the Tempe Drain. ADOT, the COP and the COT have an intergovernmental agreement (IGA) that stipulates the maximum amount of flow each agency can discharge into the Tempe Drain. The Tempe Drain serves as the outfall for the 48<sup>th</sup> St storm drain line. Of potential concern might be any potential improvements to the 48<sup>th</sup> storm drain line that might increase discharge from the 48<sup>th</sup> St storm drain line above the rate stipulated by the IGA.

### **2.7.4 Salt River Project (SRP)**

SRPs primary concerns involve the Western and Highline Canals. Flooding along the canals, primarily the Highline Canal, is problematic not only for maintenance and

operation but also from a public relations perspective. The Highline Canal intercepts offsite runoff along its length and is frequently overtopped during intense storm events. Overtopping of the canal causes erosion and is a continual maintenance issue in some areas. In addition, flooding issues downstream of the canal is often attributed to the canal itself and/or considered to be a contributing factor. SRP practice is to drain the Highline Canal prior to the onset of large storm events.

SRP expressed some concern that hydrology models might reflect that the canals provided flood storage and provide some attenuating effects. Consequently, the incisions of the canals themselves were removed from the model topography and the models reflect a wide flat ground surface at the elevation of the embankments.

## 3. EXISTING FACILITIES & ASSESSMENTS

### 3.1 WATERSHED & WASHES

The study area watershed generally drains to the northwest; from the South Mountains to the Salt River. Possibly due the area's extended history of agriculture and its gradual urban conversion, no continuous natural washes remain to drain the watershed to the Salt River, nor are there any continuous manmade conveyances except as provided by the COP's storm drain system. Storm water not captured by the storm drain system, retained in basins or impounded behind canals is carried overland and along surface streets.

The upper portion of the watershed (roughly south of the Highline Canal) is more steeply sloped and primarily residential developments. This area is directly impacted by runoff from the South Mountains. Some washes descend out of the mountains and continue through developed areas but they quickly dissipate and essentially disappear prior to the Highline Canal. The lower portion of the watershed (north of the Western Canal) is much flatter and surface drainage is almost exclusively carried overland or along streets.

### 3.2 REGIONAL RETENTION

Within the study area there are no large regional retention facilities owned by the FCDMC, the COP or the COT. The COP owns a large regional retention basin located at the entrance to South Mountain Park on Central Ave and just outside the study area. The basin is the outfall for a large watershed that extends south into South Mountain Park. An initial assessment of the retention basin's capacity and operation performed at the outset of the project determined that the basin had sufficient volume to retain the 100-yr, 6-hr and the 100-yr, 24-hr events and that any flow overtopping the basin would drain to the northwest to the Laveen Study Area. This retention basin is currently being studied in more detail with new mapping and survey as part of the District's South Phoenix Two Basins Project (FCD 2011C008). For the purposes of this project, the retention basin is not considered part of this study.

Small local retention basins are located throughout the study area; however, they are primarily constructed to meet development requirements for onsite retention. North (downstream) of the Highline Canal, some developments have designed open space for conveyance and retention to try to help control flow that might overtop the canal (e.g. Groves at South Mountain, Las Colinas, and Pines at South Mountain). However, two large retention basins are located in the Dobbins Creek subdivision (Dobbins Rd and 10<sup>th</sup> St). These basins provide onsite retention for the development but also retain mountain runoff that passes through the Thunderbird Country Club Golf Course (TCCGC). The TCCGC itself provides some additional retention.

### 3.3 STORM DRAIN SYSTEM

#### 3.3.1 Existing Storm Drain System

With the exception of the 48<sup>th</sup> St storm drain line which outfalls to the Tempe Drain, all study area storm drain outfall to the Salt River. Storm drain lines extend down all major north-south arterial streets from the Salt River or I-10 and upstream to Baseline Road. Laterals extending down many east-west major and minor arterial streets supplement the storm drain systems. With the exception of the 7<sup>th</sup> Avenue storm drain system, no storm drain system extends south of Baseline Road (See Figure 3-1). ADOT, the COP and the COT have an IGA for flow discharges to the Tempe Drain, the outfall for the 48<sup>th</sup> St. storm drain line and the only outfall that is not to the Salt River. The IGA stipulates the maximum amount of flow each agency can discharge into the Tempe Drain. With the exception of the relatively minor drainage improvements constructed in conjunction with roadway improvement projects, there are no known plans to significantly upgrade or improve the existing storm drain system within the study area.

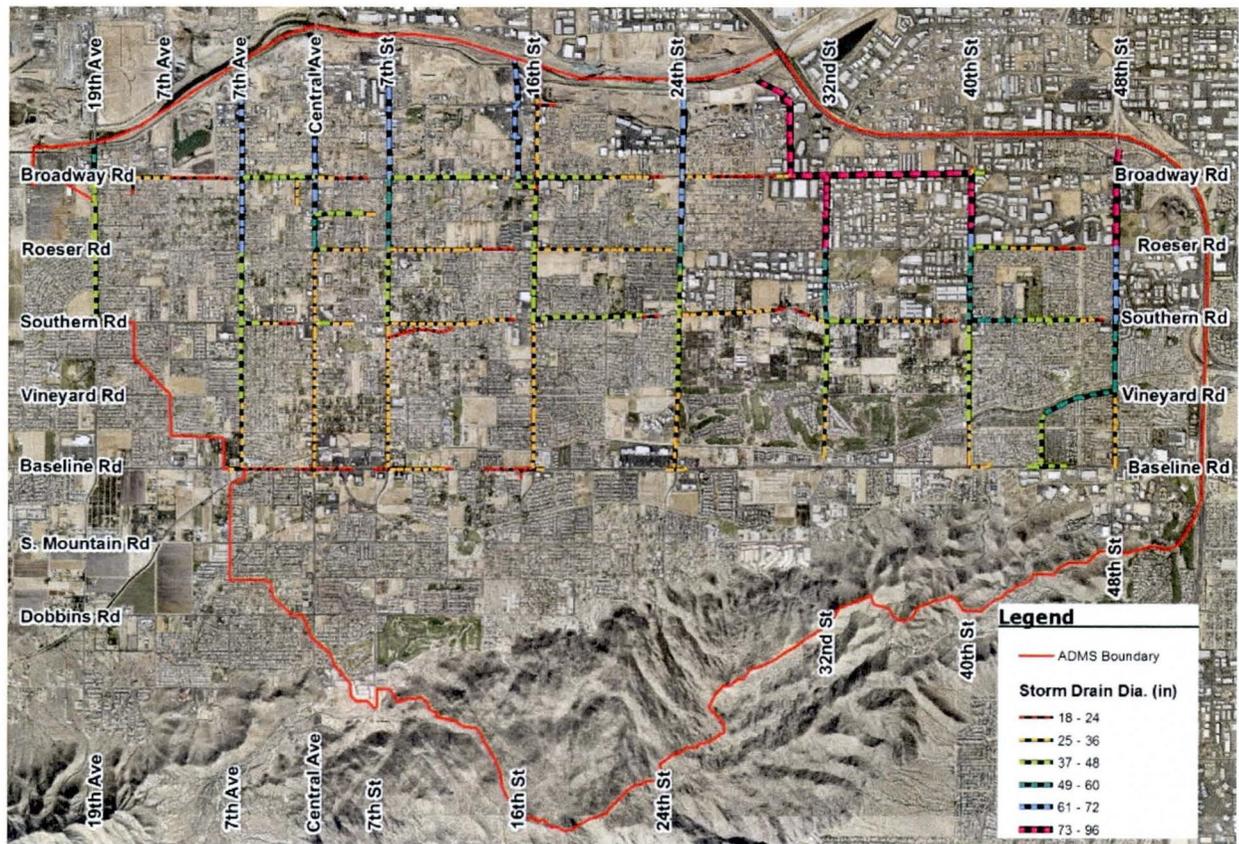


Figure 3-1: Study area storm drain system.

### 3.3.2 Assessment Considerations

There are a number of caveats when considering the storm drain assessments that follow. Most important is that FLO-2D is not currently well suited for integrated hydraulic/hydrologic modeling of complex storm drain systems and the approach used requires a number of important assumptions discussed in the Hohokam ADMS H&H Report. It is recommended that any subsequent project alternatives that include storm drain improvements either to address any perceived storm drain inadequacies and/or to serve as an outfall for flood mitigation alternative perform a more detailed hydraulic assessment of the impacted system utilizing more conventional hydraulic analyses and more accurate topographic data.

### 3.3.3 Storm Drain Assessment

Details of the storm drain capacity assessment are provided separately in the Hohokam ADMS Hydrology & Hydraulics (H&H) Report. The storm drain assessment was based upon estimated pipe full capacities of the existing storm drain pipes and flow captured by inlets modeled in the FLO-2D model. For purpose of storm drain assessment, hydrology for the future conditions, 6-hour events were utilized. Overall, the existing storm drain system appears to be adequate for the 2-year event. However, storm drain inadequacies arise for the 10-yr and 100-yr events. The assessments of the storm drain systems are briefly described in the sections below and summarized in Table 3-1.

#### 3.3.3.1 2-Year Events

For the 2-year events, all storm drain systems have adequate capacity to convey the flow. No pipes exceed their estimated pipe capacity and where pipes are limited to less than their estimated pipe capacities, the captured flows are less than the capacity restriction placed on the pipes.

#### 3.3.3.2 10-Year Events

For the 10-year events, the following systems were not fully adequate to convey flow under pipe full conditions:

- 7<sup>th</sup> Ave upstream of Roeser Rd.
- Central Ave downstream of Broadway Rd.
- 7<sup>th</sup> St has one pipe exceeding pipe full capacity but the magnitude is considered minor and the overall system could be considered adequate
- 16<sup>th</sup> St from Roeser Rd to Southern Ave.
- The pipes identified as the 32<sup>nd</sup> St main lines are adequate; however the large pipe diameter East Broadway Lateral (EBL) is not adequate. This lateral has 40<sup>th</sup> St as a contributing lateral.
- 40<sup>th</sup> St–East Southern Lateral and 40<sup>th</sup> St Main line from Baseline to Southern.

3.3.3.3 100-Year Events

For the 100-year events, the following systems were not fully adequate to convey flow under pipe full conditions:

- 19<sup>th</sup> Ave between Broadway Road and Atlanta Ave.
- 7<sup>th</sup> Ave
- Central downstream of Broadway Rd.
- 7<sup>th</sup> St between Roeser Rd. and Southern Ave
- 16<sup>th</sup> St
- 24<sup>th</sup> St between Vineyard Rd. and Southern Ave.
- 32<sup>nd</sup> St.
- 40<sup>th</sup> St.
- 48<sup>th</sup> St. exceeds pipe full capacity from Pecan Rd to Vineyard Rd. The magnitude is relatively small and the pipe could be considered to be adequate.

**Table 3-1: Storm Drain Capacity Assessment**

Main Line	2-Year	10-Year	100-Year
19 <sup>th</sup> Avenue	Adequate	Adequate	Not adequate Broadway - Atlanta
7 <sup>th</sup> Avenue	Adequate	Not adequate Upstream of Roeser	Not adequate
Central Avenue	Adequate	Not adequate Salt River – Broadway	Not adequate Limits placed on upstream pipes & laterals due to lack of capacity
7 <sup>th</sup> Street	Adequate	Generally adequate. Minor under-capacity in one pipe	Not adequate. Roeser - Southern
16 <sup>th</sup> Street	Adequate	Not adequate Southern – Roeser	Not adequate
N 16 <sup>th</sup> Street	Adequate	Adequate	Adequate
24 <sup>th</sup> Street	Adequate	Adequate	Not adequate Vineyard - Southern
32 <sup>nd</sup> Street	Adequate (no negative system flows and no limits exceeded)	Main Line adequate but large diameter East Broadway Lateral not adequate	Not adequate
40 <sup>th</sup> Street	Adequate (no negative system flows and no limits exceeded)	Not adequate	Not adequate
48 <sup>th</sup> Street	Adequate	Adequate	Generally adequate. Minor under capacity in one pipe.
Basis of assessment based upon future land use, 6-hour conditions.			

## **4. IDENTIFIED FLOOD HAZARD AREAS**

### **4.1 GENERAL**

The identification of flood hazards in the study area is the result of multiple sources. Existing FEMA floodplains identify existing regulatory flood hazards. Drainage complaints lodged with the municipalities or received directly from residents from public meetings help identify more detailed and specific problem areas. In addition, FLO-2D simulations substantiate the existence of existing known problem areas and identify potential flooding sources. The model results also help identify other potential hazard areas that may not have been identified in drainage complaints.

### **4.2 FEMA FLOODPLAINS**

Outside of the floodplain limits of the Salt River (Flood Hazard Zone AE), existing study area floodplains are approximately determined (Zone A) and located immediately upstream of the Western and Highline Canals. The remainder of the study area is located in shaded and unshaded Zone X. Flood Insurance Rate Maps (FIRMs) for the study area are provided in Appendix A.

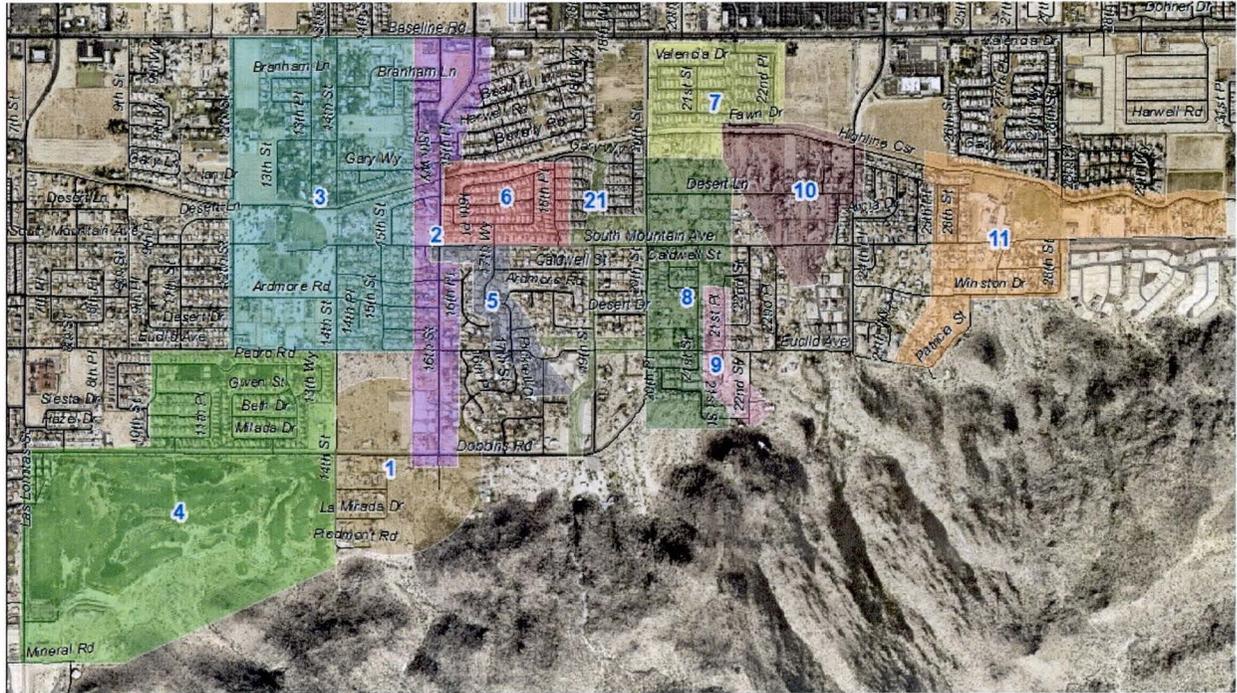
### **4.3 HISTORIC FLOODING AND DRAINAGE COMPLAINTS**

As part of the data collection effort, the COP provided an initial list of specific flooding complaints dating back to the early 1990's. These sites along with other flooding locations identified and obtained from public meetings were investigated to assess the nature and potential cause of the flooding or complaint and categorized as either local issues or regional issues. In addition, storm events during the course of the study provided additional information on problem flooding areas. For one storm even on July 31, 2010, the COP provided an emergency storm report identifying areas of reported storm flooding and the COP's response. The areas identified correlated well with the previously documented drainage complaint areas.

The results of the drainage complaint investigations are documented separately in the Hohokam ADMS Data Collection Report. An exhibit showing the location and nature of flooding/drainage complaints is provided in Appendix B. Based upon these complaints and the results of the FLO-2D analyses several general flooding problem areas were identified and presented to the participants in the Phase I Brainstorming Session.

## 4.4 PROBLEM AREAS DISCERNED FROM FLO-2D ANALYSES

Using FLO-2D results, potential problem areas were identified and numbered. These locations are shown in Figure 4-1, Figure 4-6 and Figure 4-9. Many of the identified problem areas are consistent with documented drainage complaints and areas with known drainage and/or flooding issues.



**Figure 4-1: Identified Problem Areas in the Southwest Area**

### 4.4.1 Area #1: Montana Vista Subdivision (16<sup>th</sup> St. & Dobbins Rd.)

This Montana Vista development located at the corner of 16<sup>th</sup> St and Dobbins is subject to mountain runoff from two separate drainage areas (see Figure 4-1). At the southeast corner of the development, a 10'x4' reinforced concrete box culvert (RCBC) with a grated inlet is provided to convey flow north through the development via the culvert and a rectangular channel downstream that extends as far as the development at which point unconfined flow continues northerly along 16<sup>th</sup> St.

The principal source of water is from the relatively small, eastern most drainage area that discharges to the intersection of 16<sup>th</sup> St and Dobbins. The western drainage area is much larger and drains north along 15<sup>th</sup> St to Dobbins and then either east to the 16<sup>th</sup> St intersection or into the Montana Vista development through the front entrance gates. Some flow does continue northwest overland towards 14<sup>th</sup> St and Circle K Park. The combination of these generally unconfined flows and the apparent inadequacy of the culvert contribute to the flooding within the development. A residential property located on Dobbins Rd., upstream of the culvert and another located on 16<sup>th</sup> St, downstream of

the culvert/channel and residential properties adjacent to 15<sup>th</sup> St, south of Dobbins may also be subject to potential flooding.

#### 4.4.2 Area #2: 16<sup>th</sup> St. from Dobbins Rd. to Baseline Rd.

Flooding along 16<sup>th</sup> St has been documented and specifically a property just north of the Highline Canal and 16<sup>th</sup> St has been flooded to some extent twice in the past 3 years. Much of the drainage within 16<sup>th</sup> St. initially originates from the two mountainous drainage areas that impact the Montana Vista Subdivision at 16<sup>th</sup> St and Dobbins Rd. However, 16<sup>th</sup> St also receives flow from side streets (e.g. from South Mountain Ave to the east) and distributes flow to sides streets (e.g. Euclid Ave to the west) as it continues north towards Baseline Road (see Figure 4-1). The 16<sup>th</sup> St corridor is entwined with flooding and drainage issues at Circle K Park, Montana Vista, Highline Canal, Desert Lane, Gary Way, 15<sup>th</sup> Way, Branham Lane and possibly even 14<sup>th</sup> St and 15<sup>th</sup> St. The lack of a containing conveyance along 16<sup>th</sup> St. results in flow being distributed through the streets aggravating drainage conditions in these areas.



Figure 4-2: Flooding along 16th St during recent event.

#### 4.4.3 Area #3: Circle K Park (14<sup>th</sup> St. & South Mountain Ave.)

Circle K Park itself appears to experience shallow flooding indirectly from mountain runoff overland, through streets (Euclid Ave, S. Mountain Ave. and 14<sup>th</sup> St) and from flow passing through the Dobbins Creek Retention Basins (see Figure 4-1). However, the concentration of flow along the east side of the park and the flow overtopping of Highline Canal at 13<sup>th</sup> and 14<sup>th</sup> St. is the area of most concern. Recent improvements have been constructed along the east boundary of the park that include a riprap lined inlet for street drainage, a wide shallow ponding area, concrete inlets to the canal and bridges across the canal and the inlets. Even prior to the constructed improvements, this location has documented historic flooding issues related to flow overtopping the canal. However, shortly after the construction of improvements, a block wall along the canal was undermined flooding the property(s) downstream of the canal. Though undermined, the wall itself did not fail during the event and does fail in any of the project

FLO-2D analyses. Flow overtopping the canal contributes to flooding issues along 13<sup>th</sup> Pl. and 14<sup>th</sup> St.

#### **4.4.4 Area #4: Thunderbird Country Club G.C. (7<sup>th</sup> St. & Dobbins Rd.)**

The Thunderbird Country Club Golf Course (TCCGC) is located south of Dobbins Rd between 7 St. and 14<sup>th</sup> St. (see Figure 4-1). Being located at the base of the mountain, the golf course receives a significant amount of runoff from the mountains. The golf course stores and attenuates some flow. Two separate 2-8'x4' culverts crossing Dobbins Rd. (called Dobbins Creek West 1 (DCW1) and East 1 (DCE1)) and discharge flow from the golf course to channels through the Dobbins Creek subdivision located immediately downstream and into two retention basins within Dobbins Creek. The basins discharge flow directly to Euclid Ave and continue north through residential properties and towards the Circle K Park.

The golf course area is identified as a problem area because it has a significant impact on area hydrology. Through the data collection effort, it became known that the parcel has already been rezoned and illustrative development plans have been submitted for the conversion of the golf course to a mixed use residential and resort development. A potential decrease in the retention/flow attenuation or increase in runoff volume provided by the golf course and even a change in the distribution flow between the two culverts crossing Dobbins Road could have significant impacts on the Dobbins Creek channels and retention basin and flows downstream of Dobbins Creek including residential properties and Circle K Park. Despite the culverts being the same size, FLO-2D results show the east culvert conveying a significantly more flow than the west culvert.

#### **4.4.5 Area #5: S. Mountain Ave. & 17<sup>th</sup> Way**

FLO-2D results indicate a significant amount of drainage and flooding from the intersection of 16<sup>th</sup> St and South Mountain Ave to Euclid and 17<sup>th</sup> Way (see Figure 4-1). The source of the floodwater is flow crossing the culvert on Dobbins Road at the Boy Scout Camp and diverted west at a downstream culvert in Mountainside Estates. Flow diverted west, overtops the road at 18<sup>th</sup> St and the natural drainage path takes flow down west down Euclid Ave., north along 17<sup>th</sup> Way and then west down South Mountain Ave to 16<sup>th</sup> St. Significant flow in the streets presents the potential for flooding of adjacent properties and contributes to flooding issues in Area 2 along 16<sup>th</sup> St.

#### **4.4.6 Area#6: Vista Portica (17<sup>th</sup> Way & S. Mountain Ave.)**

Vista Portica is a development located just upstream of the Highline Canal and east of 16<sup>th</sup> St. The developments sole retention basin is located at the northwest corner of the development and the southeast corner of 16<sup>th</sup> St and the Highline Canal (see Figure 4-1). A curb opening catch basin along 16<sup>th</sup> St, just prior to the Highline Canal, captures flow and discharges it into the basins. Overtopping of the basin and the blockage and accumulation of sediment in the catch basin along 16<sup>th</sup> St are persistent drainage

issues. Overtopping of the basin can be attributed to significant drainage to the retention basin not only from the development but also from offsite flows along 16<sup>th</sup> St; Highland Ranch (from the east via Francisco Dr), and South Mountain Ave (via 17<sup>th</sup> Way). Generally, documented complaints focus on the retention basin; however, it is possible that some residential properties might be subject to potential flooding due to flow from South Mountain Ave and overtopping of the basin.

#### 4.4.7 Area #7: The Pines at South Mountain (21<sup>st</sup> Way & Baseline Rd)

The Pines at South Mountains is located at 21<sup>st</sup> Way and Baseline Road, immediately downstream of the Highline Canal (see Figure 4-1). This location has some documented drainage issues related to flow overtopping the canal and passing through the site. Along the boundary with the canal, the development has a detention area that drains offsite flow from the Highline Canal to two large grated inlets. The inlets convey flow north to a retention basin along Baseline Road via 2-48" pipes. Flow not captured by the inlets is conveyed overland north along 21<sup>st</sup> St. and discharged into the retention basin. Documented complaints and FLO-2D results indicate that there is significant flow overtopping the canal at this location that floods 21<sup>st</sup> Ave, and potentially cross streets. The source of the offsite water can be attributed to flow in 20<sup>th</sup> St, runoff from the mountains that passes through the Siesta Foothills development and north through developed and undeveloped properties, and runoff from a smaller mountain drainage area that drains north along a drainage tract between 21<sup>st</sup> St and 21<sup>st</sup> PI and then overland north to the Highline Canal.



*Figure 4-3: Sediment in road & the Pines detention basin after recent event.*

#### 4.4.8 Area #8: Siesta Foothills Area (20<sup>th</sup> St. & Euclid Ave.)

Runoff from the mountains and possible some flow diverted northwest along Dobbins Rd from the Boy Scout Camp culvert crossing Dobbins Rd contribute to flooding issues in the vicinity of 20<sup>th</sup> St and Dobbins Rd. (see Figure 4-1). On Euclid Ave, east of 20<sup>th</sup> St, two residential properties are documented as having recurring drainage and flooding issues. The properties are located in a historic flow path but recent development upstream (Siesta Foothills) has interrupted drainage patterns and flow is now discharged to Euclid Ave. at concentrated locations east of 20<sup>th</sup> St.

The source of floodwater can be attributed to surface flows from 20<sup>th</sup> St; a bubble up outlet on Euclid Ave. which is the outlet for a culvert on the east side of 20<sup>th</sup> St.; and a 48" outlet that discharges water captured in two separate mountainside drop inlets south of Siesta Foothills (see Figure 4-4). This floodwater passes north overland through the properties along small drainages and continues north overland to the Highline Canal.

Included in potential flooding areas are properties downstream impacted by these flows and in the Siesta Foothills development where flow in excess of the capacity of the mountainside drop inlets drain into the streets and to an existing retention basin at the southeast corner of 20<sup>th</sup> Street and Euclid Avenue.



**Figure 4-4: Bubble up and 48" outlet discharging to Euclid Ave.**

#### **4.4.9 Area #9: 21<sup>st</sup> Pl. & Euclid Ave.**

Runoff from the mountains drains northwest to 21<sup>st</sup> St and then north across Euclid to a drainage tract between 21<sup>st</sup> Pl and 21<sup>st</sup> St. Along the west side of 21<sup>st</sup> Pl, several properties have documented drainage and flooding issues which is supported with FLO-2D results (see Figure 4-1 and Figure 4-5).



**Figure 4-5: Residential flooding along 21st Pl & Euclid Ave. during recent event.**

#### 4.4.10 Area #10: 22<sup>nd</sup> St – 24<sup>th</sup> St, North of S. Mountain Ave.

West of 24<sup>th</sup> St, mountain runoff is conveyed north across Euclid Ave. through a dip crossing (see Figure 4-1). The flow is fairly well contained until the crossing of South Mountain Ave. where flow splits to the north east along a shallow wash and northwest overland. Downstream of the flow split, flows impact several large lot residential properties.



Figure 4-6: Identified Problems in the Southeast Area

#### 4.4.11 Area #11: 25<sup>th</sup> Pl – 32<sup>nd</sup> St, Ponding along the Highline Canal

Between 25<sup>th</sup> and roughly 32<sup>nd</sup> St., there are sizeable areas and notable depths of ponding along the Highline Canal (see Figure 4-1 & Figure 4-6). Developments north of the canal have structurally sound perimeter walls that are barriers to flow and no significant accommodations are made to divert flow east or west. Impacted are properties south of the canal including an elementary school, a public park, a community center and some residential properties east of 28<sup>th</sup> St. Residential properties further south are impacted by the shallow overland flow contributing to ponding along the canal.

#### 4.4.12 Area #12: Cortland Point (36<sup>th</sup> St & Highline Canal)

The Cortland Point subdivision is located along the north side of the Highline Canal between 34<sup>th</sup> Pl. and 36<sup>th</sup> St. (see Figure 4-6). This location has documented residential flooding. Along the Highline Canal, the adjacent residential properties are graded two to three feet lower than the Highline Canal. Runoff from the mountains overtops the canal

and either ponds behind block walls or drain directly down the canal embankment into Melody Drive. Water ponding along the block walls seeps through the blocks and in one recorded event, collapsed the wall and flooded the property and Francisco St.



**Figure 4-7: Cortland Point (left) and Shadow Mountain Villas flooding (right).**

#### **4.4.13 Area #13: Shadow Mountain Villas (36<sup>th</sup> St & Baseline Rd)**

Shadow Mountain Villas Condominiums is located at 36<sup>th</sup> St and Baseline (see Figure 4-6). The condo property is downstream of Cortland Point and the Highline Canal. This location has documented flooding in a parking lot located along the Highline Canal and may receive runoff from the mountains through Cortland Point via 34<sup>th</sup> Pl and/or Melody Drive and 36<sup>th</sup> St. However, the grading and drainage design of the condo development itself contributes, if not causes, the majority of the drainage and flood issues. The interior development drainage is handled by interior roads some of which have inverted crowns to increase conveyance capacity. These roads all drain to the north parking lot which also serves as the development's retention basin. Unfortunately, parked cars are susceptible to flooding several feet deep. During small rainfall events, the parking lot likely still floods to some degree. During large events, offsite drainage from flow overtopping the canal may enter the site and aggravate the situation and increase the ponding depth of the parking lot.

#### **4.4.14 Area #14: 42<sup>nd</sup> St. & Baseline Rd.**

A wash runs roughly to 42<sup>nd</sup> St and terminates at a combined culvert/storm drain hydraulic inlet south of Baseline Road (see Figure 4-6). Upstream of the wash, flow is well contained and does not appear to significantly impact adjacent residential properties during the 100-year events. However, the peak discharges for the 100 year events greatly exceed the capacity of the hydraulic inlet which outfalls to a 30" storm drain pipe and a 30" culvert. For the 100-yr, 6-hr (existing) the peak discharge is estimated at approximately 590 cfs. The consequences of the flow overwhelming the inlet is that Baseline Rd. would be flooded (2-3 ft deep) and floodwater could impact properties along the northwest corner of 40<sup>th</sup> St. and Baseline Rd.



**Figure 4-8: Inlet at 40th St. & Baseline (left) & wash near Beautiful Lane (right).**

#### **4.4.15 Area #15: 46<sup>th</sup> St. & Beautiful Lane**

In the proximity of 46<sup>th</sup> St and Beautiful Lane, two separate washes/drainages contribute to flooding of industrial buildings south of Beautiful Lane and potentially industrial properties north of Beautiful Lane along Baseline Road (see Figure 4-6). The western-most drainage is primarily responsible for documented flooding of an industrial building. The building site was placed in the historic flow path and mountain runoff floods the parking lot and ponds behind an elevated Highline Canal which parallels Beautiful Lane. The eastern most wash has a larger drainage area and is more incised. Near Beautiful Lane, flow from the wash passes through several drop inlet and detention basins ultimately being drained by a storm drain that crosses the Highline Canal and discharges to a retention basin along Baseline Road. The magnitude of the runoff, the limited capacity of the storm drain pipes/drop inlet and blockage from debris likely contribute to the accumulation of water and potential flooding behind Highline Canal.

#### **4.4.16 Area #16: Ponding along the Western Canal**

Along the length of the Western Canal there are areas of ponding behind the canal due to the elevation of the canal embankments (see Figure 4-6 and Figure 4-9). Most likely, much of this is due to local drainage runoff but during flood events flow from Baseline Rd. and south of Baseline may contribute to flooding issues.

In addition, there are some areas downstream of the canal such as between 14<sup>th</sup> St and 16<sup>th</sup> St., where the situation is similar to Cortland Point, which could be adversely impacted from flow overtopping the canal. Water is impounded between residential block wall and the canal embankment and no accommodations are made for drainage. Failure of these walls could likely result in the flooding of downstream properties.



**Figure 4-9: Identified Problems in the Central Area**

#### 4.4.17 Area #17: Vineyard Rd. / 18<sup>th</sup> St. / 19<sup>th</sup> Pl. / 20<sup>th</sup> St.

Drainage issues along Vineyard, west of 16<sup>th</sup> St, 18<sup>th</sup> St and 20<sup>th</sup> St have been observed in the field during field reconnaissance (see Figure 4-9). The observed issue was significant street flooding and ponding. No flooding of adjacent residential properties was observed and no formal complaints have been received by the COP. FLO-2D results do indicate the shallow flooding of several residential properties at the terminus of 19<sup>th</sup> Pl. At this location, 19<sup>th</sup> Pl. terminates in a cul-de-sac and a drainage outlet is provided to the east to 20<sup>th</sup> St. The results also reflect ponding along Vineyard Road and significant flows along 18<sup>th</sup> St (110 cfs at 18<sup>th</sup> St. south of Nancy Lane) and 20<sup>th</sup> St (200 cfs at Cross Section 70 – 20<sup>th</sup> St. & Alta Vista) south of Southern Ave. The flows contribute to flooding of Southern Ave and likely residential properties north of Southern Ave where in the general vicinity, several complaints have been documented.

The source of the floodwater is likely primarily interior street runoff concentrating in these collector streets. However, for the 100-year events, flow overtopping the Western Canal also contributes to flows and aggravates drainage and flooding conditions. Overtopping flow is captured by hardened channels between the residential block walls and the canal embankment and discharge into the streets at cul-de-sacs terminating at the canal at 17<sup>th</sup> St., 18<sup>th</sup> St., 19<sup>th</sup> St. and 19<sup>th</sup> Pl.



**Figure 4-10: Partial closure of Vineyard Rd west of 18<sup>th</sup> St during recent storm.**

#### **4.4.18 Area #18: Contempo Tempe Mobile Home Park**

This mobile home park is located east of 48<sup>th</sup> St and north of the Western Canal. FLO-2D results indicates a wide area of shallow flooding (1-1.5 ft of depth) surrounding the mobile home park's northwest retention basins just south of Southern Ave (see Figure 4-6). The source of the floodwater appears to originate with the mobile home park itself and likely due to a combination of an undersized basin and the fact that the basin has a perimeter block wall and inflow is limited to small drainage inlets.

#### **4.4.19 Area #19: South Mountain Community College (SMCC)**

Flow overtopping the canal just west of 24<sup>th</sup> St contributes to flooding primarily of the south parking lot of SMCC and retention area (see Figure 4-9).

#### **4.4.20 Area #20: 2<sup>nd</sup> St. & Weir Ave**

This appears to be a low lying area subject to periodic flooding. FLO-2D results indicate depth of 1 ft to 2.5 ft over a block of single family homes. During a site visit a resident confirmed the area had drainage and flooding issues in the past (see Figure 4-9).

#### **4.4.21 Area #21: 19<sup>th</sup> St – Dobbins Rd to Highline Canal**

Runoff from the mountains passes through the Boy Scout Camp and crosses Dobbins Rd through a large culvert (see Figure 4-1). Much of this flow is conveyed north to a 2-6'x4' RCBC through the Mountain Estates development. At the culvert some flow is diverted either northwest towards 17<sup>th</sup> Way or passes through the culvert and is discharged to a series of weirs along the west side of 19<sup>th</sup> St. All flow eventually ends up at the corner of 19<sup>th</sup> St and Euclid Ave. From north of Euclid Ave., 19<sup>th</sup> St. is an inverted crown road to convey flow. This flow contributes to flooding along Euclid Ave, 19<sup>th</sup> St and along South Mountain Ave. Residential flooding issues have been documented at 19<sup>th</sup> St. and South Mountain Ave.

## 5. CULTURAL RESOURCES

### 5.1.1 Class I Survey Summary

The preparation of the Hohokam ADMS includes a Class I survey of the cultural resources within the project area. The survey area includes a one-mile buffer around the study boundary. Below is a brief summary of the results of the Class I survey. The complete survey, *A Class I Cultural Resources Inventory Survey of 16, 000 Acres for the Hohokam Area Drainage Master Plan, Phoenix, Maricopa County, Arizona. October 2010 (LSD)* is provided separately.

The survey included literature and records review of archaeological site files, inventory reports and data at; the Arizona State Historic Preservation Office (SHPO); the Arizona State Museum (ASM) using AZSITE; site files at the Pueblo Grande Museum (PGM); the City of Phoenix and City of Tempe Historic Preservation Departments to determine boundaries of City-listed historic districts and the National Register Information System to gather information about National Register of Historic Places (NRHP)-listed properties in the study area. Historic General Land Office (GLO) maps were also reviewed and road features and canals are depicted on those maps.

The records search indicated that more than 200 surveys have been completed within the study area. Collectively, these surveys encompass 616 acres, or approximately 4 percent of the study area. The archaeological surveys identified 112 prehistoric and historic cultural resources. The prehistoric sites span the length of the Hohokam occupation in the Phoenix Basin; site types range from artifact scatters and petroglyph sites, to agricultural sites associated with canals and field houses, to large village sites with cemeteries. The historic period sites include canals (Roosevelt Canal, Western Canal, San Francisco, Hayden, and North Branch Highline), multiple spurs of the Southern Pacific Railroad (Welton-Phoenix-Eloy and Tempe-West Chandler), and cemeteries, as well as artifact/trash scatters with and without features, and buildings.

The majority of previously recorded cultural resources within the Hohokam study area have not been evaluated for their NRHP eligibility. Three historic properties are listed in the NRHP including the Niels Peterson House, the Phoenix Carnegie Library and Park, and the Ralph H. Stoughton Estate. Eight other cultural resources and five sites have been previously determined eligible for inclusion in the NRHP. For the remainder of the identified cultural resources, 4 were recommended for testing to determine NRHP-eligibility, 45 were recommended NRHP-eligible, and 60 are unevaluated or of unknown eligibility.

If existing or newly recorded NRHP-eligible cultural resources could be affected by drainage improvements, these resources shall be treated in a manner consistent with the Secretary of the Interior's Guidelines for the Treatment of Historic Properties, applicable Arizona statutes, and City of Tempe and City of Phoenix regulations.

## 6. CSFHM INVENTORY & ANALYSIS

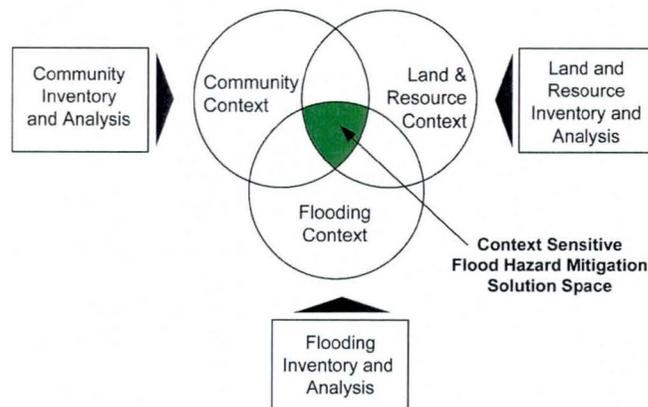
### 6.1 INTRODUCTION

The District's Context Sensitive Flood Hazard Mitigation (CSFHM) Planning and Design Approach was implemented as a part of the Hohokam ADMS. The CSFHM Approach is designed to serve as a framework and tool for the development of flood hazard mitigation plan alternatives that integrate the three basic required functions of being **A**cceptable to local communities, **C**ompatible with landscape resources and **E**ffective in reducing flood losses (**ACE**). A context sensitive solution is one that is capable of performing all three of these basic functions.

The CSFHM model examines the interrelationship between three contexts: Community, Land & Resources and Flooding (see Figure 6-1). The Community Context was defined in this study based upon an inventory and analysis of the direction and vision contained in local community plans including:

- Phoenix General Plan 2002
- City of Tempe General Plan (2030)
- Baseline Area Master Plan (1996)
- Target Area B Redevelopment Plan (1998)
- Rio Salado Beyond the banks Area Plan (2003)
- Rio Montana Area Plan (2000)
- South Central Avenue Corridor Study (1993)

The Land & Resources Context was defined, by the District, through an inventory and assessment of the valued characteristics of landscape resources that included scenery, recreation and open spaces. The Flooding Context was defined through an inventory of flooding types and an assessment of public exposure to flooding.



**Figure 6-1: Context Sensitive Flood Hazard Mitigation Planning & Design Model**

Application of the CSFHM Approach involves use of the six step process outlined below:

1. Project Goals & Objectives Establishment
2. Range of Possible Flood Hazard Mitigation Solutions Identification
3. Context Inventories
4. Context Analyses
  - a. Predictive Analysis
  - b. Comparative Analysis
5. Alternatives Formulation and Analysis
6. Recommended Plan Selection and Refinement

The District identified a range of flood hazard mitigation solutions, including various Structure Types, Structural Methods and Landscape Design Themes that are often considered for the development of flood hazard mitigation plan alternatives (see Table 6-1). A fully context sensitive solution requires the identification and use of a combination of Structure Types, Structural Methods and Landscape Design Themes that are all capable of performing the three functions of being Acceptable, Compatible and Effective (ACE). Detailed descriptions of the Structure Types, Structural Methods and Landscape Design Themes can be found in the District's Flood Protection Structure Types, Methods and Landscape Design Themes Handbooks available on the District's website.

**Table 6-1: Aspects of Flood Hazard Mitigation Solutions**

Structure Types	Structural Methods	Landscape Themes
• Natural Structure	• Natural	• Natural Sonoran Desert Upland
• Underground Pipe	• Soft Structural	• Natural Sonoran Desert Upland Riparian
• Channel Levee	• Semi-Soft Structural	• Natural Lower Sonoran Desert
• Conveyance Channel	• Enhanced Hard Structural	• Natural Lower Sonoran Desert Riparian
• Storage Basin	• Semi-Hard Structural	• Natural Sonoran Desert Hydro Riparian
• Dam	• Hard Structural	• Semi-Natural Sonoran Desert • Enhanced Desert • Desert Park • Desert Oasis • Urban Plaza

Information contained in the inventories of the three contexts was then used in a Predictive Analysis to assign acceptability, compatibility and effectiveness ratings to each Structure Type, Structural Method and Landscape Design theme. These ratings and the inventory maps were then utilized in GIS to produce maps that revealed the range of acceptable, compatible and effective Structure Types, Structural Methods and

Landscape Design Themes for discrete geographic areas within the study area. Finally, a Comparative Analysis was undertaken to combine and synthesize the results of the Predictive Analysis maps for each context. This step revealed the range of Structure Types, Structural Methods and Landscape Design Themes that are capable of simultaneously performing all three functions (ACE) of a context sensitive solution for discrete geographic areas within the study area.

The range of context sensitive solutions that were revealed from implementation of the Predictive and Comparative Analysis steps of the CSFHM Approach are intended to serve as the building blocks for development of context sensitive plan alternatives during the alternatives formulation stages of the study. The CSFHM analysis of the three contexts is also serves as a baseline for evaluating the context sensitivity of the Alternatives during the alternatives evaluation stage of the study.

The following sections briefly describe the inventory and analysis of the Community, Land & Resource and Flooding Contexts for the Hohokam ADMS. A more detailed description of the CSFHM process is provided in the Hohokam ADMS Data Collection Report.

## 6.2 COMMUNITY CONTEXT

This section contains an overview of the inventory and analysis of the Community Context. The inventory of the Community context included collection and review of community plans containing direction relating to the acceptability of various flood hazard mitigation solutions that might be considered in the development of plan alternatives for the Hohokam ADMP. The primary sources of information that were utilized in assessment of the Community Context included: 1) the Phoenix General Plan, 2) The General Plan Recreation and Open Space Elements, and 3) Specific Area Plans, including the Baseline Area Master Plan, Target Area B Redevelopment Plan, Rio Solado Beyond the Banks Plan, Rio Montana Area Plan and the South Central Avenue Corridor Study. A summary of the inventory of these plans along with the Community Context Acceptability Class maps for the Structure Types, Structural Methods and Landscape Design Themes that were produced for each of the above inventoried community plans may be found in the Hohokam ADMS Data Collection Report.

### 6.2.1 Results of Community Context Analysis

To determine the range of acceptable FHM solutions, the planning team participated in a workshop to review the available character and aesthetic elements of the plans and identify the acceptability of the Structural Methods, Structure Types, and Landscape Themes within each specific study area. Using GIS, the District compiled the workshop information into datasets that combined the acceptability of each component for all the study areas and each component.

#### 6.2.1.1 Structure Type Acceptability

All structure types except for a dam were determined to be acceptable throughout most of the study area (see Figure 6-2). In open space and park areas, all other methods would be acceptable with the exception of a channel levee and a dam. All structural methods would be acceptable in areas comprised of heavy commercial and industrial land uses which may be found in the eastern portion of the study area.

#### 6.2.1.2 Structural Methods Acceptability

Throughout the study area, the natural method, soft structural, and semi-soft structural were generally determined to be acceptable (see Figure 6-3). Hard structural methods would be acceptable only in limited areas.

#### 6.2.1.3 Landscape Themes Acceptability

Throughout most of the study area, all landscape themes would be acceptable except Natural Sonoran Desert Hydro Riparian and the Urban Plaza (see Figure 6-4). In the South Mountain area, the acceptable themes are limited to the Natural Sonoran Desert themes while generally most themes except the Natural Sonoran Desert Upland and Natural Sonoran Desert Upland Riparian themes would be acceptable.



## Hohokam Area Drainage Master Study/Plan Community Context Structure Types Acceptability Map

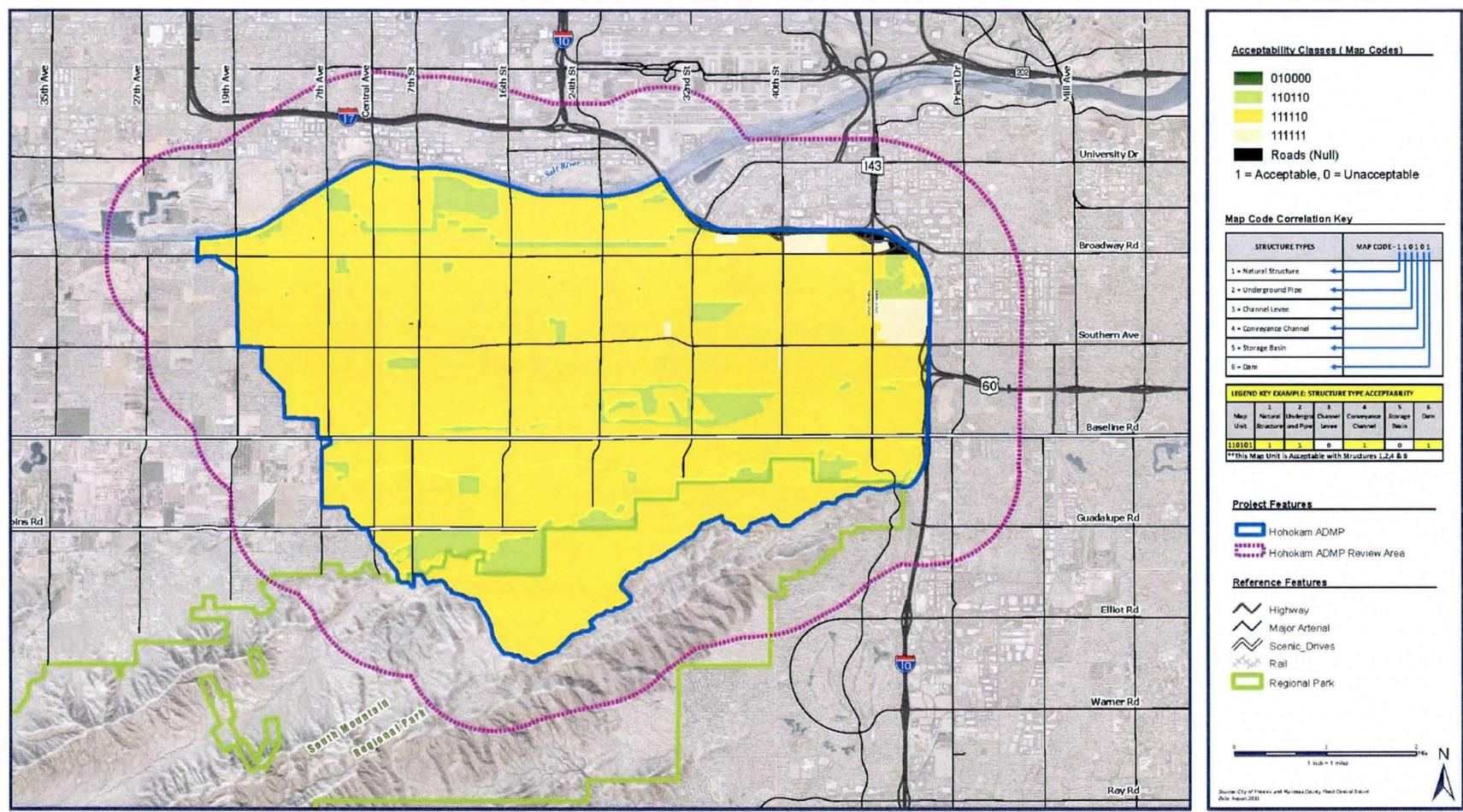


Figure 6-2: Structure Types Acceptability

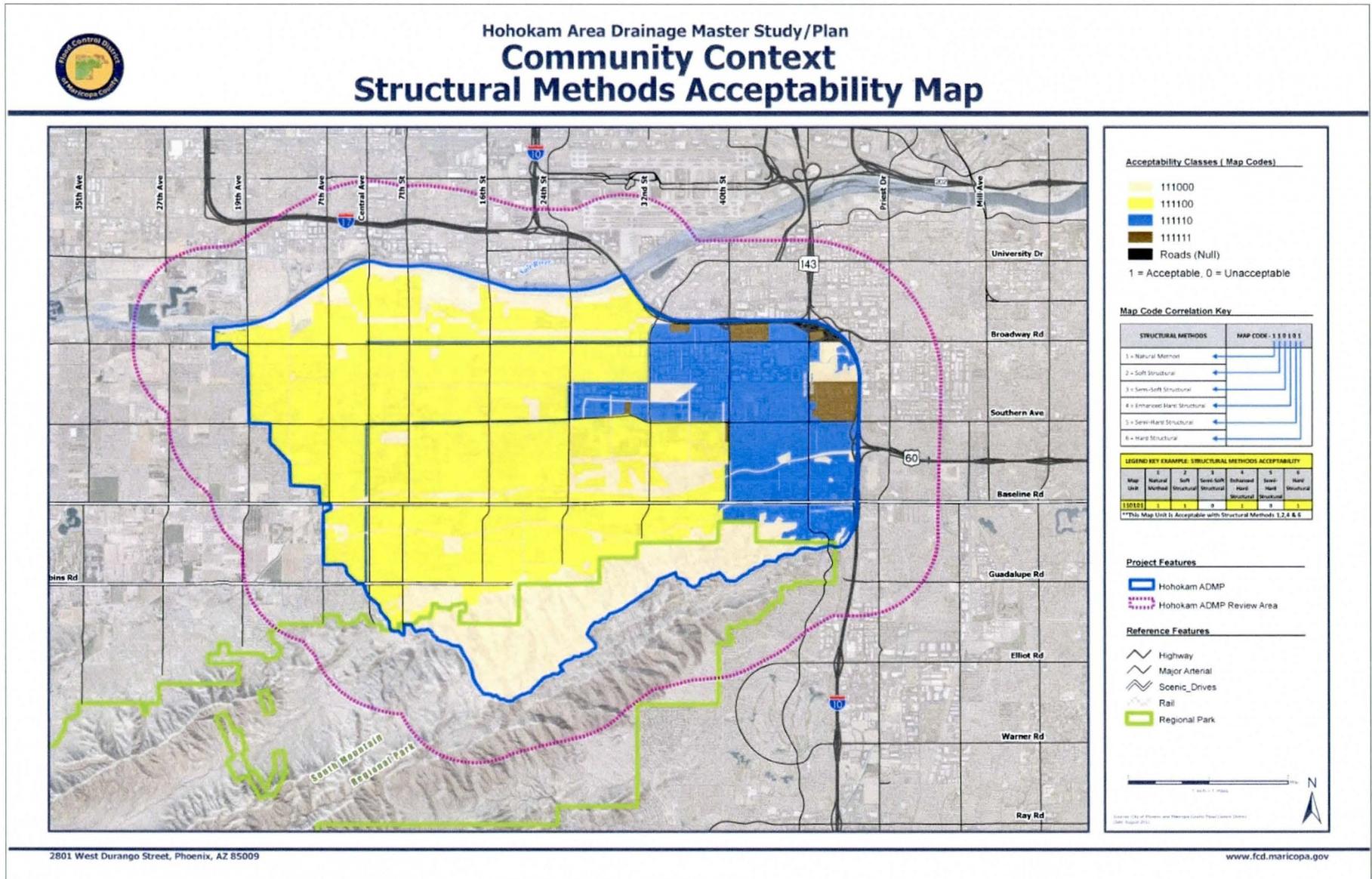
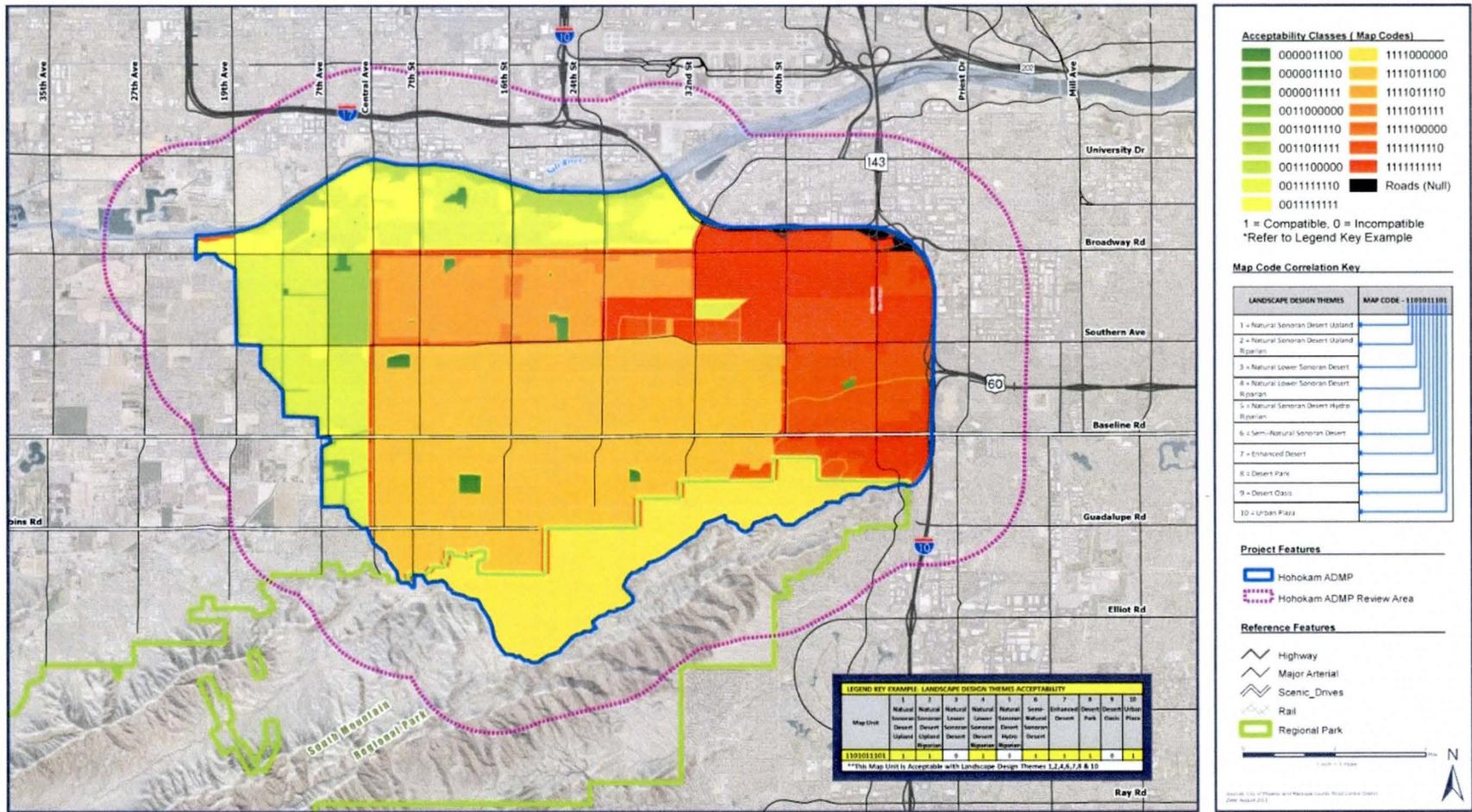


Figure 6-3: Structural Methods Acceptability



## Hohokam Area Drainage Master Study/Plan Community Context Landscape Design Themes Acceptability Map



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Figure 6-4: Landscape Design Theme Acceptability

## 6.3 LAND AND RESOURCE CONTEXT

This section provides an overview of the inventory and analysis of the Land and Resource Context. A project level inventory and analysis (Project LIA) was provided by the District that was derived from its county-wide Landscape Inventory and Analysis (County-wide LIA). This assessment included inventories of scenery, recreation and open space resources. It also included assessments of the compatibility of a range of flood hazard mitigation structure types, structural methods and landscape design themes with these inventoried resources. For more detailed information about the Project LIA for the Hohokam ADMS, please refer to the Hohokam ADMS Data Collection Report.

The following is a summary of the compatibility of possible structural methods, structure types, and landscape design themes with the combined landscape resources (scenery, recreation and open space) of the study area. The compatibility of structure types, structural methods and landscape themes in the land and resource context are shown in the LIA provided in the Hohokam ADMS Data Collection Report.

### 6.3.1.1 Structure Types Compatibility

Natural structures are the only flood hazard mitigation structure types that are compatible within the floodway and flood fringe of the Salt River as well as the slopes of South Mountain (Figure 6-5). All structure types would be compatible over the majority of the study area, and in a few areas, all of the structure types, except for dam structures are compatible with the land and resource context.

### 6.3.1.2 Structural Methods Compatibility

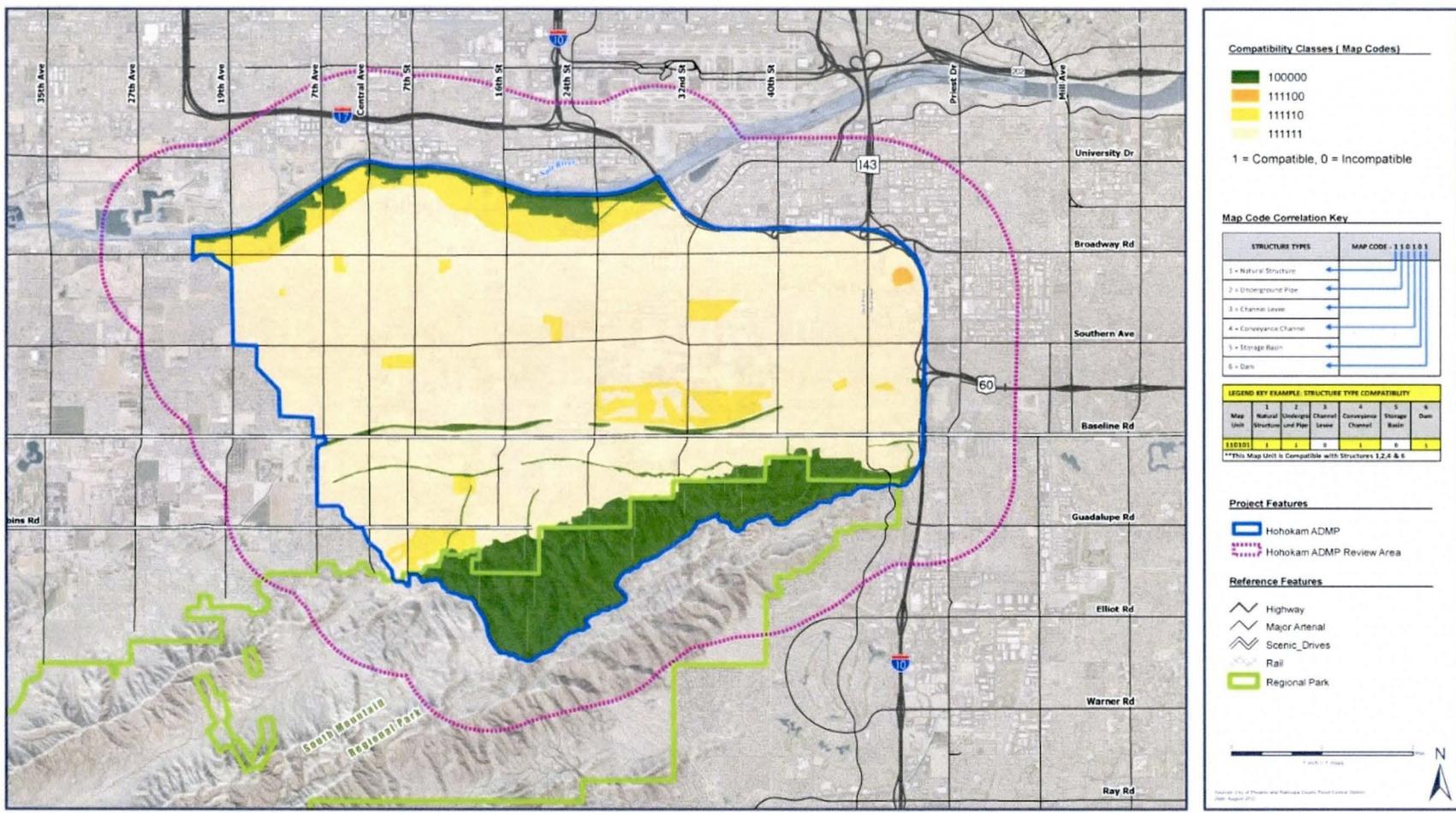
Natural is the only structural method that is compatible within the floodways, flood fringe areas and the slopes of South Mountain (Figure 6-6). The Semi-Soft Structural, Soft Structural and Natural Methods are compatible for a majority of the study area. Hard Structural Method is compatible in areas of heavy commercial or industrial use which are mostly located in the northern and eastern portions of the study area. All of the structural methods are compatible within the industrial valley plain landscape units.

### 6.3.1.3 Landscape Design Theme Compatibility

The Natural Sonoran Desert Uplands and Riparian landscape design themes would be compatible within the South Mountain area (Figure 6-7), while Natural Lower Sonoran Desert Riparian and Hydro-riparian landscape design themes would be compatible in areas along the Salt River. Within the developed areas throughout most of the study area, landscape design themes including the Semi-natural Desert, Enhanced Desert, Desert Park, Oasis and plaza themes would be compatible.



## Hohokam Area Drainage Master Study/Plan Land & Resource Context Structure Types Compatibility Map



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**Figure 6-5: Future Combined Structure Types Compatibility**



## Hohokam Area Drainage Master Study/Plan Land & Resource Context Structural Methods Compatibility Map

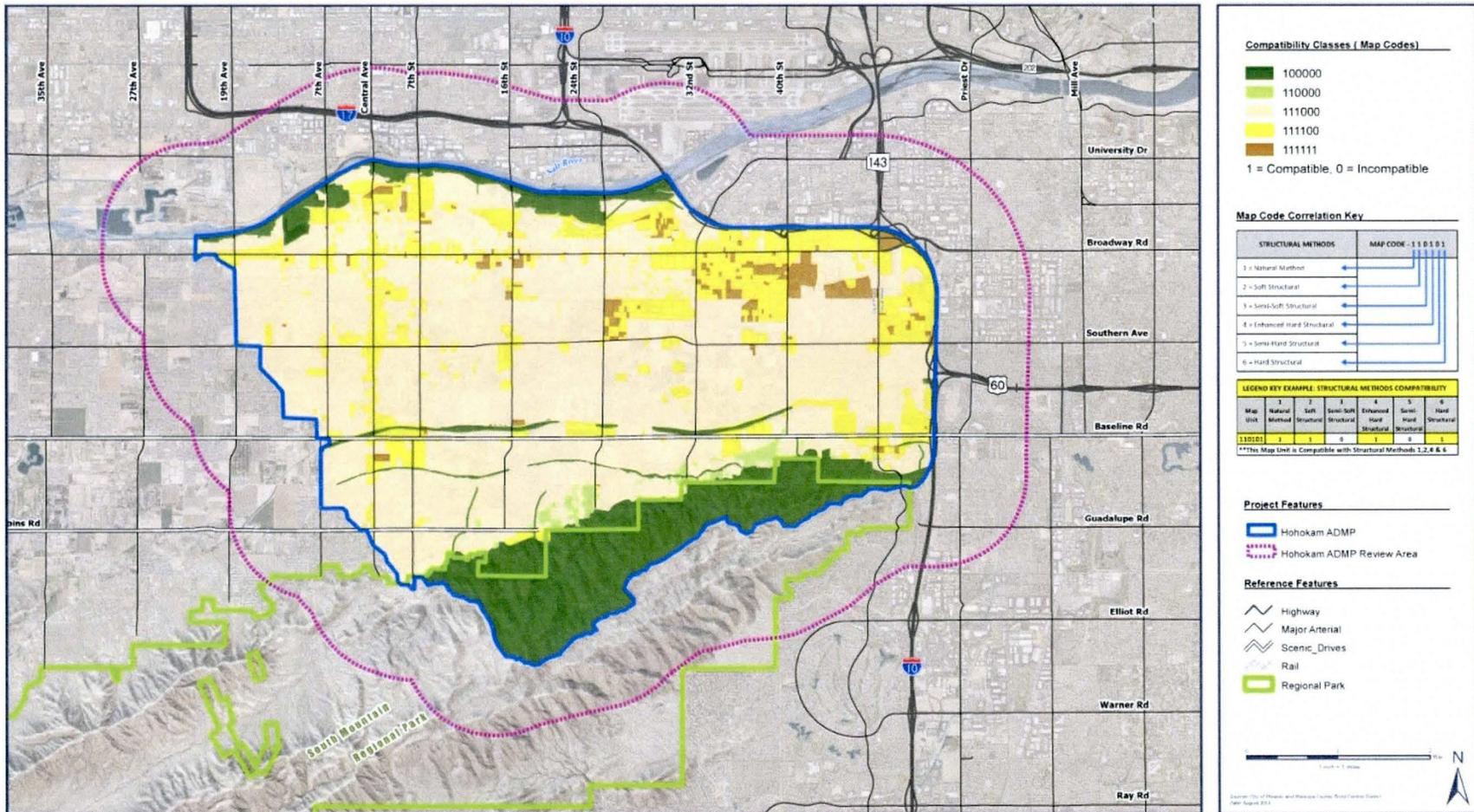
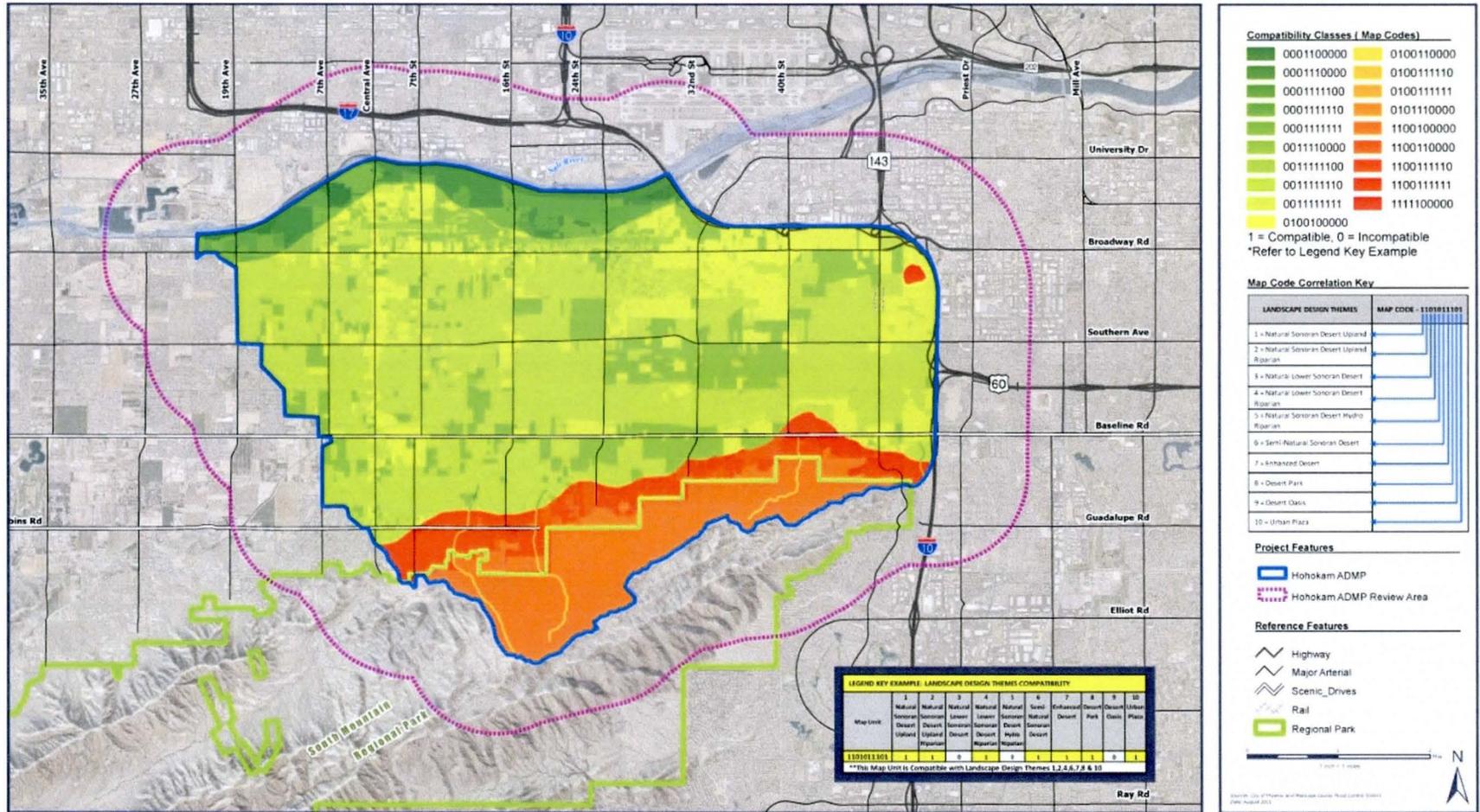


Figure 6-6: Future Combined Structural Methods Compatibility



## Hohokam Area Drainage Master Study/Plan Land & Resource Context Landscape Design Themes Compatability Map



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Figure 6-7: Future Combined Landscape Theme Compatibility

## 6.4 FLOODING CONTEXT

This section contains an overview of the inventory and analysis of the Flooding Context. The inventory of the Flooding Context included an identification and evaluation of the flooding types and the flood hazards that are associated with them. The inventory also included an assessment of flood risks based upon the proximity of humans to the identified flood hazards. The flooding type over most of the study area is overland sheet flow with the notable exception of riverine wash flows that occur within the incised channels located within the mountain slopes of South Mountain.

To identify the effective approaches to mitigate flooding hazards, the study team participated in an evaluation workshop to determine the Structural Methods, Structure Types, and Landscape Themes for those elements that would be effective for each type of flooding hazard.

### 6.4.1.1 Effective Flood Control - Structure Types

Similar to the flood control methods, all structure types except a natural structure would be considered effective over the majority of the area (see Figure 6-8). In the areas of riverine flow, underground pipe, channel levee and conveyance channels would be considered the most effective flood control structure types. In the South Mountain area, all structure types would be effective.

### 6.4.1.2 Effective Flood Control - Structural Methods

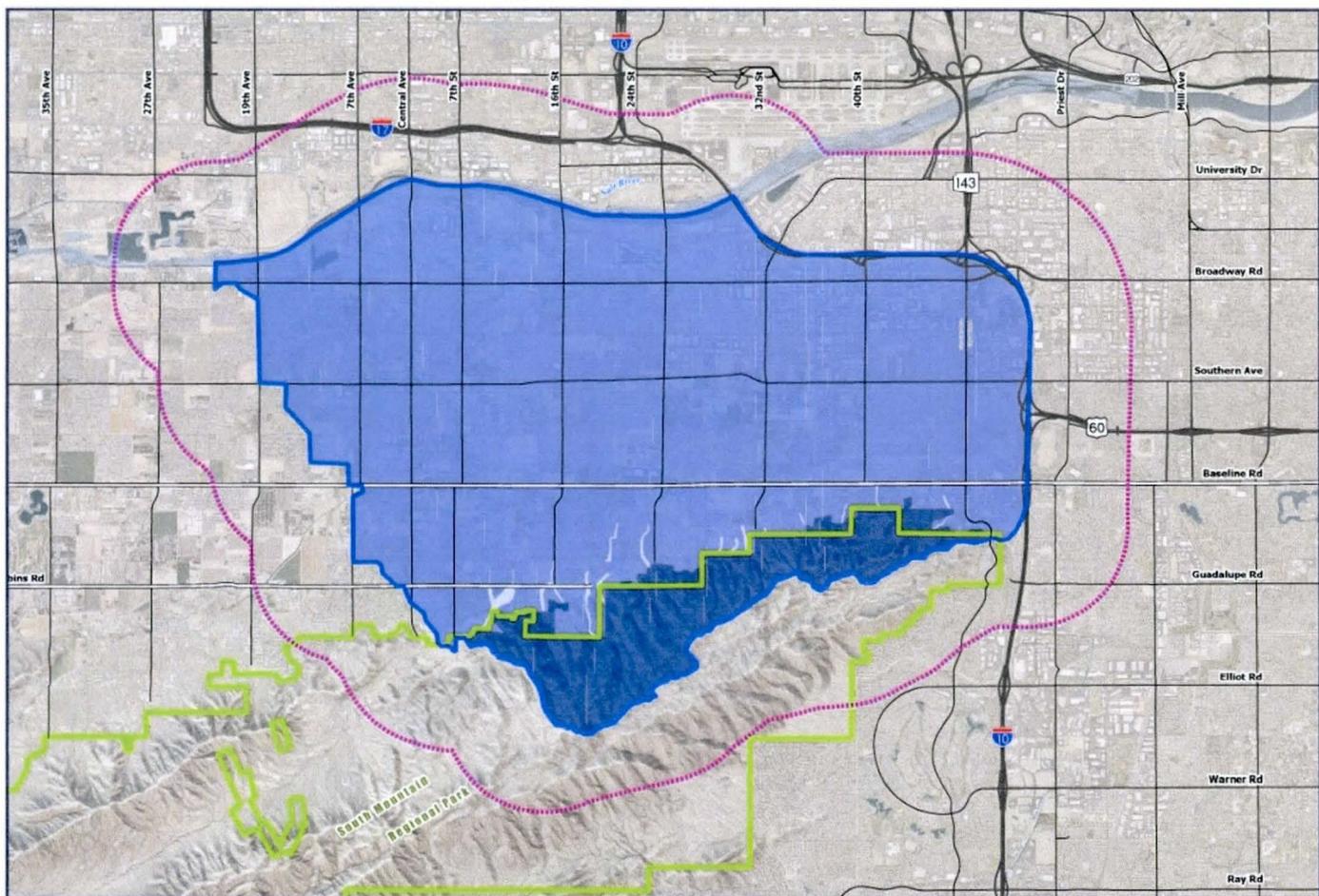
The simplicity of the drainage patterns and flooding types leads to a simple solution for developing effective flood control methods (see Figure 6-9). Except within the South Mountain Park, all flood control methods except a natural method could be effective. Within South Mountain Park all methods would be considered effective.

### 6.4.1.3 Effective Flood Control - Landscape Themes

For landscape themes, only the limited riverine flow areas would have some limitations on themes for the flooding context (see Figure 6-10). In those areas the Natural Sonoran Desert Upland and Upland Riparian Themes as well as the Desert Oasis and Urban Plaza themes would not be considered effective. Throughout most of the study area, all themes except the two natural Sonoran Desert themes would be considered effective for the flooding context. In the South Mountain Area, all landscape themes would be effective.



## Hohokam Area Drainage Master Study/Plan Flooding Context Flooding Types Structure Types Effectiveness



**Effectiveness Classes ( Map Codes)**

- 011100
- 011111
- 111111

1 = Effective, 0 = Ineffective

**Map Code Correlation Key**

STRUCTURE TYPES	MAP CODE	1	1	0	1	1
1 = Natural Structure	←	1	1	0	1	1
2 = Underground Pipe	←	1	1	0	1	1
3 = Channel Levee	←	1	1	0	1	1
4 = Erosion Control	←	1	1	0	1	1
5 = Storage Basin	←	1	1	0	1	1
6 = Dam	←	1	1	0	1	1

**LEGEND KEY EXAMPLE: STRUCTURE TYPE EFFECTIVENESS**

Map Unit	1 Natural Structure	2 Underground Pipe	3 Channel Levee	4 Erosion Control	5 Storage Basin	6 Dam
011100	1	1	0	1	0	1

**\*\*This Map Unit is Effective with Structures 1,2,4 & 6**

**Project Features**

- Hohokam ADMP
- Hohokam ADMP Review Area

**Reference Features**

- Highway
- Major Arterial
- Scenic Drives
- Rail
- Regional Park

Scale: 1" = 1 Mile  
North Arrow

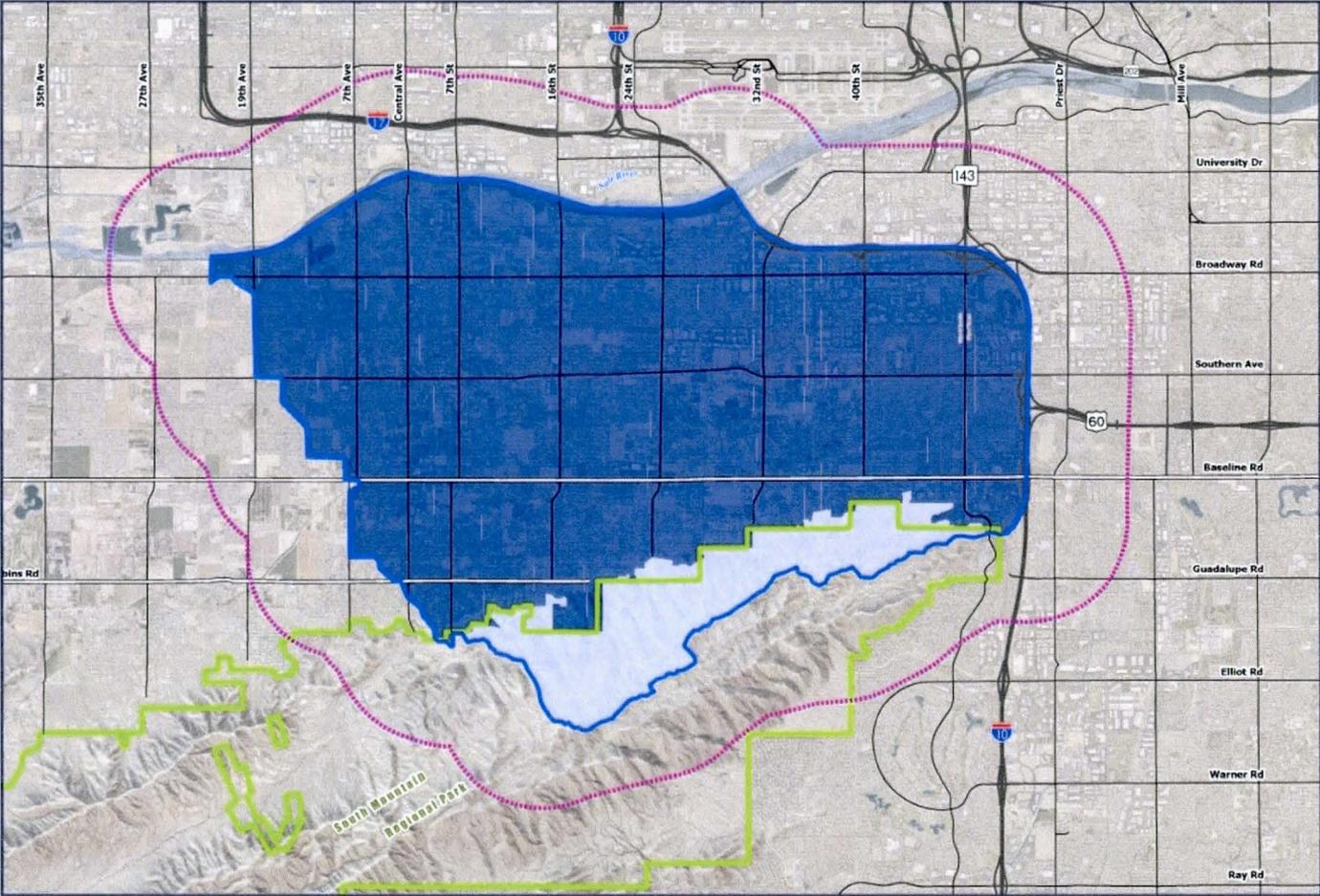
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**Figure 6-8: Flooding Context Structure Types Effectiveness**



## Hohokam Area Drainage Master Study/Plan Flooding Context Flooding Types Structural Methods Effectiveness



**Effectiveness Classes ( Map Codes)**

011111  
111111

1 = Effective, 0 = Ineffective

**Map Code Correlation Key**

STRUCTURAL METHODS	MAP CODE - 1 1 0 1 1 1
1 = Natural Method	1
2 = Soft Structural	2
3 = Semi-Soft Structural	3
4 = Enhanced Hard Structural	4
5 = Semi-Hard Structural	5
6 = Hard Structural	6

**LEGEND KEY EXAMPLE: STRUCTURAL METHODS EFFECTIVENESS**

Map Unit	Natural Method	Soft Structural	Semi-Soft Structural	Enhanced Hard Structural	Semi-Hard Structural	Hard Structural
110101	1	3	0	0	0	1

\*\*\*Map Unit is Effective with Structural Methods 1,2,4 & 6

**Project Features**

- Hohokam ADMP
- Hohokam ADMP Review Area

**Reference Features**

- Highway
- Major Arterial
- Scenic Drives
- Rail
- Regional Park



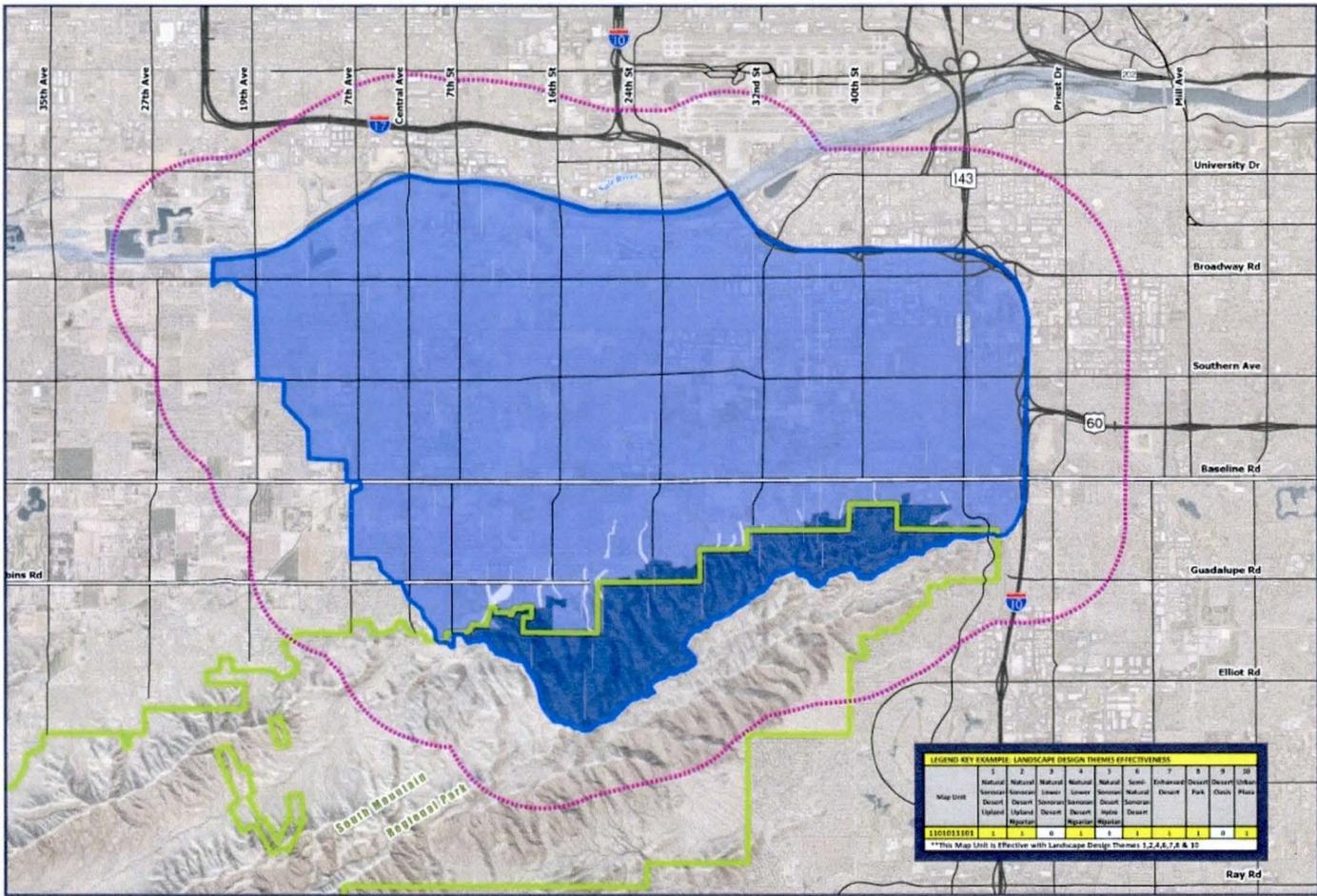
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Figure 6-9: Flooding Context Structural Methods Effectiveness



## Hohokam Area Drainage Master Study/Plan Flooding Context Flooding Types Landscape Design Themes Effectiveness



**Effectiveness Classes ( Map Codes)**

- 0011111100
- 0011111111
- 1111111111

1 = Effective, 0 = Ineffective  
\*Refer to Legend Key Example

**Map Code Correlation Key**

LANDSCAPE DESIGN THEMES	MAP CODE - 1101011101
1 = Natural Sonoran Desert Upland Riparian	1
2 = Natural Sonoran Desert Upland Riparian	2
3 = Natural Lower Sonoran Desert Riparian	3
4 = Natural Lower Sonoran Desert Riparian	4
5 = Natural Sonoran Desert Hydro Riparian	5
6 = Semi-Natural Sonoran Desert	6
7 = Irrigated Desert	7
8 = Desert Park	8
9 = Desert Class	9
10 = Urban Plaza	10

**Project Features**

- Hohokam ADMP
- Hohokam ADMP Review Area

**Reference Features**

- Highway
- Major Arterial
- Scenic Drives
- Rail
- Regional Park

**LEGEND KEY EXAMPLE: LANDSCAPE DESIGN THEMES EFFECTIVENESS**

Map Code	1	2	3	4	5	6	7	8	9	10
1101011101	1	1	0	1	1	1	1	1	0	1

\*\*This Map Code is Effective with Landscape Design Themes 1,2,4,4,7,8 & 10

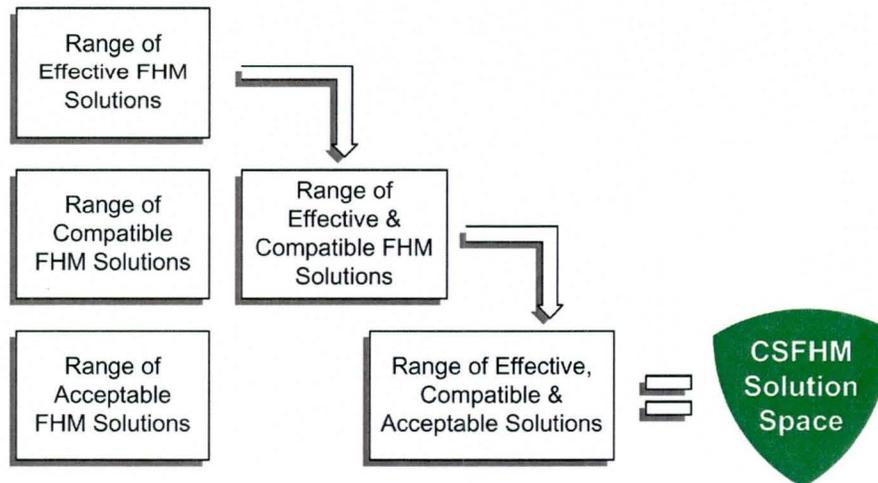
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Figure 6-10: Flooding Context Landscape Design Themes Effectiveness

## 6.5 CSFHM ANALYSIS AND RESULTS

In the final step of the analysis, information from the analysis maps for the three contexts is combined using GIS to identify those FHM solutions that concurrently meet all three of the basic functional requirements of being acceptable, compatible and effective (ACE) for a CSFHM solution. The comparative analysis begins with a comparison of the range of effective solutions with the range of compatible solutions to identify the set of solutions that is both effective and compatible (see Figure 6-11). The effective/compatible set of solutions are then compared with the range of acceptable solutions to identify the set of solutions that meet the ACE criterion.



**Figure 6-11: Comparative Analysis Process**

### 6.5.1.1 Context Sensitive Structure Types

For most of the study area, underground pipe, channel levee, conveyance channel or storage basin could be part of a context sensitive flood control solution (see Figure 6-13). In most open space areas, all structure types except for the natural channel and channel levee would be context sensitive. In the South Mountain area, only the natural structure would be context sensitive. In limited locations in the northeast portion of the study area all methods except the natural structure would be context sensitive.

### 6.5.1.2 Context Sensitive Structural Methods

For most of the study area, Natural, Soft Structural and Semi-Soft Structural methods would be part of a context sensitive solution (see Figure 6-12). In the north and eastern parts of the study area and along some of the arterial roads, enhanced Hard Structural methods might also be considered context sensitive.

### 6.5.1.3 Context Sensitive Landscape Design Themes

The large number of landscape design themes creates a somewhat complex array of landscape themes that would be a context sensitive part of a flood control solution. However, there is generally a good selection of landscape themes that would be considered context sensitive in most areas so that the landscape design would have the flexibility to meet a variety of site specific landscape conditions. In general the Natural Sonoran Desert Upland and Upland Riparian themes would be context sensitive in the South Mountain area (see Figure 6-14). Semi Natural Sonoran Desert, Enhanced Desert and Desert Oasis would be context sensitive in the bajada areas at the base of the mountain. For most of the study area, the Natural Lower Sonoran Desert themes, the Semi-natural Sonoran Desert, enhanced Desert and Desert Park would be context sensitive.



## Hohokam Area Drainage Master Study/Plan Context Sensitive (ACE) Structure Types Class Map

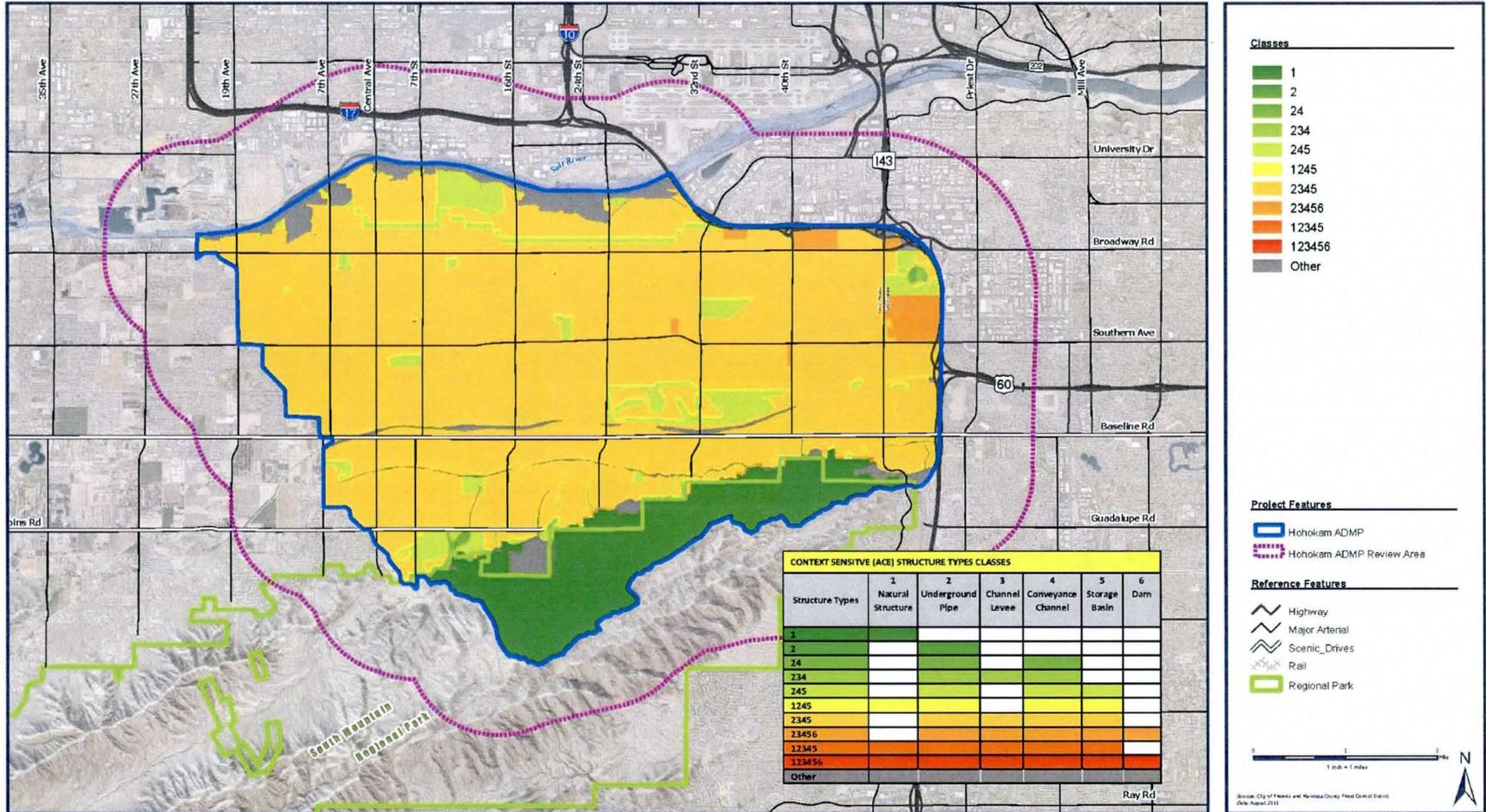
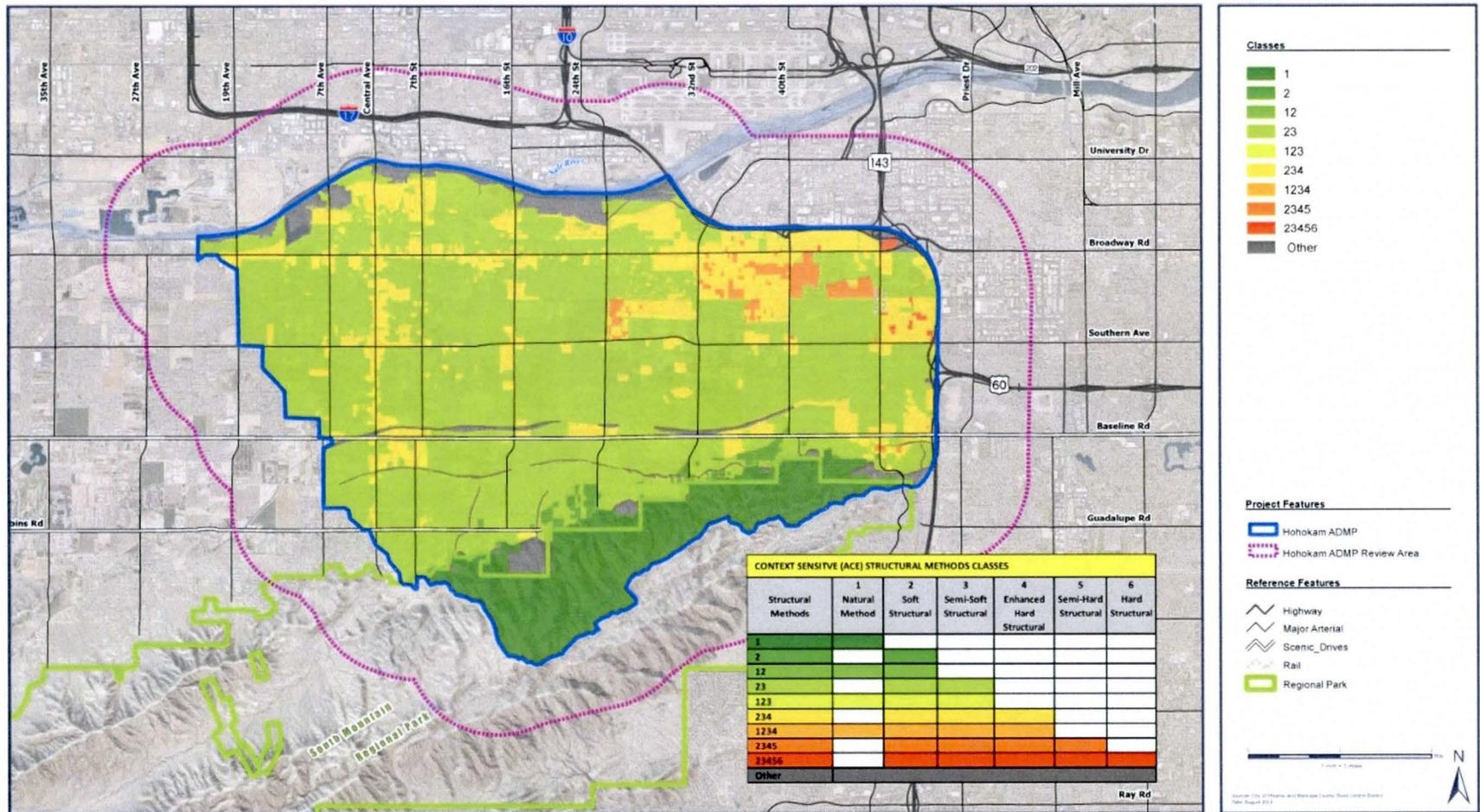


Figure 6-12: Structure Types Context Sensitive Classes



## Hohokam Area Drainage Master Study/Plan Context Sensitive (ACE) Structural Methods Class Map



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**Figure 6-13: Structural Methods Context Sensitive Classes**

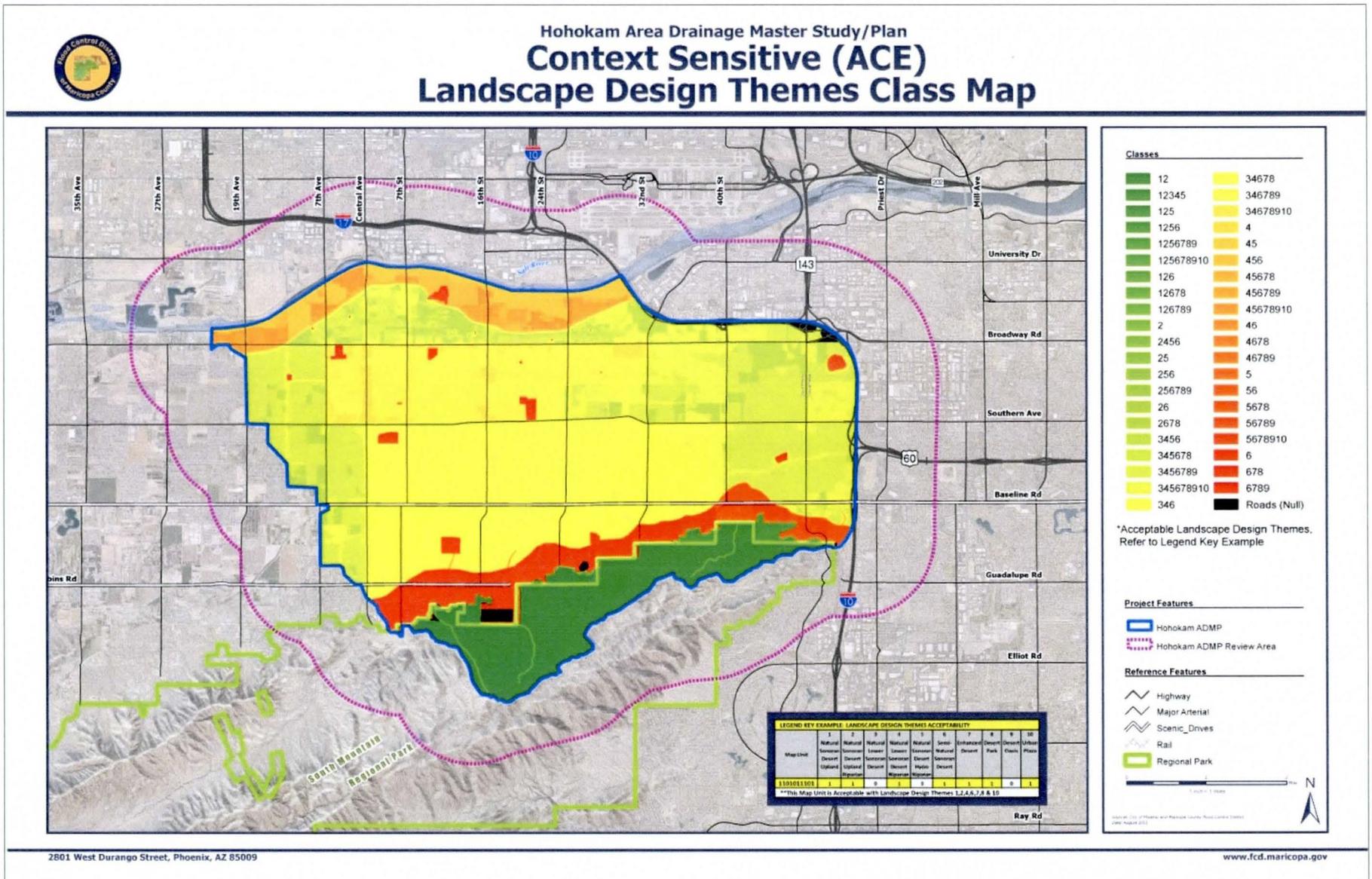


Figure 6-14: Landscape Design Themes Context Sensitive Classes

## 7. BRAINSTORMING SESSION

### 7.1 GENERAL

Project team members, additional District personnel, and project stakeholders were invited and participated in the Phase I Brainstorming session. For purposes of the brainstorming session, eight areas were defined based upon the identified flood hazards areas and the potential source of the flooding issues. The areas are only generally defined and often overlap with adjacent areas since flooding issues are often interrelated (see Figure 7-1). At the session, the results of the data collection effort were provided including the general plans, cultural and land resources, study area opportunities and constraints and the results of the Landscape Inventory Analysis performed by the District. The results of the FLO-2D analyses were also presented including a FLO-2D animation to help define the problem areas and show potential flooding sources and contributing drainage areas. Participants were then divided into six working groups with each group assigned a brainstorming area with the exception of one group (Group 6) that was assigned three (Areas 6A, 6B and 6C). The groups then brainstormed ideas to address the flooding issues and presented them to the entire project team.

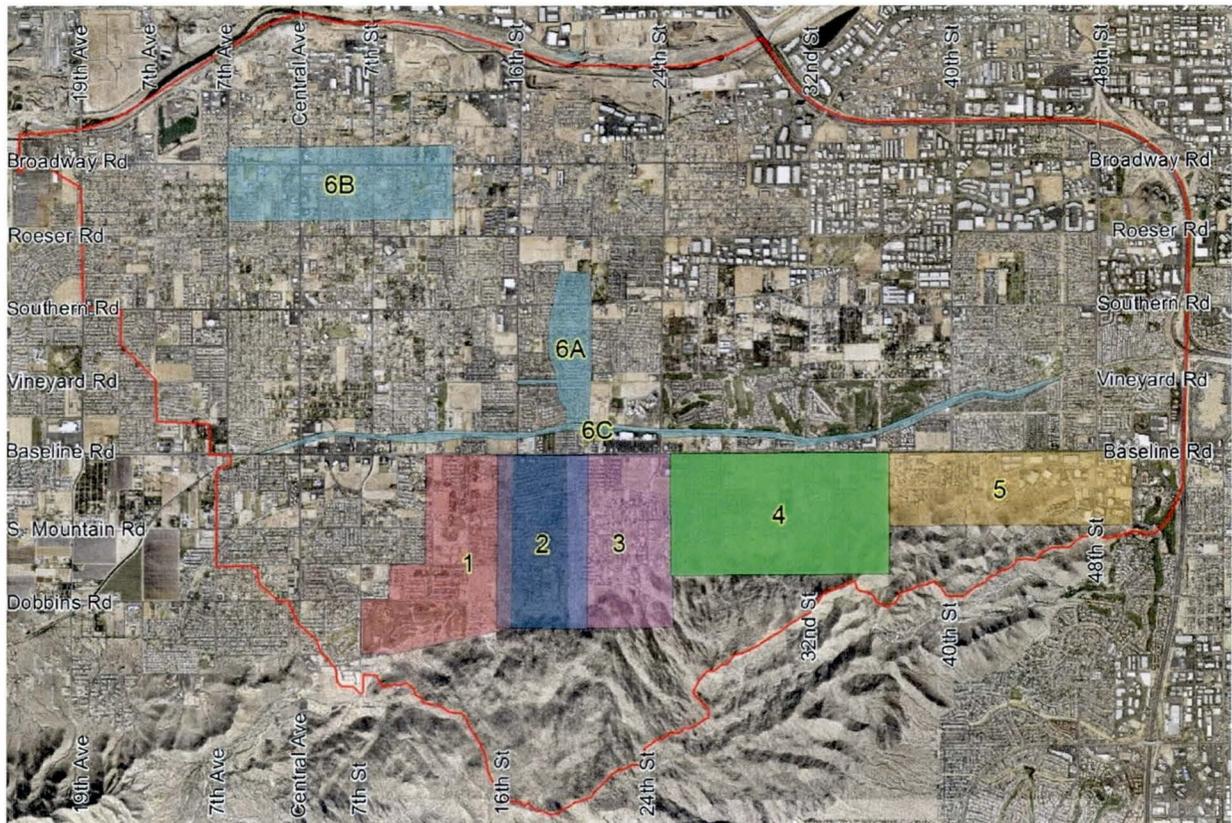


Figure 7-1: Brainstorming Areas

## 7.2 BRAINSTORMED ALTERNATIVES

This section summarizes the alternatives developed as part of the brainstorming session. Alone, the specific concept summarized may not achieve the desired flood mitigation but may be proposed as a component of an alternative comprised of multiple concepts.

### 7.2.1 Area 1: 7<sup>th</sup> St – 16<sup>th</sup> St (Circle K Park)

- 1) Dobbins Rd Channel: Channel east-west along Dobbins Road as a means to capture and convey mountain runoff (9<sup>th</sup> St to 20<sup>th</sup> St)
- 2) 16<sup>th</sup> St/Dobbins Basin: Basin in vacant parcel northwest of 16<sup>th</sup> St and Dobbins
- 3) Basin south of Circle K Park: Purchase low density residential parcels south of the park for a basin
- 4) Circle K Park Basin: Rebuild/regrade Circle K Park to provide retention
- 5) Basin Northwest of Circle K Park: Basin in vacant parcel north of Highline Canal and west of park
- 6) Siphon under Highline Canal: Construct siphon under Highline canal (to convey floodwater to above basin north of Highline Canal)
- 7) Improvements to be included as part of golf course development: Require/work with future development of Thunderbird Country Club Golf Course/Vistal Development to assure and possibly to provide additional improvements for area flood mitigation (retention, channel)
- 8) Dobbins/20<sup>th</sup> St Basins: Basin in vacant area southeast of Dobbins/20<sup>th</sup> St (south of Siesta Foothills)

### 7.2.2 Area 2: 16<sup>th</sup> St – 20<sup>th</sup> St (Siesta Foothills/Boy Scout Camp)

- 1) Boy Scout Camp Basin System (Eastern System):
  - a. Capture Boy Scout Camp Wash flows into a basin southeast of Dobbins
  - b. Convey to a larger basin
  - c. Convey from basins via new storm drain in 20<sup>th</sup> St.
  - d. Scalp peak with basins on either side of 20<sup>th</sup> St (or Highline Canal) subject to Baseline storm drain capacity
- 2) 16<sup>th</sup> St Basins and Storm Drain (Western System):
  - a. Basin upstream of Dobbins at 16<sup>th</sup> St

- b. Route flows from basin under Dobbins via pipe to a basin northwest of 16<sup>th</sup> St and Dobbins (southeast of Euclid & 14<sup>th</sup> St).
  - c. Discharge basin into storm drain along 16<sup>th</sup> St
  - d. Discharge storm drain to Baseline Rd storm drain
- 3) Boy Scout Camp Offline Basin System (Eastern System):
- a. Offline basin to boy Scout Camp Wash flows
  - b. Convey westerly via channel to Western System Alternative (above)
- 4) Boy Scout Camp Offline Basin System 2 (Eastern System):
- a. Offline basin to boy Scout Camp Wash flows
  - b. Convey flow by storm drain to along Euclid and west to storm drain system in 16<sup>th</sup> St.

### 7.2.3 Area 3: 20<sup>th</sup> St – 24<sup>th</sup> St (Pines at South Mountain)

- 1) String of Pearls System: Series of channel and basins along the existing flow path west of 23<sup>rd</sup> Pl. Take flow to basins at northwest corner of 24<sup>th</sup> St / Highline Canal.
- 2) Highline Canal Channel: Channel to deliver flow to a basin(s) at northwest corner of 20<sup>th</sup> St / Highline Canal and/or at northwest corner of 24<sup>th</sup> St / Highline Canal
- 3) Dikes along base of mountains: Dike system along base of mountains
- 4) Channel along base of mountains: Channel system along base of mountains
- 5) Mountain Dam: Dam across major flow west of 24<sup>th</sup> St in mountains.
- 6) Dam at 24<sup>th</sup> St: Dam at 24<sup>th</sup> St and Euclid
- 7) Canal Storm Drain System: Storm drain system parallel to Highline Canal. Discharge to extensions of the existing storm drain system south of Baseline Rd.
- 8) East String of Pearls: Series of basins that delivers flow to a big basin at Baseline and 24<sup>th</sup> St.
- 9) Increase park retention: Enlarge existing retention in park
- 10) Move/Remove Houses: Move all houses.

- 11) Spillways from Highline Canal: Provide spillways in Highline Canal to detention basins.

#### **7.2.4 Area 4: 24<sup>th</sup> St – 36<sup>th</sup> St (Cortland Point)**

- 1) 28<sup>th</sup> St & South Mountain System:
  - e) Basins south of Winston
  - f) Outfall storm drain 27<sup>th</sup> St to South Mountain Ave
  - g) Collector storm drains in South Mountain Ave
  - h) Outfall basins at Puerto Park
- 2) 32<sup>nd</sup> St & Highline Canal System:
  - f) Bleed-off storm drain to 24<sup>th</sup> St storm drain
  - g) Collector storm drain at South Mountain Ave from 30<sup>th</sup> St to 32<sup>nd</sup> St
  - h) Collector storm drain on south side of Highline Canal from 34<sup>th</sup> St-32<sup>nd</sup> St
  - i) Outfall storm drain and basin at 32<sup>nd</sup> St (basin west side of 32<sup>nd</sup> St north of Highline Canal.)
  - j) Bleed off storm drain on 32<sup>nd</sup> St north to Baseline
- 3) 36<sup>th</sup> St & Highline Canal System:
  - d) Small basins along the Highline Canal to either the north or south side from 34<sup>th</sup> PI to 36<sup>th</sup> St
  - e) Channel/floodwall on north side of Highline Canal from 34<sup>th</sup> PI to 36<sup>th</sup> St outfalling to basin(s)
  - f) Outfall storm drain north in 36<sup>th</sup> St to Baseline then west in Baseline to 32<sup>nd</sup> St storm drain.

#### **7.2.5 Area 5: 36<sup>th</sup> St – 48<sup>th</sup> St (South Mountain Industrial Area)**

- 1) 42<sup>nd</sup> St or 42<sup>nd</sup> PI / Baseline Basin: Basin at end of wash south of Baseline. Bleed off to existing storm drain in Baseline. Potential multi-use opportunity as a trailhead for park
- 2) Basin at Beverly Road S of Highline Canal: Basin southwest of 14<sup>th</sup> St and Vineyard

#### **7.2.6 Area 6A: 16<sup>th</sup> St – 20<sup>th</sup> St (North of Western Canal)**

- 1) Storm Drain: Provide a storm drain system along 18<sup>th</sup> St, 20<sup>th</sup> St and/or Vineyard to capture flows.
- 2) 16<sup>th</sup> St / Vineyard Basin: Basin in undeveloped area west of 20<sup>th</sup> St & south of Vineyard (undeveloped property belonging to South Mountain Community College)

- 3) 14<sup>th</sup> St / Vineyard Basin: Basin southwest of 14<sup>th</sup> St and Vineyard
- 4) 16<sup>th</sup> St / Vineyard Basin: Basin northwest of 16<sup>th</sup> St and Vineyard

### **7.2.7 Area 6B: 3<sup>rd</sup> St & Broadway (Low Laying Area)**

- 1) Multiple Small Basins: Provide retention in available vacant parcels in general areas that bleed to the storm drain on Broadway or Central
- 2) Extend/expand storm drain into/in the area to alleviate flooding (and bleed basins): Extend the storm drain systems from Central and/or Broadway into area to capture flows and provide a means to bleed off multiple small retention basins.

### **7.2.8 Area 6C: Ponding along the south side of Western Canal**

- 1) Basins along south side of Canal: Provide a storm drain system along 18<sup>th</sup> St, 20<sup>th</sup> St and/or Vineyard to capture flows.
- 2) No Action: Expectation that improvements upstream would address and/or alleviate ponding along the canal.

## **7.3 SEED IDEAS**

Prior to the brainstorming session, 48 seed ideas were conceived for the twenty one problem locations previously described (Section 4.4) and for general areas in order to address more widespread or systemic problems such as storm drain deficiencies, flooding along the Highline Canal and future development. These seed ideas are summarized in a table provided in Appendix C. The table briefly describes the idea then indicates which problem areas the idea would address and the nature of the impact on the flooding location. Because many of the flooding issues are interconnected, the impact of a seed idea was categorized as being either "primary" or "secondary". A "primary" impact would indicate the idea would have a direct impact on flows contributing to the flooding problem at that specific location. A "secondary" impact would indicate the idea would have a lesser or more indirect impact on the flooding problems.

These seed ideas were not presented to the participants as part of the brainstorming session, so as not to influence or bias potential ideas generated by the attendees. Many of the seed ideas were similarly envisioned by the brainstorming groups, however, some were not. Consequently, the seed ideas are included for possible consideration in Phase II.

## **8. PHASE II ALTERNATIVE DEVELOPMENT**

### **8.1 ALTERNATIVE DEVELOPMENT PROCESS**

Phase II, the Hohokam Area Drainage Master Plan (ADMP), will utilize the results of the ADMS to formulate flood mitigation alternatives and through three levels of alternative formulation, analysis and evaluation, ultimately make recommendations for drainage improvements for the study area. The ADMP will expand upon the public outreach and involvement efforts and develop concept plans for recommended improvements. Recommended improvements will be prioritized and an implementation plan will be prepared.

#### **8.1.1 Level 1: Alternatives Formulation and Preliminary Analysis**

For Level 1, the developed brainstorming alternatives and seed ideas generated in Phase I will be utilized, in part or in their entirety, to develop more comprehensive alternatives for each problem area to be evaluated further in Level 2. The preliminary analysis and evaluation of alternatives will be based upon information available from the Phase I hydrologic & hydraulic analyses and other data collection efforts. A preliminary evaluation matrix will be developed and used to help assess and identify alternatives for further development and assessment in Level 2.

#### **8.1.2 Level 2: Alternatives Analysis**

For Level 2, alternatives will be further developed to determine the engineering feasibility, effectiveness and approximate costs. The alternatives will be developed to an extent to identify potential utility impacts, ROW/land acquisition requirements, potential multi-use opportunities, and environmental/cultural resource issues. Integral to the development and assessment of the alternatives will be the District's CSFHM approach that will consider flooding context, land and resource context and community text. A more detailed evaluation will be used to assess project alternatives and public input will be obtained and considered as part of the community context to identify recommended alternatives for further development in Level 3.

#### **8.1.3 Level 3: Recommended Alternatives Analysis**

Level 3 will develop the recommended alternatives to a level at which 15% plans can be completed. Hydrologic and hydraulic models will be developed to reflect the recommended alternatives. A landscape and multiple-use guidelines along with an implementation plan will be developed to guide future development and provide a strategy for implementation. Public meetings will be held to present the recommended alternatives.

## 9. REFERENCES

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**APPENDIX A**

**Flood Insurance Rate Maps  
(FIRMs)**









**NOTES TO USERS**

This map is to be used in determining the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, nor does it identify areas of special hazard. The community risk engineer should be consulted for areas not shown on this map.

To obtain more detailed information on areas shown as **Base Flood Elevations (BFE)** or **Special Flood Hazard Areas (SFHA)**, users are encouraged to consult the Flood Insurance Rate Study (FIRMS) report for the community. The FIRMS report contains the Flood Insurance Study (FIS) report for the community. The FIS report should be used for all information regarding areas shown on this map. The FIS report should be used in conjunction with the FIRMS report for purposes of construction and flood management.

**Coastal Base Flood Elevations** shown on this map apply only to areas of 0-10 National Geodetic Vertical Datum of 1929 (NGVD 29). Users of this FIRMS should be aware that coastal base elevations are also provided in the Summary of Statistics, Elevation Table in the Flood Insurance Study report for this jurisdiction. Elevations shown on the Summary of Statistics Elevation Table should be used for construction and flood management purposes when they are higher than the elevations shown on this FIRMS.

**Boundaries of the Floodways** were computed at cross sections and interpolated between cross sections. The Floodways were based on hydraulic computations with regard to requirements of the National Flood Insurance Program. Floodways, walls and other pertinent features are as provided in the Flood Insurance Study report for the jurisdiction.

Certain areas not on Special Flood Hazard Areas may be protected by **Wetland** and **Other** areas. Refer to Section 2.4 Flood Protection Measures of the Flood Insurance Study report for information on flood control structures for the jurisdiction.

The **Imperviousness** used in the preparation of this map was National Flood Insurance Zone 374 (Urban Residential). The **horizontal datum** used is NAD83. (GNSS) satellite differences in datum adjustment projections on State Plane zones used in the production of FIRMS for adjustment or corrections may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of the FIRMS.

Flood elevations on this map are referenced to the National Geodetic Vertical Datum of 1929. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov> or contact the National Geodetic Survey at the following address:

National Geodetic Survey  
 National Geodetic Survey Office  
 Silver Spring, Maryland 20910  
 1215 East-West Highway  
 Silver Spring, Maryland 20910  
 301-761-3201

To obtain current elevation information, contact information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 761-3202 or visit us online at <http://www.ngs.noaa.gov>.

**Base Map** information shown on this map was derived from multiple sources. Base map files were provided in digital format by Maricopa County. Orthographic images were provided at a scale of 1:5000 using NOAA's GeoEye satellite imagery as of December 2009.

This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRMS for this jurisdiction. The boundaries and features that were transferred from the previous FIRMS may not have been updated to conform to these new stream channel configurations. As a result, the Flood Insurance Study, Elevation Table in the Flood Insurance Study report, which contains configuration specific data, may reflect stream channel features that differ from what is shown on this map.

**Coastal Base Flood Elevations** on this map are based on the best data available at the time of publication. It is recognized that for applications on the elevation may have occurred after this map was published, may contain certain appropriate community features to meet current elevation level features.

Please refer to the appropriate **Map Index** for an overview of the county showing the layout of map sheets, community map numbers, addresses, and a listing of Communities with boundaries National Flood Insurance Program data for each community as well as a listing of the panels in which each community is located.

Contact the **FIRMS Map Service Center** at 1-800-358-3614 for information on available products associated with the FIRMS. Additional products may include previously issued editions of this map. A Flood Insurance Study report and digital versions of this map. The FIRMS Map Service Center may also be reached at Tel: 1-800-358-3614 and its website at [www.firms.gov](http://www.firms.gov).

If you have **questions about this map**, or questions concerning the National Flood Insurance Program in general, please call 1-877-874-8747 or 1-877-338-3327 or visit our Web Page website at [www.firms.gov](http://www.firms.gov).



**LEGEND**

**SPECIAL FLOOD HAZARD AREAS SUBJECT TO INSURATION BY THE NATIONAL FLOOD INSURANCE PROGRAM**

- ZONE A1** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE A2** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE A3** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE A4** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE A5** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE X** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood
- ZONE VE** Special Flood Hazard Area (SFHA) - Areas of 1% Annual Chance Flood

- OTHER FLOOD AREAS**
- ZONE X** Areas of 1% Annual Chance Flood
- OTHER AREAS**
- ZONE D** Areas of 1% Annual Chance Flood
- COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS**
- OTHERWISE PROTECTED AREAS (OPA)**

- CBRS AREAS AND OPAs** are normally marked with an asterisk in Special Flood Hazard Areas
- 1% Annual Chance Flood**

**MAP REVISIONS**

REVISION 1: 10/15/09 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 2: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 3: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 4: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 5: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

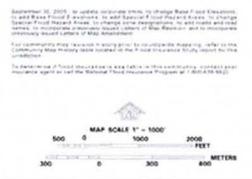
REVISION 6: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 7: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 8: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 9: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.

REVISION 10: 07/18/2010 - Updated Flood Insurance Rate Study (FIRMS) data for the community.



**NATIONAL FLOOD INSURANCE PROGRAM**

**PANEL 2610F**

**FIRM FLOOD INSURANCE RATE MAP**

**MARICOPA COUNTY, ARIZONA AND INCORPORATED AREAS**

**PANEL 2610 OF 4350**

DATE: 07/18/2010 10:00 AM

COMMUNITY: MARICOPA COUNTY

MAP NUMBER: 04013C2610F

MAP REVISED: SEPTEMBER 30, 2005

Federal Emergency Management Agency



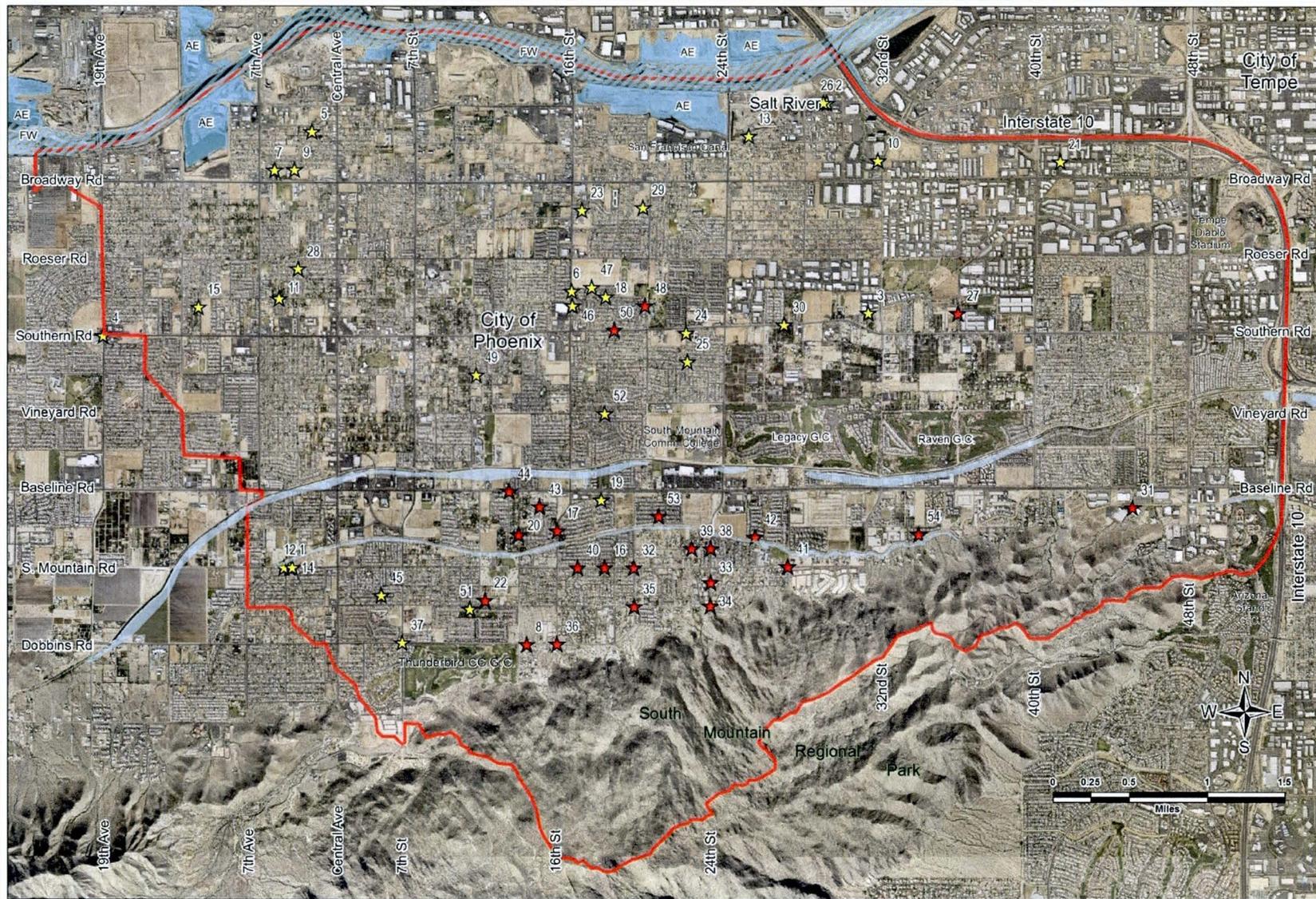
**APPENDIX B**

**DRAINAGE COMPLAINTS  
EXHIBIT**



# Hohokam Area Drainage Master Study/Plan

## Contract FCD2009C029



### Legend

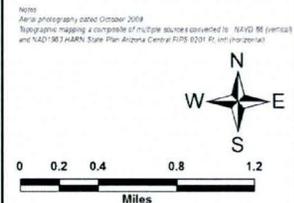
ADMS Boundary

#### Drainage Complaints

- Local Issue
- Regional Issue

#### Flood Hazard Zone

- Zone A
- Zone AE
- Floodway



**Drainage Complaint Locations Exhibit**



# APPENDIX C

## SEED IDEAS

Seed Alternatives for Hohokam ADMS

Concept	Description	Montana Vista	16th St. from Dobbins Rd. to Baseline Rd.	Circle K Park	Thunderbird CCGC	S. Mountain Ave. & 17th Way	Vista Portica	Pines at South Mountain	Siesta Foothills Area	21st Pl. & Euclid Ave.	22nd St. - 24th St. North of S. Mountain Ave.	25th Pl. - 32nd St. Ponding Along Highline Canal	Cortland Point	Shadow Mtn Villas	42nd St. & Baseline Rd.	46th St. & Beautiful Lane	Ponding along the Western Canal	Vineyard Rd/18th St/19th Pl/20th St	Contempo Tempe Mobile Home Park	S. Mountain Community College (SMCC)	2nd St. & Weir Ave	19th St from Euclid Ave to Highline Canal	Along Highline Canal	Storm Drain Deficiencies	Future Developments								
		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	A	B	C	D	E						
1	New retention basin in vacant parcel north/west of Montana Vista. Could also provide retention of flow thru TGCC/Vistal and alleviate 16th St.	P	P	P	P																												
2	Retention basins in several parcels south of Dobbins Rd at 16th St	P	P	S																													
3	Channelize flow & new culvert across Dobbins, extend rectangular channel downstream of MV to a (new) nearby retention basin.	P	P	S																													
4	New retention basin at: * NW of S Mtn & 16th St * S of Baseline & W of 16th St		P	S																													
5	New 16th St storm drain connected to * Baseline storm drain * new retention basins along 16th St		P	S																													
6	Modify Dobbins Creek basins to improve retention (expand, modify to balance flow between basins, change distribution of flow to basins)			P	P																												
7	COP to stipulate Vistal development to provide sufficient retention and distribute flow to reduce downstream flooding.			P	P																												
8	Retention basin in Circle K Park (in desert landscape area)		S	P																													
9	Retention basin in vacant land north of Circle K Park between 12th St and 13th St.			P																													
10	Retention basin in vacant land east of 14th St and north of Circle K park.			P																													
11	Provide retention basin in boy scout camp facility					P	P	P	S																								
12	Storm drain along 17th Way and S. Mtn to downstream retention or storm drain system					P	P	S																									
13	Construct inverted crown road along 17th Way and S. Mtn and then collect flow into a storm drain or retention basins					P	P	S																									
14	19th or 20th St storm drain/conveyance from Dobbins or Euclid to downstream retention or to extension of storm drain system.					P	P	S	S																								
15	Retention basin in vacant land S of Siesta Foothills & E of Dobbins/20th St							P	P																								
16	Storm drain from Siesta Foothills culvert to a retention basin at the SE or NW corner of 20th St & Highline Canal or to storm drain system							P	P																								
17	Inverted crown road to convey more flow to drainage easement									P																							

Seed Alternatives for Hohokam ADMS

	Description	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	A	B	C	D	E
	Concept	Montana Vista (16th St. & Dobbins Rd.)	16th St. from Dobbins Rd. to Baseline Rd.	Circle K Park (14th St. & South Mountain Ave.)	Thunderbird CCGC (7th St. & Dobbins Rd.)	S. Mountain Ave. & 17th Way	Vista Portica (17th Way & S. Mountain Ave.)	Pines at South Mountain (21st Way & Baseline Rd.)	Siesta Foothills Area (20th St. & Euclid Ave.)	21st Pl. & Euclid Ave.	22nd St. - 24th St. North of S. Mountain Ave.	25th Pl. - 32nd St., Pondering Along Highline Canal	Cortland Point (36th St. & Highline Canal)	Shadow Mtn Villas (36th St. & Baseline Rd.)	42nd St. & Baseline Rd.	46th St. & Beautiful Lane	Ponding along the Western Canal	Vineyard Rd/18th St/19th Pl/20th St	Contempo Tempe Mobile Home Park	S. Mountain Community College (SMCC)	2nd St. & Weir Ave	19th St from Euclid Ave to Highline Canal	Along Highline Canal	Storm Drain Deficiencies	Future Developments		
18	Retention basin E of 22nd Pl and N and/or S of S. Mtn Ave							S			P																
19	Retention Basin NE and NW of Highline Canal and 24th St							P	P	S	P	P															
20	Channel from South Mountain Ave to north of Highline Canal between 22nd Pl and 23rd Pl alignments.							S	S		P																
21	Retention Basin Sof Winston and 27th St											P															
22	Expand existing retention basin E of 28th St and S of S.Mtn Ave											P															
23	Retention Basin N of Highline and W of 32nd St.											P										P					
24	Collector channel or storm drain along Valley View Dr W of Patricia St and along Patricia St (along edge of S. Mtn Park)											P															
25	Retention basin along Patricia St S of Winston Dr (near 27th St and edge of S. Mtn Park)											P															
26	Retention upstream of Canal at Cortland Point												P	S													
27	Retention basin N of Highline Canal and E of 36th St											P	S														
28	At Cortland Pt, Regrade behind block walls along the Highline Canal to drain flow to the east.											P	S														
29	Retention basin along S of Baseline and W of 42nd St														P												
30	Increase 40th St Storm Drain Capacity to accomodate some flow														P												
31	Retention Basin S of Beautiful Lane and W of 46th St upstream of existing building															P											
32	Retention Basin W of 48th St and N of Beverly Lane															P											
33	Expand retention basin NW of 24th St and Western Canal, construction collector channel along N side of Western Canal to convey flow to basin.																S			P							
34	Where possible, construction a collector channel along the S side of the conal to divert flow to retention basins																P										
35	Retention Basin in vacant land N of Weteran Canal and W of SMCC (around track)																	P		S							
36	Extend a storm drain lateral from 16th St down Vineyard Rd to 19th Pl																	P									
37	Extend storm drain laterals from Southern Ave down 18th St and 20th St																	P									
38	Retention Basin in mobile home park just SE of existing Basin																		P								

Seed Alternatives for Hohokam ADMS

Description		1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	A	B	C	D	E	
Concept		Montana Vista (16th St. & Dobbins Rd.)	16th St. from Dobbins Rd. to Baseline Rd.	Circle K Park (14th St. & South Mountain Ave.)	Thunderbird CCGC (7th St. & Dobbins Rd.)	S. Mountain Ave. & 17th Way	Vista Portica (17th Way & S. Mountain Ave.)	Pines at South Mountain (21st Way & Baseline Rd.)	Siesta Foothills Area (20th St. & Euclid Ave.)	21st Pl. & Euclid Ave.	22nd St. - 24th St., North of S. Mountain Ave.	25th Pl. - 32nd St., Ponding Along Highline Canal	Cortland Point (36th St. & Highline Canal)	Shadow Mtn Villas (36th St. & Baseline Rd.)	42nd St. & Baseline Rd.	46th St. & Beautiful Lane	Ponding along the Western Canal	Vineyard Rd/18th St/19th Pl/20th St	Contempo Tempe Mobile Home Park	S. Mountain Community College (SMCC)	2nd St. & Weir Ave	19th St from Euclid Ave to Highline Canal	Along Highline Canal	Storm Drain Deficiencies	Future Developments			
39	Extend new/expand existing lateral from Broadway S down 2nd St																											
40	Construct a basin at SEC of Broadway and Central Ave																											
41	Rapid Infiltration Basin or Ret Basin with Rapid Dry Wells at small parcel on SWC of 2nd St and Marguerite Ave																											
<b>General Concepts</b>																												
42	Where possible, construction a collector channel along the S side of the canal to divert flow to retention basins			P				P			P	P	P	S			P	P	S				P					
43	Inverted crown roads with storm drain underneath for low flow events.	P	P	P		P	P	S	S	P	P	S	P	S				P	P		P	P						
44	Development of sub-regional storm drain systems south of Broadway. e.g. Dobbins/Euclid/S. Mtn-16th St storm drain	P	P	P																								
45	Utilize Highline Canal to collect flows. Canals drained directly to adjacent retention and/or connected by pipe to retention basins and/or the storm drain system.		P	P				P					S	P	S								P					
46	COP to require stipulations to provide storage along the Highline Canal specifically to address offsite flows and overtopping of the canal.																								P			
47	"Tile" Highline Canal																											
48	Retention basins in large parcels throughout the study area to serve as outfalls to new local storm drain systems or relieve to flow in existing storm drain system.	P	P	P		P	S				P	P						P			P	P	P	P				

P indicates the alternative is expected to have a primary impact for the specific problem area  
 S indicates the alternative is expected to have an impact on the specific problem area but a lesser or secondary impact.

